

Appendix A

Hydrostratigraphic Model of Cambridge, Ontario

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Section 1

Introduction

This report documents the development of the hydrostratigraphic model for the bedrock in the area of Cambridge, Ontario. The analyses described in this report build on the fundamental efforts of Frank Brunton, Ontario Geology Survey (Brunton 2008, 2009). Jinhui Zhang has conducted the analyses and Christopher Neville is responsible for internal review.

Hydrogeologic analyses in development for the Cambridge area incorporate layers representing both overburden and bedrock units; however, during the development of the hydrostratigraphic model, particular attention is directed toward the bedrock. This emphasis reflects the fact that groundwater supplies in the Cambridge area are derived primarily from bedrock wells. Recent well testing in the Pinebush Well Field (PBPW1-06) and Clemens Mill Well Field (CMPW1-06 and CMPW2-06) also focused on the bedrock (Golder, 2011a).

In several instances, the developed layers correspond to the stratigraphic units identified in Brunton (2008, 2009). However, there are instances in which model layers are delineated to represent key aquifer and aquitard units that are not tied to a particular geological formation. For example, a separate layer is defined to represent the interface between the overburden and the top of rock. Therefore, the work presented here is referred to as a hydrostratigraphic model, following the terminology of Maxey (1964) and Seaber (1988). A hydrostratigraphic unit is defined as a body of rock that composes a geologic framework for a reasonably distinct hydrologic system (Maxey, 1964; p. 126). These units are intended to serve as a fundamental unit for describing hydrogeologic systems in the field based on the properties of the rock that affect groundwater conditions (Seaber, 1988).

Previous groundwater models of the Cambridge area have not incorporated analyses of the bedrock structure with comparable detail (Golder, 1991; Beak et al., 1995; and Duke Engineering, 1998). An immediate question that arises is: why is it now necessary to revisit the bedrock hydrostratigraphy in the Cambridge area? The groundwater modelling for the Cambridge area provides an opportunity to incorporate the recent advances made by the Ontario Geological Survey in characterizing the bedrock (Brunton, 2008, 2009). The framework developed by the Ontario Geological Survey provides for the first time a context for understanding the important differences in the characteristics of the bedrock across the Cambridge area. This framework guides the systematic interpretation of hydrogeologic data, and is predictive in the sense that it provides a defensible structure for the extrapolation of data beyond areas of concentrated data. This framework is essential in an analysis that is extended to natural hydrologic boundaries. The Ontario Geological Survey framework also provides a foundation for predicting the effects of developing additional groundwater resources in areas that do not have the benefit of long-term performance records.

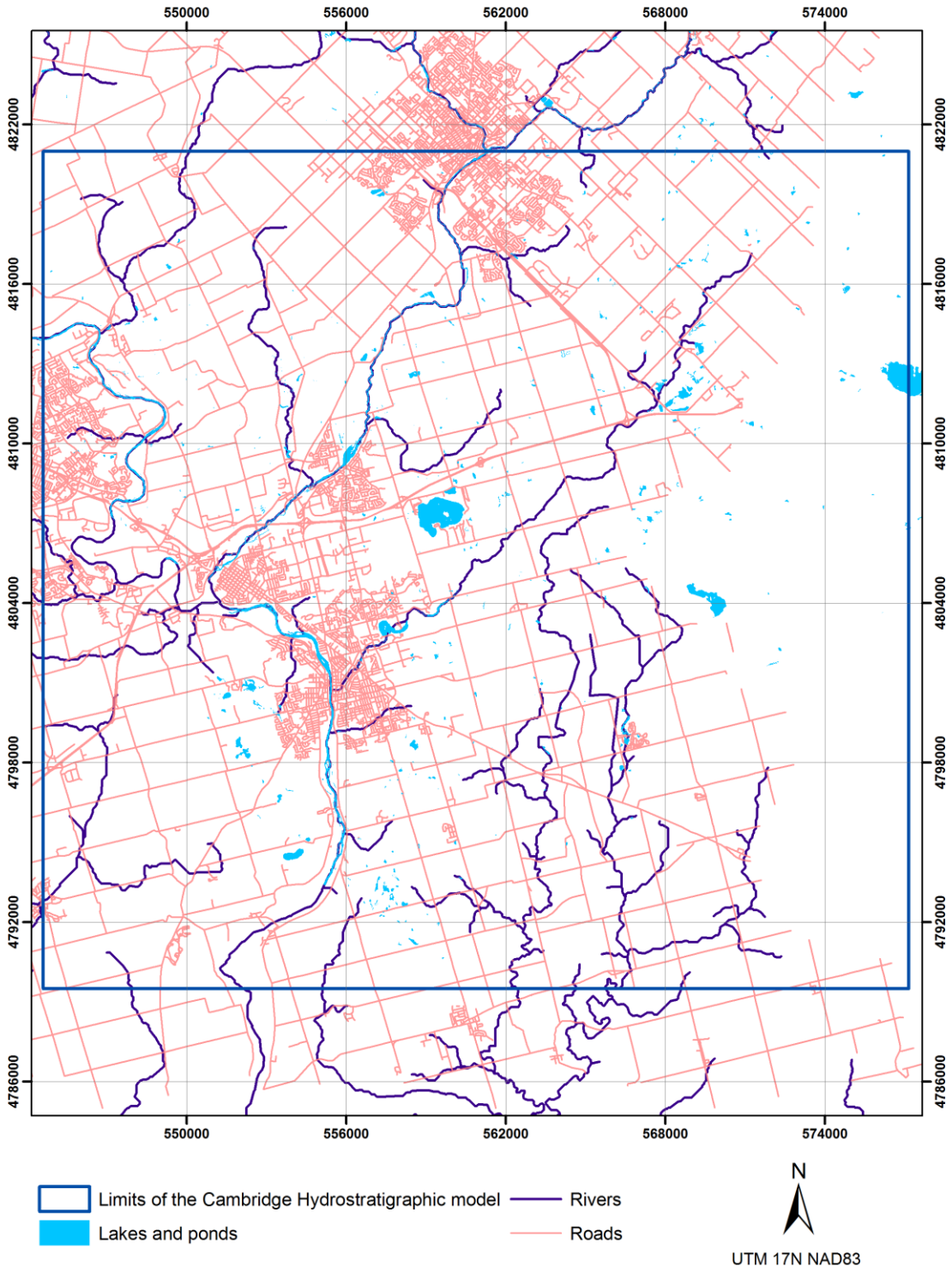
Section 2

The study area and the model surfaces

The limits of the Cambridge hydrostratigraphic model analyses are shown in **Figure 2-1**. The limits of the hydrostratigraphic model extend beyond the limits of the groundwater flow model that has been developed recently for the Cambridge area. The limits of the hydrostratigraphic model were extended deliberately beyond the limits of the groundwater model to ensure that the groundwater model could have been enlarged, had that been deemed necessary. When generating surfaces it is also simpler to work in a rectangular domain than the irregular limits of the groundwater flow model. Independent analyses of groundwater flow in the Guelph and Cambridge areas are being developed to support Tier 3 Water Budget studies in the City of Guelph, the Region of Waterloo, and an environmental assessment for the Cambridge East area. There is significant overlap of the study areas for Guelph and Cambridge and it is important that the hydrostratigraphic model be sufficiently large to extend across the area of overlap.

The Guelph Tier 3 model builds on the recent studies of the Ontario Geological Survey (Brunton, 2008, 2009). Brunton's proposed revisions to the Silurian stratigraphy are shown in **Figure 2-2** alongside the previous designations of Johnson et al. (1992). As will be discussed in detail in Sections 7 and 8, a key element of the recent Ontario Geological Survey work is the identification of members of the Eramosa Formation that have significantly different characteristics. The hydrostratigraphic model for the Guelph Tier 3 study is shown in **Figure 2-3**. The Guelph Tier 3 model includes 14 model layers to represent the overburden and 13 bedrock stratigraphic units, with some lumping of units within model layers. In collaboration with Frank Brunton, Golder Associates developed estimates of the elevations of the tops of seven of these units, indicated by the red triangles in the figure.

The layer structure for the Cambridge model is shown in **Figure 2-4**. The model includes the overburden and 12 geologic units. The top of the Cabot Head Formation is set as the bottom of the model. As discussed in Section 10, the hydraulic conductivity of the Cabot Head Formation is sufficiently low that groundwater flow within the unit is likely insignificant. The elimination of a model layer to represent the Cabot Head will allow incorporation of an additional layer to refine the representation of the overburden without increasing the overall size of the analysis. The red triangles indicate the surfaces for which estimates of elevations are available in the Cambridge area.



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Figure 2-1 Limits of the study area

Johnson et al.
(1992)

Brunton (2008, 2009)
Guelph/Cambridge

	Formation	Member
Guelph Fm.	Guelph	Hanlon
		Wellington
Eramosa Mbr. of Guelph Fm.	Eramosa	Stone Road
		Reformatory Quarry
		Vinemount
Amabel Fm.	Goat Island	Ancaster
		Niagara Falls
Unsubdivided members and Irondequoit Fm.	Gasport	Pekin ? <small>(Might be present at Shades Mill)</small>
		Gothic Hill
	Rochester	
	Irondequoit	
Reynales Fm.	Rockway	
	Merritton <small>(= Upper Fossil Hill)</small>	
Cabot Head Fm.	Cabot Head	

Not to scale

Figure 2-2 Johnson et al. (1992) and Brunton (2008, 2009) stratigraphic designations

Brunton (2008, 2009)

Guelph Tier 3 groundwater model layers

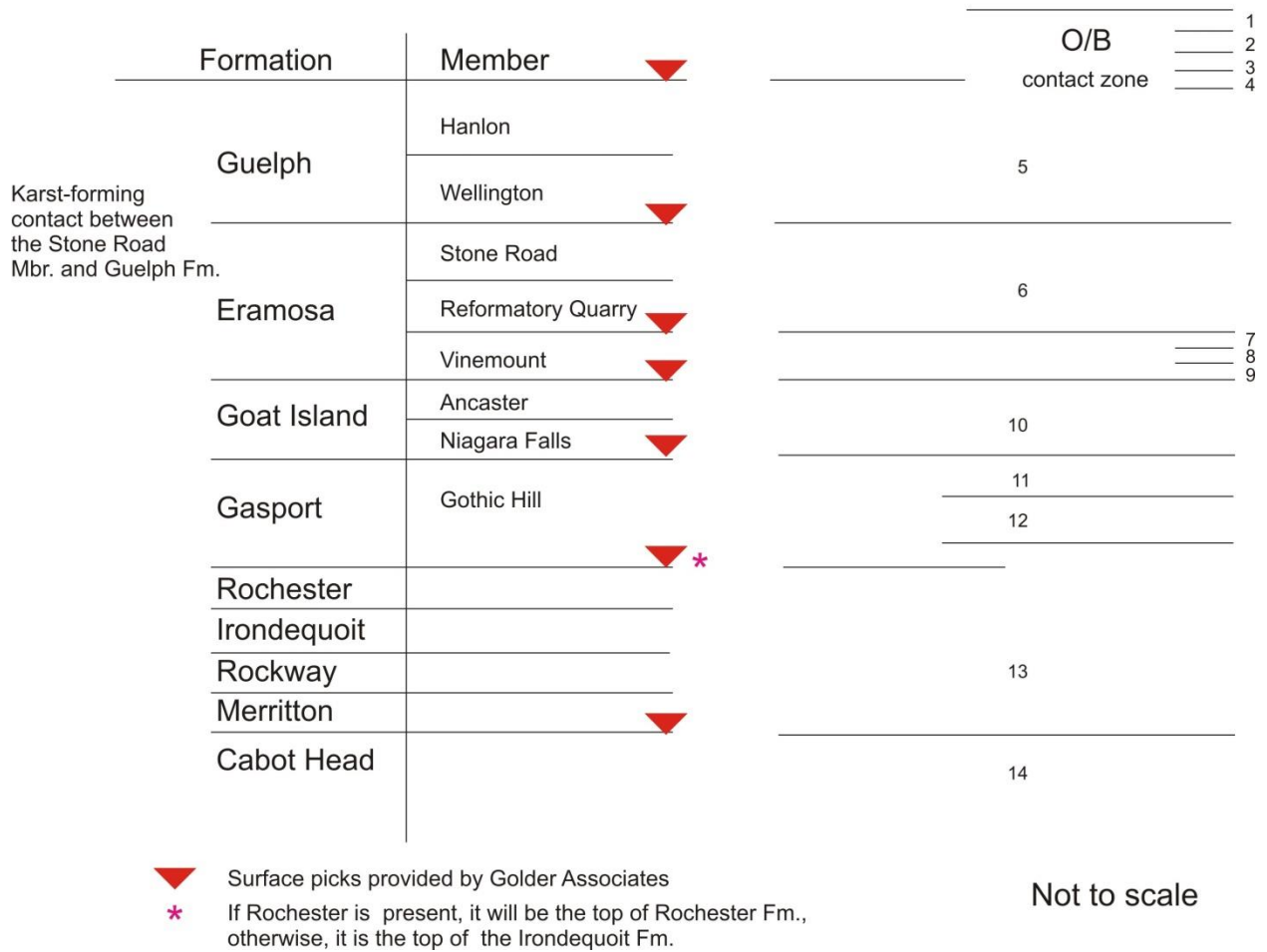


Figure 2-3 Brunton (2008, 2009) stratigraphic units and Guelph Tier 3 groundwater model layers

Brunton (2008, 2009)

Cambridge model



Figure 2-4 Brunton (2008, 2009) stratigraphic units and Cambridge groundwater model layers

Section 3

General approach for developing the model surfaces

The analysis of groundwater flow in the Cambridge area extends from the ground surface to the top of the Cabot Head Formation. The Cabot Head Formation is a shale unit that has relatively low hydraulic conductivity and in previous regional studies has been treated as the lower limit of significant regional groundwater flow (Golder, 1991; Beak et al., 1995; Lotowater, 1997; Duke, 1998; CRA, 2008; Stantec, 2011a; Stantec, 2011b; Golder, 2009b; Golder, 2009c). As discussed in Section 11, the available estimates of the transmissivity of the Cabot Head Formation are consistently relatively small such that neglecting flow in this unit is not likely to be a significant source of error (Clark et al., 1989; Beak et al., 1995; CRA, 2000; CRA, 2008; SSP&A, 2009).

The following surfaces were considered key for the development of the groundwater model:

- Ground surface;
- Top of bedrock (top of the Guelph Formation);
- Top of the Reformatory Quarry Member of the Eramosa Formation;
- Top of the Vinemount Member of the Eramosa Formation;
- Top of the Goat Island Formation;
- Top of the Gasport Formation; and
- Top of the Cabot Head Formation.

Brunton (2008, 2009) identified a third member of the Eramosa Formation in the Guelph area, the Stone Road Member. This unit overlies the Reformatory Quarry Member and is likely not present in the Cambridge area.

With the exception of the top surfaces of the Vinemount Member and the Goat Island Formation, all surfaces are interpolated from the elevations of the units identified primarily by Frank Brunton, referred to here as the “picks” in the borehole logs. For the Vinemount Member and Goat Island Formation, the thicknesses of the layers are interpolated first; the top surfaces are then calculated using the thicknesses and the bottom elevations of the formation. The top surface of the Goat Island Formation is calculated by adding its thickness to the top surface of the Gasport Formation. Similarly, the top surface of the Vinemount Member is calculated by adding its thickness to the top surface of the Goat Island. Checks are incorporated in the development of the surfaces to confirm that the derived elevations match the observed elevations of the surfaces at boreholes.

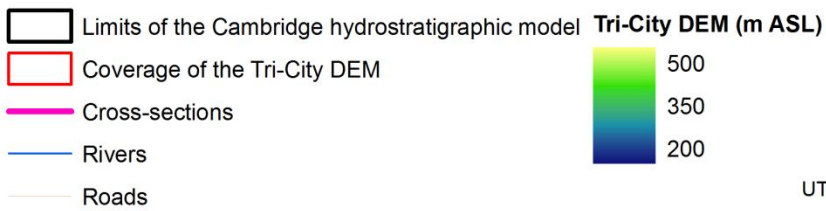
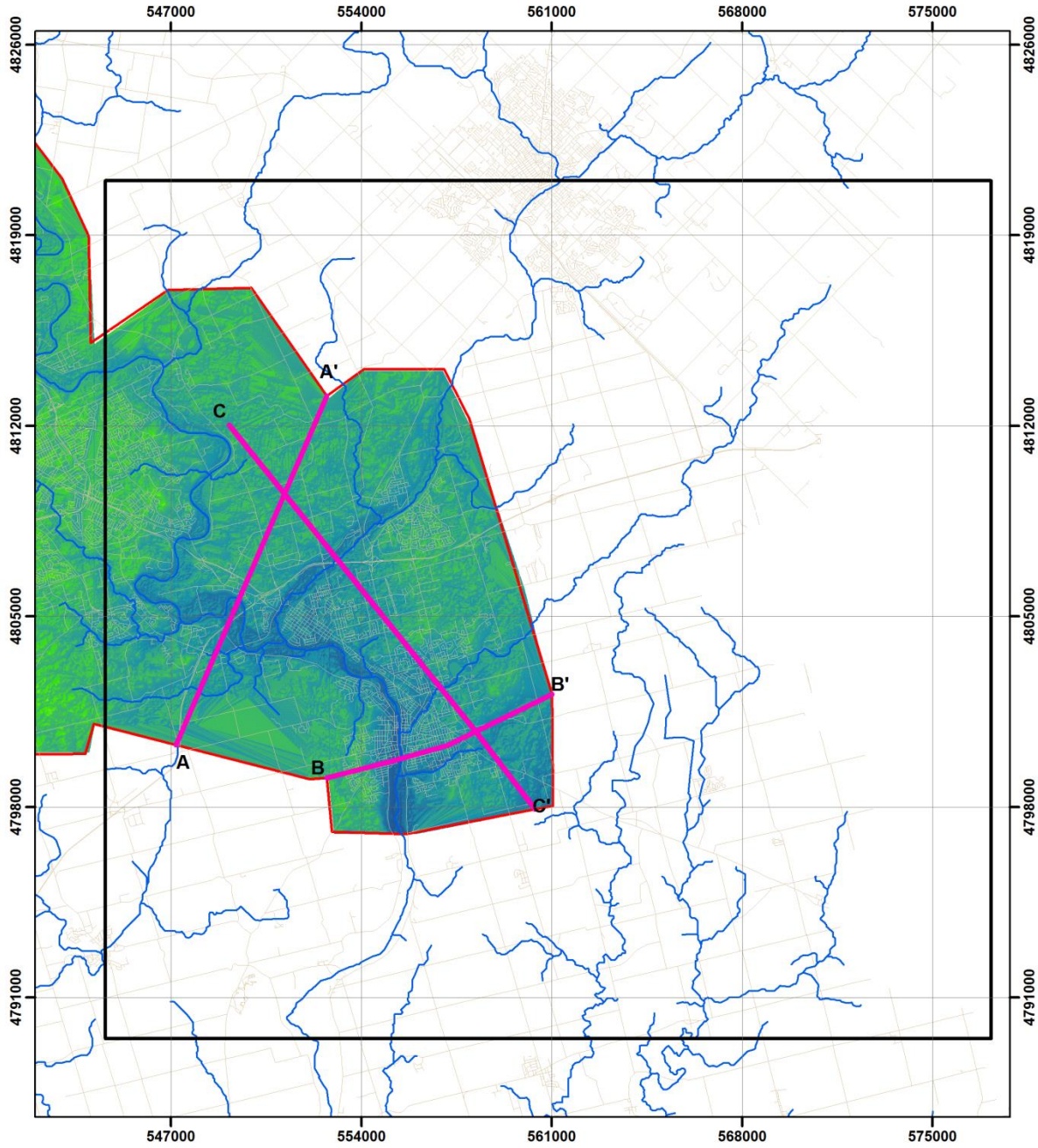
The available information and procedures for generating each surface are described in detail in the following sections.

Section 4

Ground surface elevations

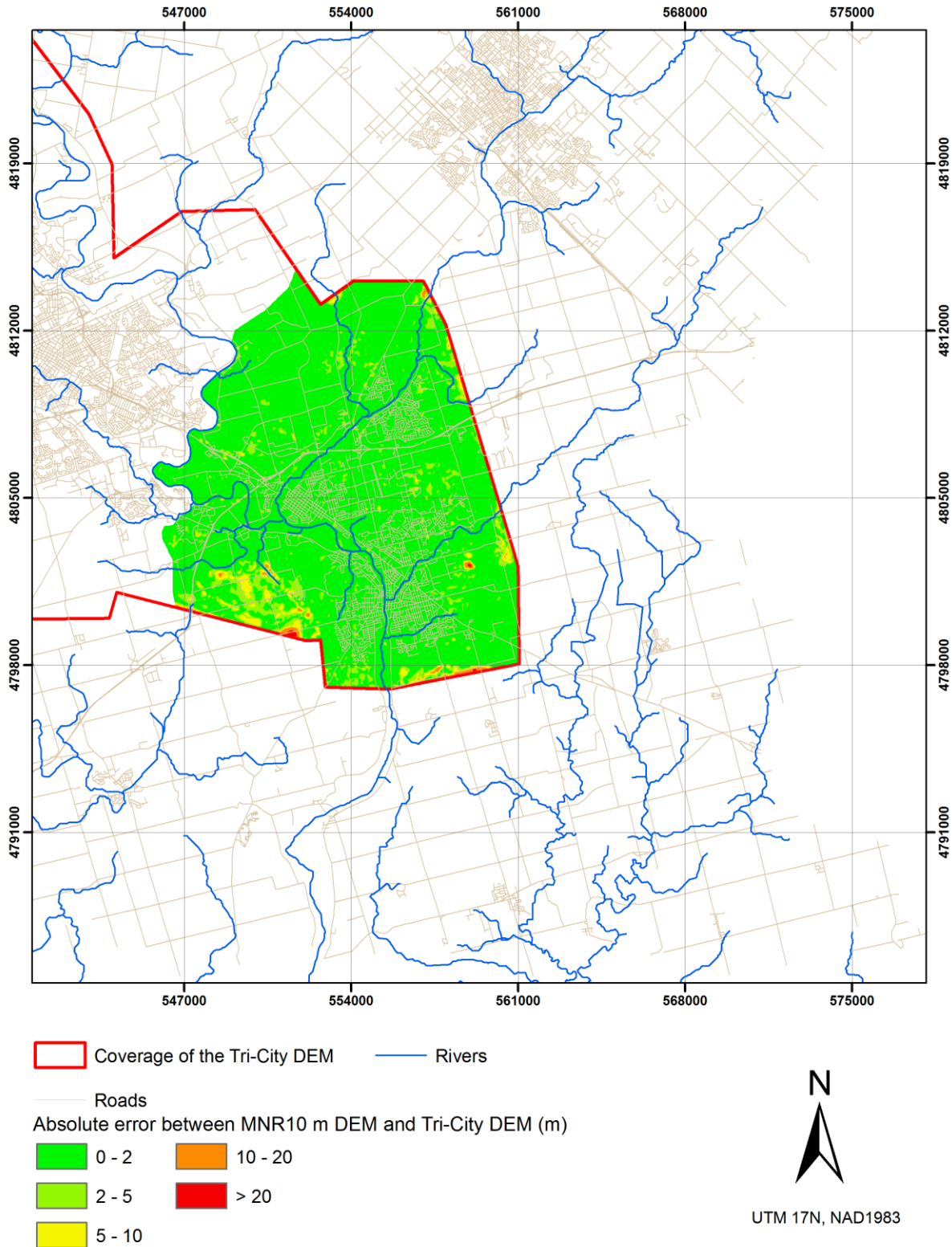
Ground surface elevations for the model are interpolated from the Tri-City Digital Elevation Model (DEM) and MNR 10 m DEM version 2 (MNR, 2005). AquaResource Inc. supplied both DEMs for this analysis. As shown in **Figure 4-1**, the Tri-City DEM has a high resolution (1 m), but does not cover the full model area. In contrast, the MNR 10 m DEM has a coarser resolution but covers the entire model area. The interpolation is conducted so that elevations from the Tri-City DEM are used where they are available, with elevations for the remainder of the area taken from the MNR DEM. To assess the consistency between two DEMs, mapped elevations are compared across the model area. The absolute elevation differences between the Tri-City DEM and the MNR 10 m DEM are presented in **Figure 4-2**. In general, the elevations from the two DEMs are consistent. Over most of the area of overlap, the differences in the elevations are less than 2 m. Large differences are generally limited to the edges of the area of the Tri-City DEM. Close examination of the edges of the Tri-City DEM revealed that the elevations near the edge were not smooth, and edge effects of the data were clearly visible.

The locations of cross-sections along which the MNR 10 m and Tri-City DEMs are compared are also shown in **Figure 4-1**. Profiles along the sections are shown in **Figure 4-3** to **Figure 4-5**. In general, the elevations along the cross-sections are consistent; differences are typically less than 2 m. However, differences in excess of 20 m are observed at the beginning of cross-section A-A'; this is likely caused by errors at the edge of the Tri-City DEM. The interpolation has been repeated excluding the area of the Tri-City DEM coverage where the edge effects are observed. The absolute differences between the two DEMs for after re-interpolation are shown in **Figure 4-6**. The results shown in **Figure 4-6** still suggest that relatively large differences exist around the Cambridge East Landfill. Review of the original DEM data suggests that the Tri-City DEM elevations are about 10-20 m higher than the MNR 10 m DEM in this area. The elevations from the Tri-City DEM have been used in this area; further investigation may be required to confirm the reliability of these elevations. The final interpolated ground surface elevations across the model are presented in **Figure 4-7**.



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Figure 4-1 Coverage of the Tri-City DEM



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Figure 4-2 Absolute elevation differences between the Tri-City and MNR 10 m DEMs

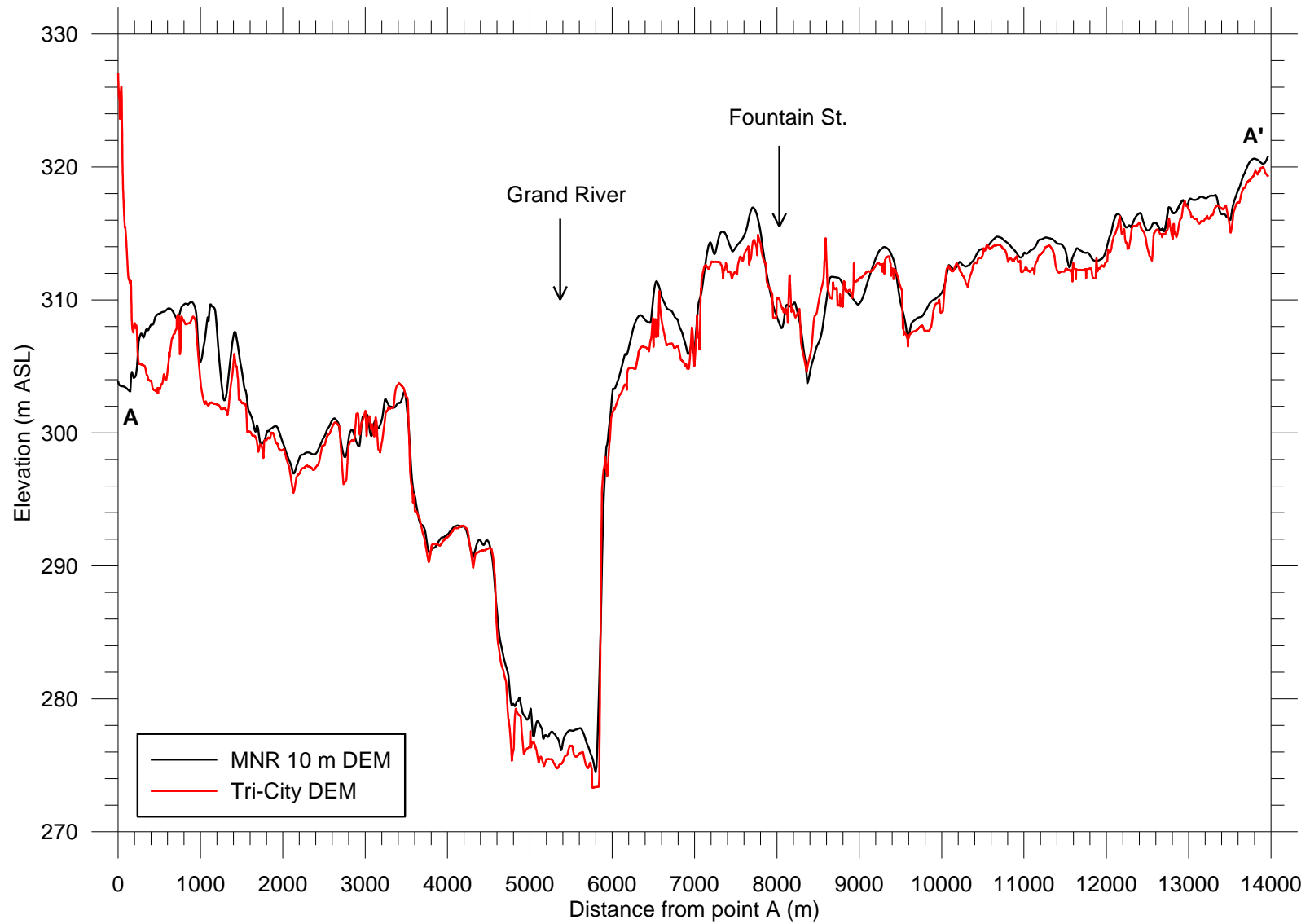


Figure 4-3 Profile of ground surface along line A-A'

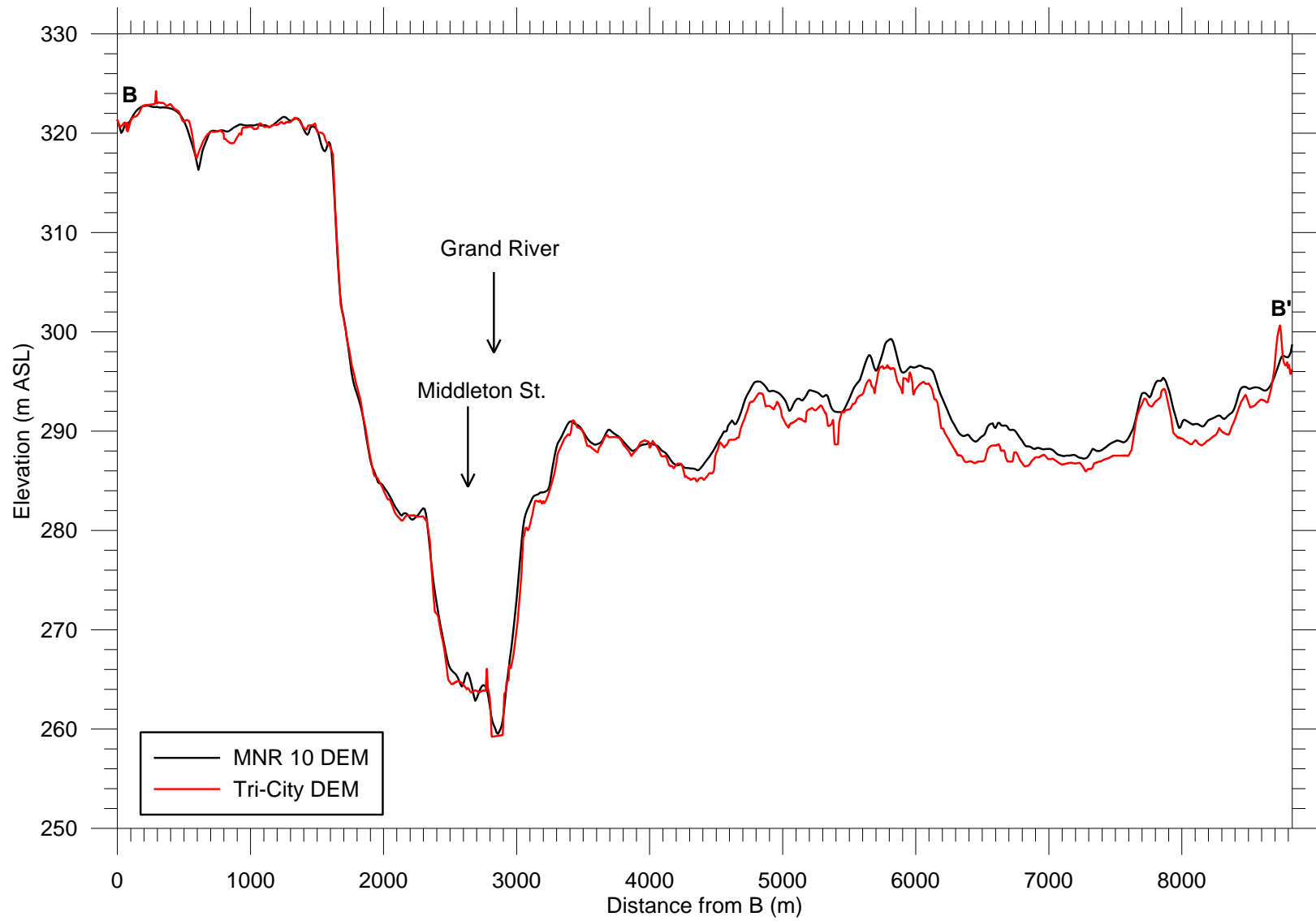


Figure 4-4 Profile of ground surface along line B-B'

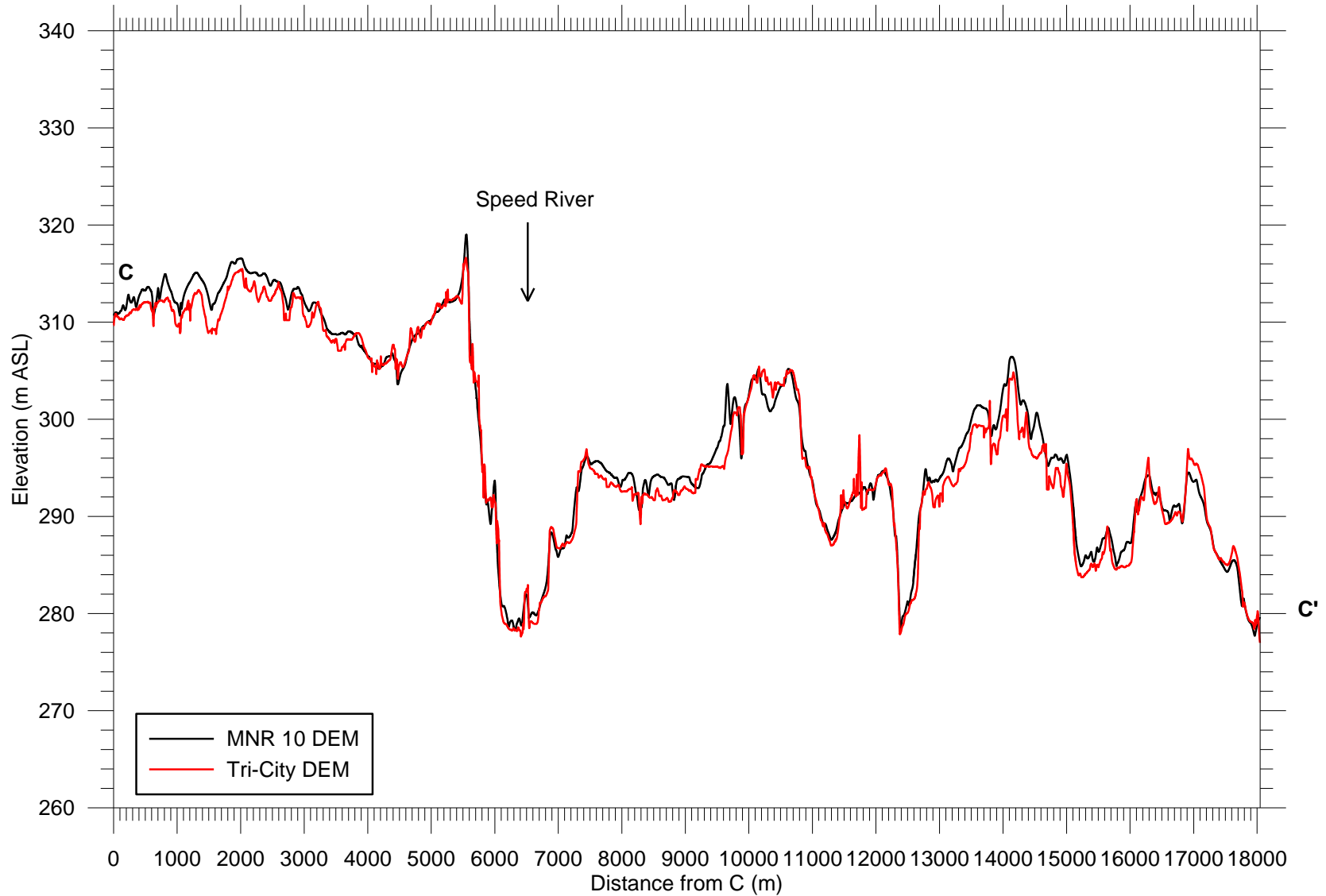
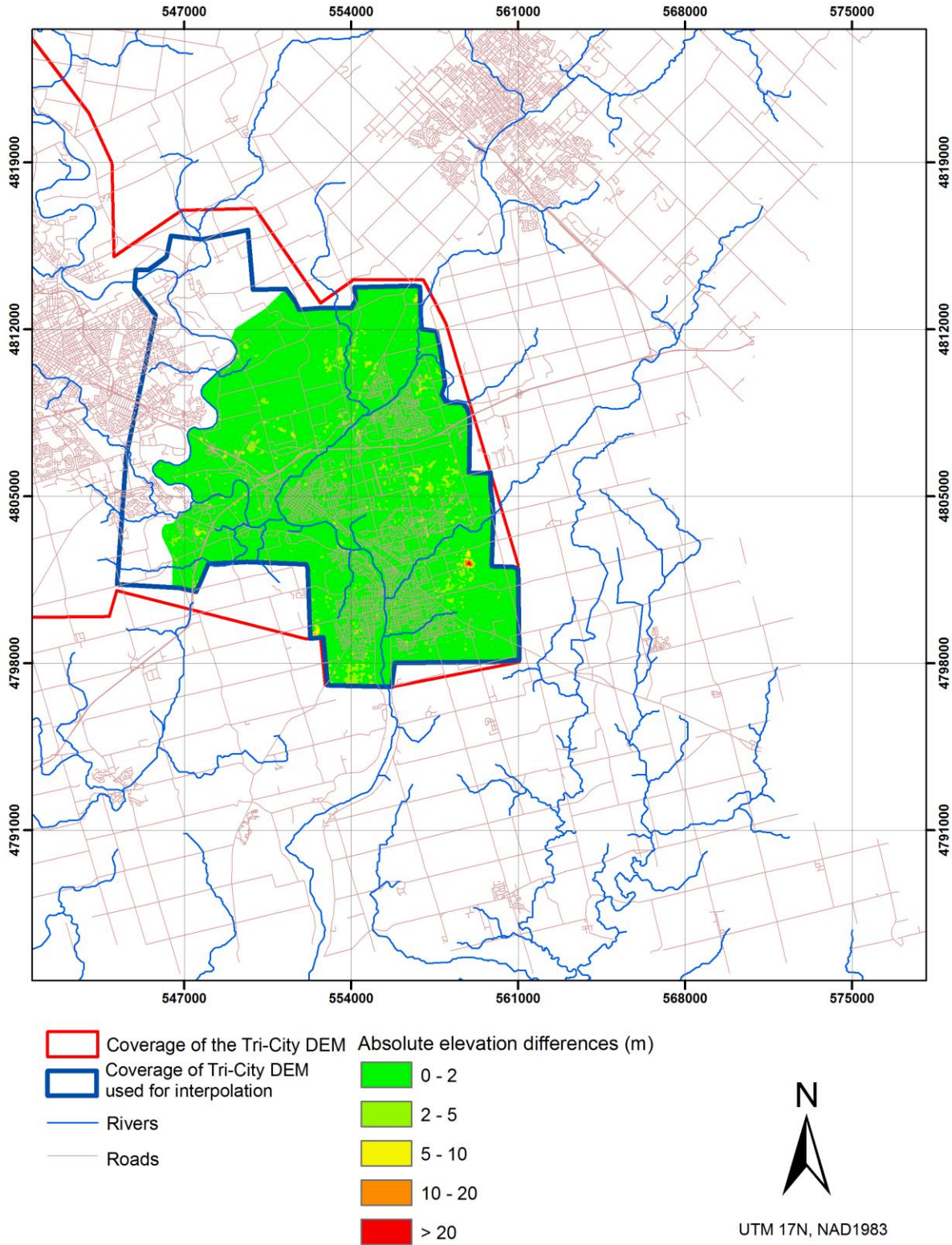
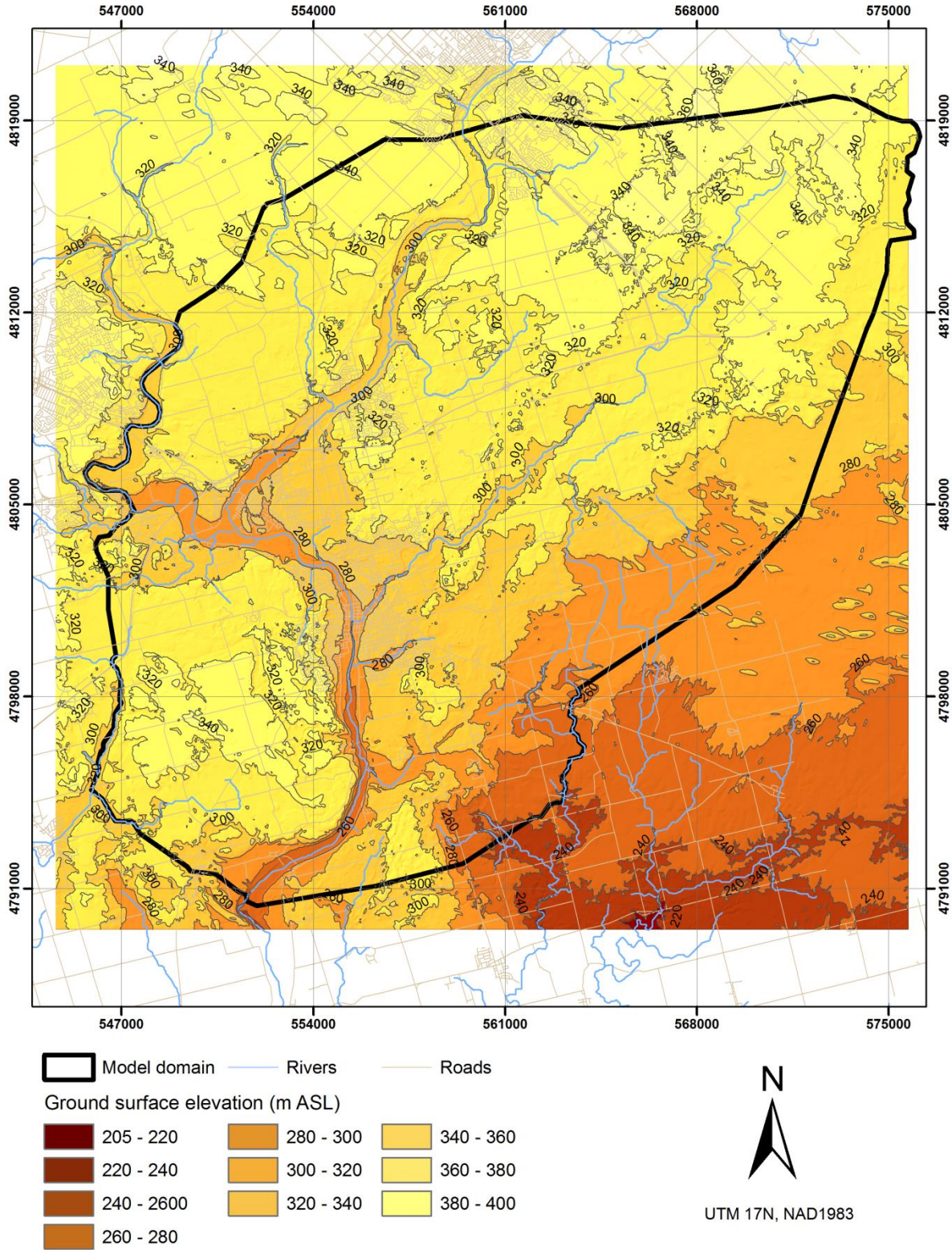


Figure 4-5 Profile of ground surface along line C-C'



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Figure 4-6 Absolute differences in elevations between the Tri-City and MNR 10 m DEM after re-interpolation



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Figure 4-7 Final interpolated ground surface elevations

Section 5

Overburden model layers

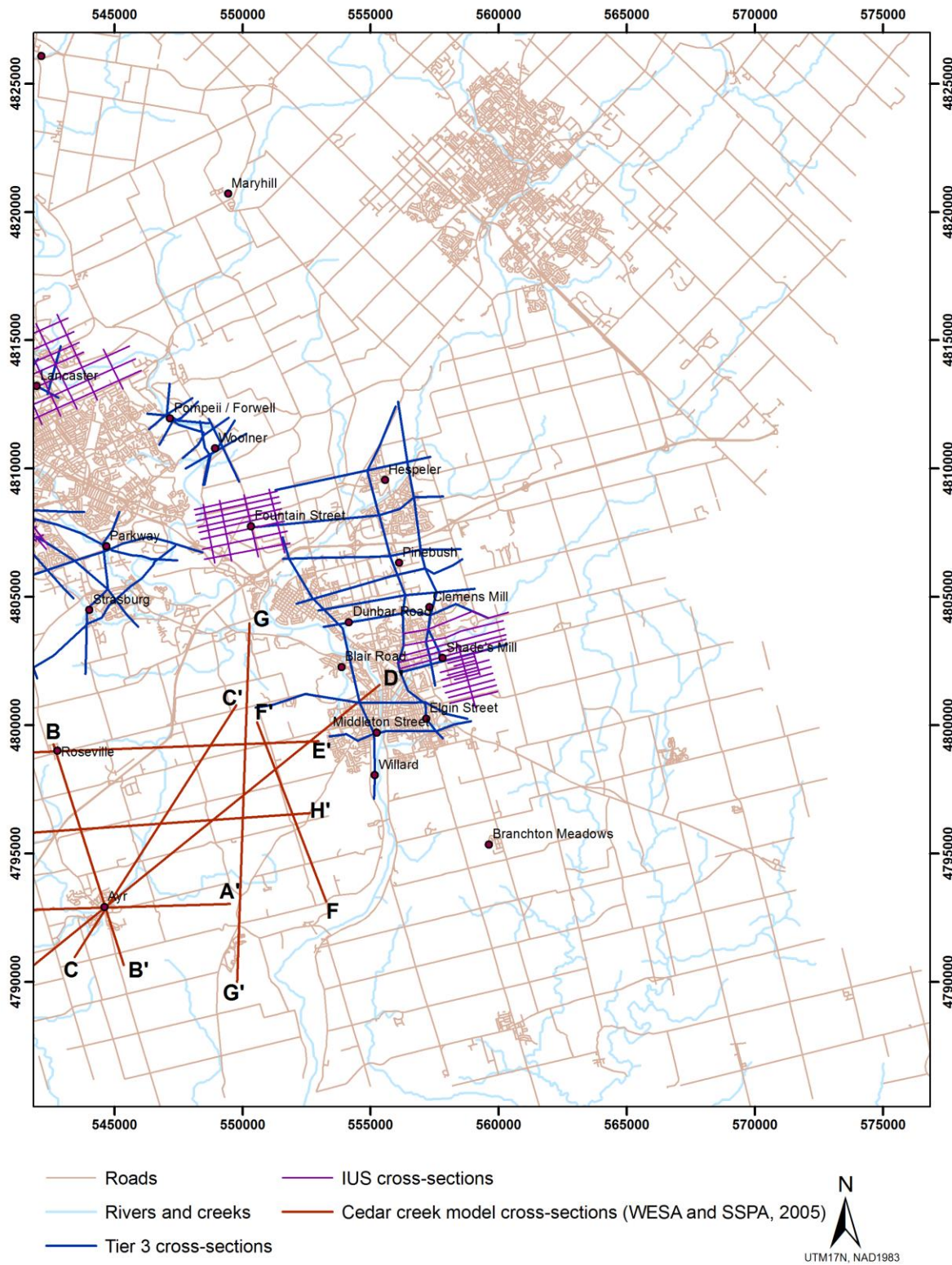
The bulk of existing and planned groundwater supplies in the Cambridge area are obtained from wells that are open across the bedrock. Therefore, in contrast to Waterloo and Kitchener, the focus of the Cambridge analyses is the bedrock rather than the overburden. The Ontario Geological Survey geological model of the overburden includes 19 units for the area (Bajc and Shirota, 2007). The OGS geologic model for the overburden does not extend over the full extents of the study area. Furthermore, not all of these units are present or significant in the Cambridge model area. Golder (2011a) indicates that among the 19 overburden units, only seven are significant in the Cambridge area. These seven units are: ATA2, AFA2, ATB1, AFB1, ATB3, ATC1 and AFD1. For the purposes of supporting a tractable numerical groundwater model, some of these units are lumped based on their lithologic and hydrogeologic characteristics. Overburden units ATA2, AFA2, ATB1 and AFB1 are combined in model layer 1. Overburden units ATB3 and ATC1 are combined in model layer 2. Model layer 3 represents AFD1, which is present only in the vicinity of P16.

Due to the discontinuity of the overburden units and the lack of information over the whole model area, the model layer surfaces for the overburden are interpolated using cross-sections based on borehole picks where detailed information is available. For the remainder of the model domain, the surfaces from the OGS overburden model are used where they are available. Beyond the limits of the OGS overburden model, the thickness of the overburden is subdivided into three layers of equal thickness.

The data considered in the development of the overburden model layers are extracted from cross-sections developed from the OGS overburden model, Tier 3 studies, IUS studies, and the Cedar Creek groundwater model (WESA and SSPA, 2005). The locations of the cross-sections are shown in **Figure 5-1**. The sources of the sections are listed on **Table 5-1**. A detailed description of the development of the overburden layers in the Cambridge area is presented in AquaResource (2012).

Table 5-1 Sources of overburden cross-sections

Category	Source	Detail data	Format
Tier 3 study	Cambridge east well field (Golder, 2011a)	12 cross-sections	PDF Figures
	Cambridge NW well field (Stantec, 2011a)	4 cross-sections	PDF Figures
	Cambridge SW well field (Stantec, 2011b)	3 cross-sections	PDF Figures
	Parkway and Strasburg well field (Stantec, 2012)		PDF figures
	Fountain street well field (Golder, 2009d)	1 cross-section	PDF Figures
IUS study	P16 area (Golder, 2009a)	12 cross-sections	PDF figures and Excel picks
	Cambridge East landfill (Golder, 2008)	18 cross-sections	PDF and Excel picks
Others	Cedar Creek model (WESA and SSPA, 2005)	8 cross-sections	Cross-sections
	OGS overburden model (Bajc and Shirota, 2007)		Electronic surfaces



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Figure 5-1 Locations of overburden cross-sections

Section 6

Bedrock topography, overburden thicknesses and the Contact Aquifer

6.1 Data sources

The following data were used to construct the bedrock surface for the Cambridge model:

- OGS bedrock picks (Gao et al., 2006);
- WRAS+ database (version 2, supplied by AquaResource);
- Data points from the Dundas Valley study (AquaResource, 2007);
- Bedrock picks for Tier 3 study (supplied by Golder);
- Bedrock picks for Tier 3 study (supplied by Stantec);
- Cross-sections from the Cedar Creek groundwater model (WESA and SSPA, 2007); and
- Bajc 2008 picks data (supplied by AquaResource)

Maps of the surficial geology also showed that bedrock outcrops over some portions of the model area (Karrow, 1987; Ontario Geology Survey, 2003). Control points were added in these areas with the elevations of the bedrock surface set equal to ground surface.

The surface of the bedrock was developed through multiple iterations. For example, the surface was developed initially using dataset that excludes the data points from Bajc and Shirota (2007), and the Bajc and Shirota data were used for quality checking. During the quality checking, it was found that Bajc and Shirota (2007) contained additional data that are critical to the development of the bedrock surface. The bedrock surface was re-developed by including these additional data.

6.2 Data organization and quality control

All data sets were checked and sorted, with duplicate data points removed based on their reliability ranking. The reliability of the data were ranked from high to low in the following order: Stantec's picks, Golder Associates' picks, picks from Dundas Valley study, Bajc's 2008 picks, OGS bedrock picks, digitized points from the Cedar Creek study, and the WRAS+ database.

It is important to note that the labels “Stantec’s picks” and “Golder’s picks” are used here only the firm that provided the data to us. In reality, Frank Brunton of the Ontario Geological Survey made almost all of the picks. Stantec's picks were considered slightly more reliable than the Golder picks because they were received at a later date. There are some duplicate picks between Stantec and Golder’s compilations. In general, the picks at the same locations are consistent. However, there are a few locations where the picks differ. John Piersol, Golder Associates, indicated in a telephone conversation that Stantec's picks should be considered more reliable at these locations, since Stantec were responsible for the drilling and logging of these wells.

The reason why a higher ranking was given to OGS picks over the WRAS+ database is that it was found during the processing that many WRAS+ entries were not consistent with other data sources; differences of over 50 m have been observed with other data sources that are internally consistent. At some locations, it was noted that the WRAS+ database indicates bedrock depths that exceed the depth of the boreholes. This ranking order was used only as a general guide; the consistency of the bedrock elevations with the surrounding points is the most important criterion outside the picks from Stantec and Golder Associates. For instance, although the picks for the Dundas Valley study were considered to be generally highly reliable, it was found that several of the Dundas Valley picks have bedrock elevations that are 20 to 30 m higher than their surroundings; these picks have been flagged and excluded from the final dataset.

Extensive efforts were made to remove duplicate and inconsistent data points. Two of the checks conducted are described below.

1. All of the data within a 1 m search radius were checked to identify duplicate points. If the distance between two data points was less than or equal to 1 m, these points were exported to a separate file for further review. The data points in the exported file were checked carefully one-by-one against other nearby data points. For points that had the same or similar bedrock elevations, only one point was retained, and the others were flagged as *duplicate points* and excluded from the final dataset. Points with elevations that were significantly different from surrounding points were flagged with a message of *inconsistent with surroundings* and were excluded from the final data.; and
2. All of the data within a 50 m search radius were checked to identify outliers. If the bedrock elevations at two points within a 50 m distance differed by more than 10 m, both points were exported to a separate file. Points assembled were reviewed carefully and their bedrock elevations were compared with the surroundings. The points with bedrock elevations that were not consistent with the surroundings were flagged and excluded from the final dataset.

After the checking, the following numbers of points were retained in the final dataset for the bedrock surface:

- OGS bedrock picks [Original set: 6,343, retained 4,562];
- WRAS + database [Original set: 3,019, retained 894];
- Data points from the Dundas Valley study [Original set: 324, retained 170];
- Bedrock picks from Golder [Original set: 53, retained 40];
- Bedrock picks from Stantec [Original set: 125, retained 121];
- Digitized cross-section points from Cedar Creek study [Original set: 2,462, retained 2,460]; and
- Bajc's 2008 picks data [Original set: 1,754, retained 1,167];

It was suggested subsequently that data points in the vicinity of P16 and the Cambridge East Landfill be removed and that only the picks compiled during the Golder Associates IUS studies be used. After this step, the numbers of data points retained from each data source were:

- OGS bedrock picks: 4,508 (a further reduction of 54);
- WRAS Plus database: 828 (a further reduction of 66);
- Data points from the Dundas Valley study: 128 (a further reduction of 42);
- Bedrock picks from Golder: 37 (a further reduction of 3);
- Bedrock picks from Stantec: 121 (unchanged);
- Digitized cross-section points from Cedar Creek study: 2,460 (unchanged);
- Bajc 2008 picks data: 1,124 (a further reduction of 44);
- Picks from Cambridge East Landfill IUS study: 65(additions);
- Picks from P16 IUS study: 23 (additions); and
- Data points from AquaResource’s final Cambridge Master Table Picks (supplied on April, 23rd, 2010): 4(additions).

The additional four data points from AquaResource’s final Cambridge Master Table Picks were supplied to SSPA after AquaResource reviewed the new preliminary bedrock surface. Three Golder picks, G5-TW1-08, H4-TW1-08 and H5-TW1-09, were removed based on the suggestion of John Piersol, Golder Associates¹. He indicated that the information given in these wells was not based on surveyed elevations, and were only the top of core provided to Mr. Frank Brunton. Based on his familiarity with the area around P9, P15 and G5, he suggested removing any WRAS and OGS picks with a bedrock elevation exceeding 295 m ASL.

A total of 3,488 data points were also incorporated in the dataset at locations of bedrock outcrops. The metadata for this study includes a listing of all data points, with an indication of which points have been excluded from the final dataset.

6.3 Mill Creek bedrock valley

Another issue that was raised during the March 26, 2010 meeting was the presence of a bedrock valley near Mill Creek. Few high-reliability data points in the current dataset were available to define the extent and depth of this bedrock valley. To investigate further the potential presence of this bedrock valley, several additional interpretations were reviewed (Ontario Department of Mines, 1963; IWS, 1974; Lotowater, 1997; and Bajc and Shirota, 2007).

The Ontario Department of Mines presented a map of the bedrock surface in 1963 (Map M2030), which was cited in the Golder Associates study of Cambridge (Golder, 1991). The relevant section of the map is reproduced in **Figure 6-1**. The bedrock valley was different from the current Mill Creek channel and not connected to the Grand River. The elevation of the deepest part of the valley was about 250 m ASL.

¹ Email communication with John Piersol (Golder Associates) on May 6, 2010.

Bedrock topography maps included in IWS (1974) and Lotowater (1997) suggested a bedrock valley near Mill Creek. Contours of the bedrock elevations, as mapped in IWS (1974) and Lotowater (1997), were digitized and are presented in **Figure 6-2** and **Figure 6-3**, respectively. The units of the original bedrock topography in the IWS (1974) map were in feet; for comparison with other studies, the unit of the bedrock elevations in **Figure 6-2** was converted to metres.

Although there are differences between the contours shown in **Figure 6-2** and **Figure 6-3**, both maps show a continuous bedrock valley from Mill Creek to the Grand River. The depth and width of the valley in the two maps are different. The valley in IWS's map follows the present channel of Mill Creek; in contrast, the Lotowater mapping suggests some offset from the present channel. Very little description was provided in IWS (1974) regarding the development of the bedrock surface. Lotowater (1997) included a gravity survey at Soper Park, which indicated that the depth of the bedrock valley ranges from 10 m to 40 m, and that the valley may split toward the northeast.

The Ontario Geology Survey published a bedrock topography map in 2006 (Gao et al., 2006). As shown in **Figure 6-4**, the bedrock valley was not interpreted to be continuous up to the Grand River. The deepest point of the valley has an elevation of 260 m ASL; however, the bedrock surface rises to an elevation of 270 m between Mill Creek and the Grand River.

A portion of the map of the top of bedrock developed by Bajc and Shirota (2007) is reproduced in **Figure 6-5**. A bedrock valley near Mill Creek was not evident in their interpretation.

The bedrock surface for the present model incorporated the bedrock valley near Mill Creek following the Lotowater (1997) interpretation. The Lotowater (1997) mapping was taken as the most reliable because it incorporated the results of a local gravity survey. The bedrock topography map from the Lotowater report (1997) was digitized, and points near Mill Creek were included in the dataset as control points.

The locations of all control points for the final dataset for the top of bedrock surface are shown in **Figure 6-6**.

6.4 Interpolated bedrock surface and inferred overburden thicknesses

The initial continuous surface of the top of bedrock was developed by interpolation of the control points, using Surfer 9 (Golden Software, 2009). Ordinary Kriging with the default linear variogram was used. The resulting surface of the top of the bedrock exceeded ground surface in some areas, likely reflecting errors introduced by the interpolation around the areas of bedrock outcrop. In these areas, the bedrock surface was revised to match ground surface.

The observed top of bedrock elevations in the boreholes are plotted in **Figure 6-7** against the interpolated elevations at the same locations. The comparison indicated that the interpolated elevations were generally consistent with the elevations of the control points. Discrepancies between the elevations of the control points and the interpolated elevations were identified at some locations; these points generally coincided with the locations of the OGS picks or WRAS+ points. The largest discrepancies occurred between points that are in close proximity; at these locations the OGS and WRAS+ elevations were not consistent with other data sources. A significant effort was devoted to removing unreliable points. A cumulative probability plot of the differences between the elevation of the control points and the interpolated elevation is shown in **Figure 6-8**. Over 98% of the points differed by less than 2 m and the mean and mean absolute differences were 0.03 m and 0.32 m, respectively. The differences at about 70% of the locations approximated a normal distribution.

The final interpolated bedrock surface is presented in **Figure 6-9**. The elevations of the bedrock surface ranged from 220 to 330 m ASL across the model domain. The bedrock valley near Mill Creek is clearly visible in a more detailed view of the bedrock surface near the area in **Figure 6-10**.

The overburden thicknesses were calculated by subtracting the continuous bedrock surface from the ground surface. After the first pass, negative thicknesses were calculated at some locations. Close inspection revealed that the cause of the negative thicknesses was due either to artefacts of the interpolation or inconsistent data. In general, the negative thickness values corresponded to locations in the WRAS+ database or to errors at bedrock outcrops.

One example of inconsistent data is described here. At borehole MI-OW1-92, Golder Associates reported a bedrock elevation of 287.9 m ASL; in contrast, Stantec reported an elevation of 263.2 m ASL. The ground surface elevation of the DEM at this point was only 264.5 m ASL. Golder Associates reported a bedrock elevation of 263 m ASL at borehole BH1-93, located about 150 m northwest of MI-OW1-92. The Stantec value seemed more reliable since it was consistent with both the ground surface elevation and surrounding boreholes. The interpolation was repeated using the value reported by Stantec. During the second pass, areas with problematic top of bedrock elevations were reviewed. Potentially unreliable elevations were flagged, and the thickness of the overburden was re-calculated. The bedrock surface was re-calculated by subtracting the overburden thicknesses from the ground surface elevations. The bedrock surface shown in **Figure 6-9** reflects these revisions. The final interpreted overburden thicknesses are presented in **Figure 6-11**.

As shown in **Figure 6-11**, the interpreted overburden thicknesses are consistent with the interpreted areas of bedrock outcrops presented in Karrow (1987) and OGS (2003). The observed and interpreted overburden thicknesses at the locations of boreholes are compared in **Figure 6-12**. For this comparison, only the high-reliability picks from Stantec and Golder Associates were used. In general, the absolute differences between the observed and interpreted overburden thickness were less than 2 m.

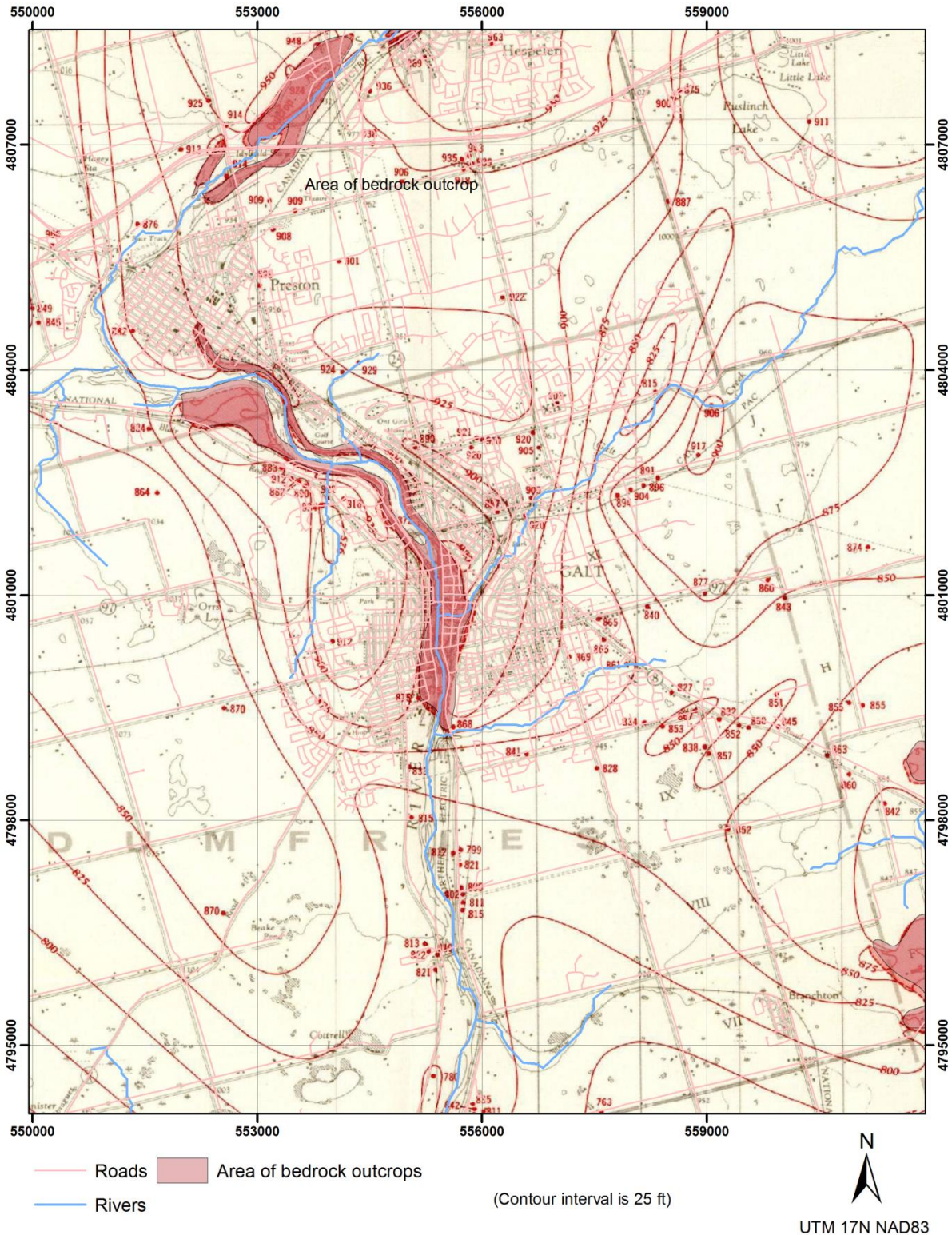
More significant differences were evident at 7 wells: OW7—95, TW15—60, SMTW4—08, TW2-60, OW10-95, TW1-55 and TW15A/80. The elevations of ground surface and bedrock surface from the high-reliability picks, and SSPA interpolated surfaces for these 7 boreholes, are listed on **Table 6-1**. Data on **Table 6-1** show that the interpolated bedrock surface closely approximates the bedrock elevations based on the picks at all 7 wells. However, the ground surface elevations for the picks differed from the ground surface elevations from the DEM. Comparing the last two columns on **Table 6-1**, it appears that much of the differences in the interpreted overburden thickness can be attributed to the differences in the estimated ground surface elevations. The DEM effectively synthesized information on ground surface elevations over the full extent of the study area. Similarly, the interpolated bedrock surface extended over the full area, and was consistent with the elevations picked from the 7 apparent outliers. Therefore, we retained the interpreted overburden thicknesses at these locations. These points are evident in **Figure 6-13**. As shown in **Figure 6-13**, the cumulative probability distribution of the differences between observed and interpolated overburden thickness for most data points approximates a straight line.

6.5 Contact Aquifer

To provide flexibility in the representation of conditions at the interface between the overburden and the upper bedrock, a distinct hydrostratigraphic unit designated the Contact Aquifer was incorporated in the analysis. The Contact Aquifer was represented with a single model layer. The Contact Aquifer was assigned a uniform thickness of 2 m, corresponding to 1 m above and below the top of bedrock surface. In areas where the bedrock outcropped, the full 2 m thickness was subtracted from the upper bedrock units.

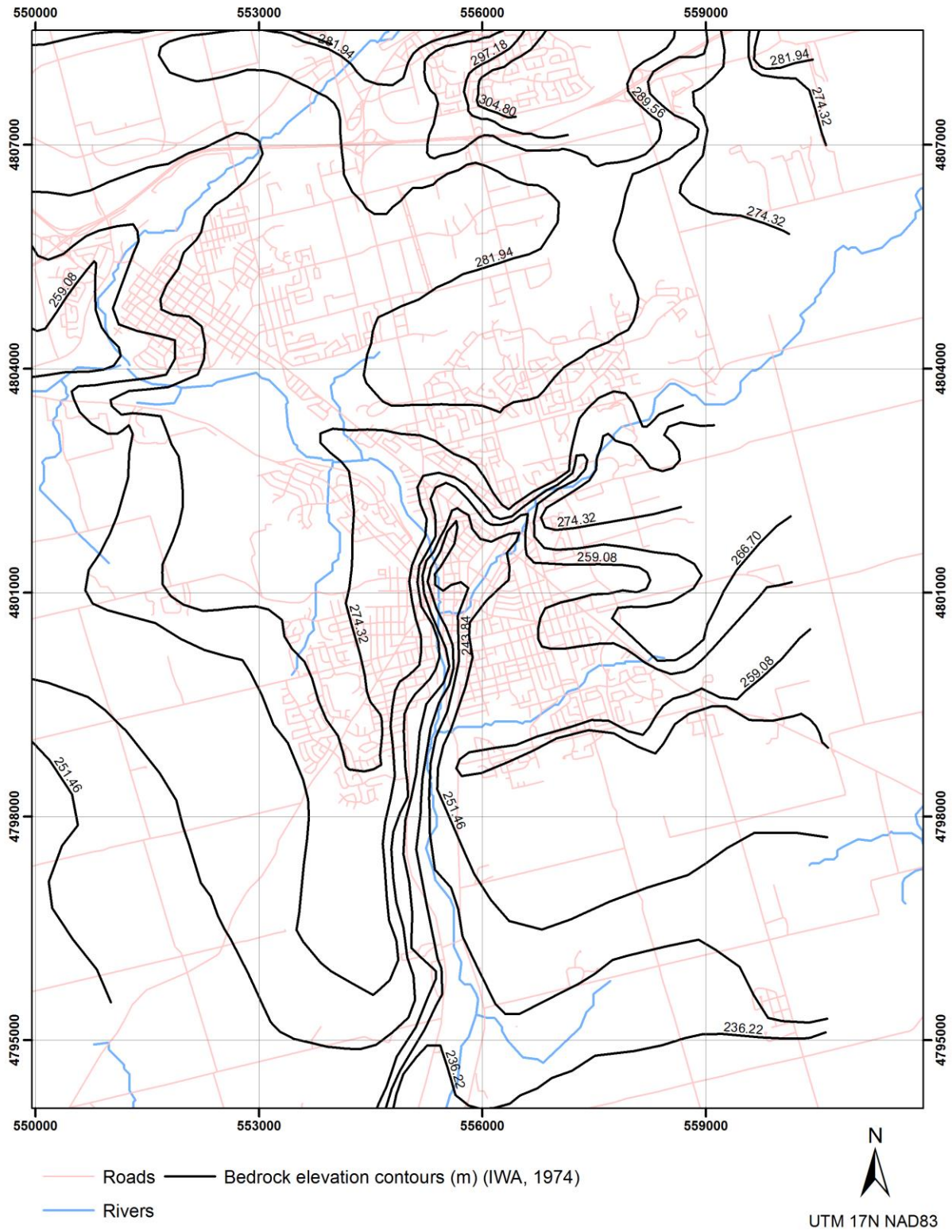
Table 6-1 Bedrock surface elevations at 7 apparent outlier wells

Borehole name	Ground elevation from picks (m ASL)	Bedrock elevation from picks (m ASL)	Overburden thickness from picks (m ASL)	Ground elevation from DEM (m ASL)	Interpolated bedrock elevation (m ASL)	Interpreted overburden thickness (m)	Difference in interpreted overburden thickness (m)	Difference in interpreted ground surface elevation (m)
OW10-95	319.00	280.00	39.00	323.16	280.87	42.29	-3.29	-4.16
OW7-95	277.30	266.61	10.69	290.06	266.12	23.94	-13.25	-12.76
SMTW4-08	320.90	277.92	42.98	296.66	277.28	19.38	23.60	24.24
TW1-55	290.00	277.81	12.19	288.32	278.67	9.65	2.54	1.68
TW15A/80	311.0	291.50	19.50	308.55	291.52	17.03	2.47	2.45
TW15-60	296.79	258.68	38.11	291.33	259.56	31.77	6.34	5.46
TW2-69	309.38	257.87	51.51	306.01	257.87	48.14	3.37	3.37



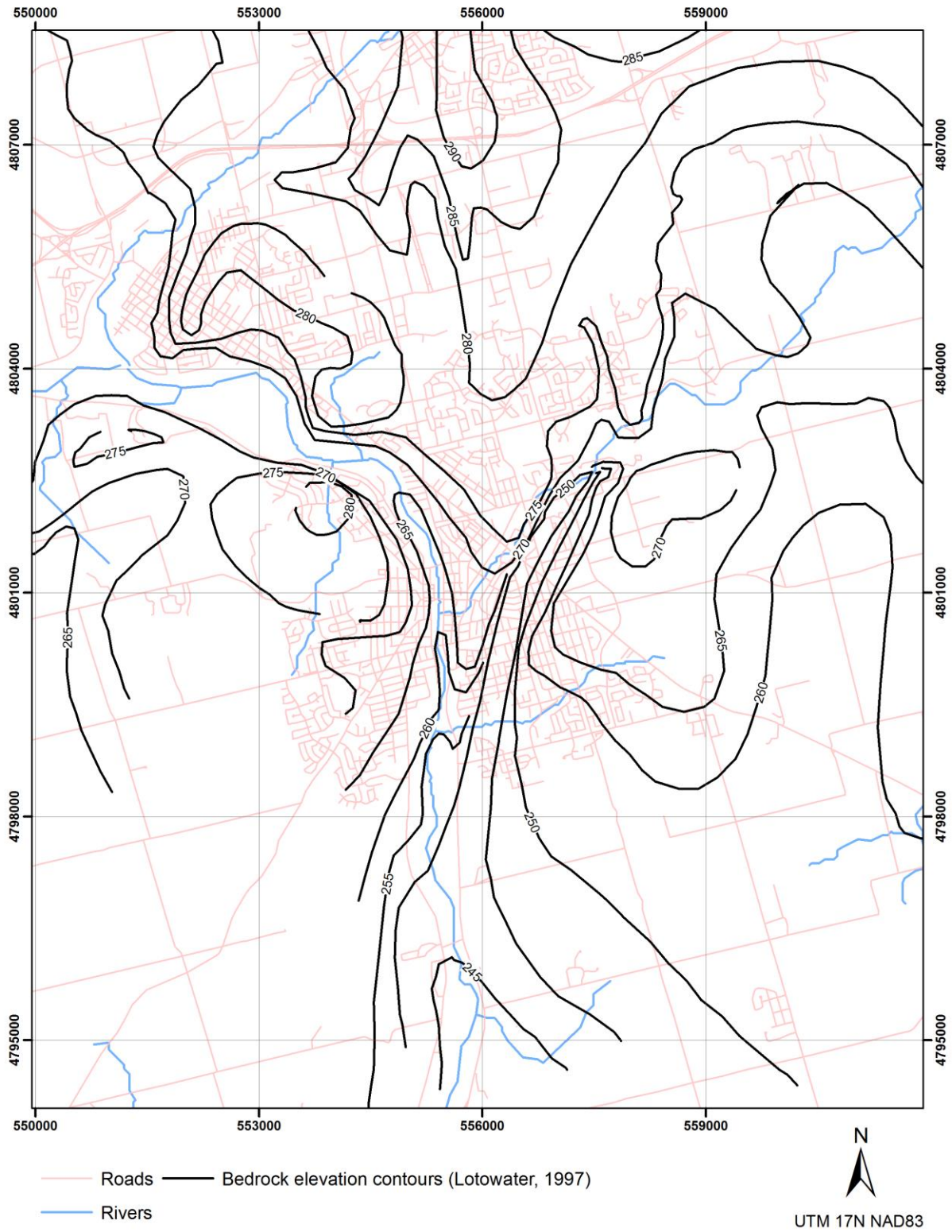
D:\Projects\SSP1234 Cambridge model\data\Classified data\Maps\Overburden_thickness\Bedrock top surface_M2030_detail.mxd

Figure 6-1 Bedrock topography (Department of Mines, Ontario, 1963, Map M2030)



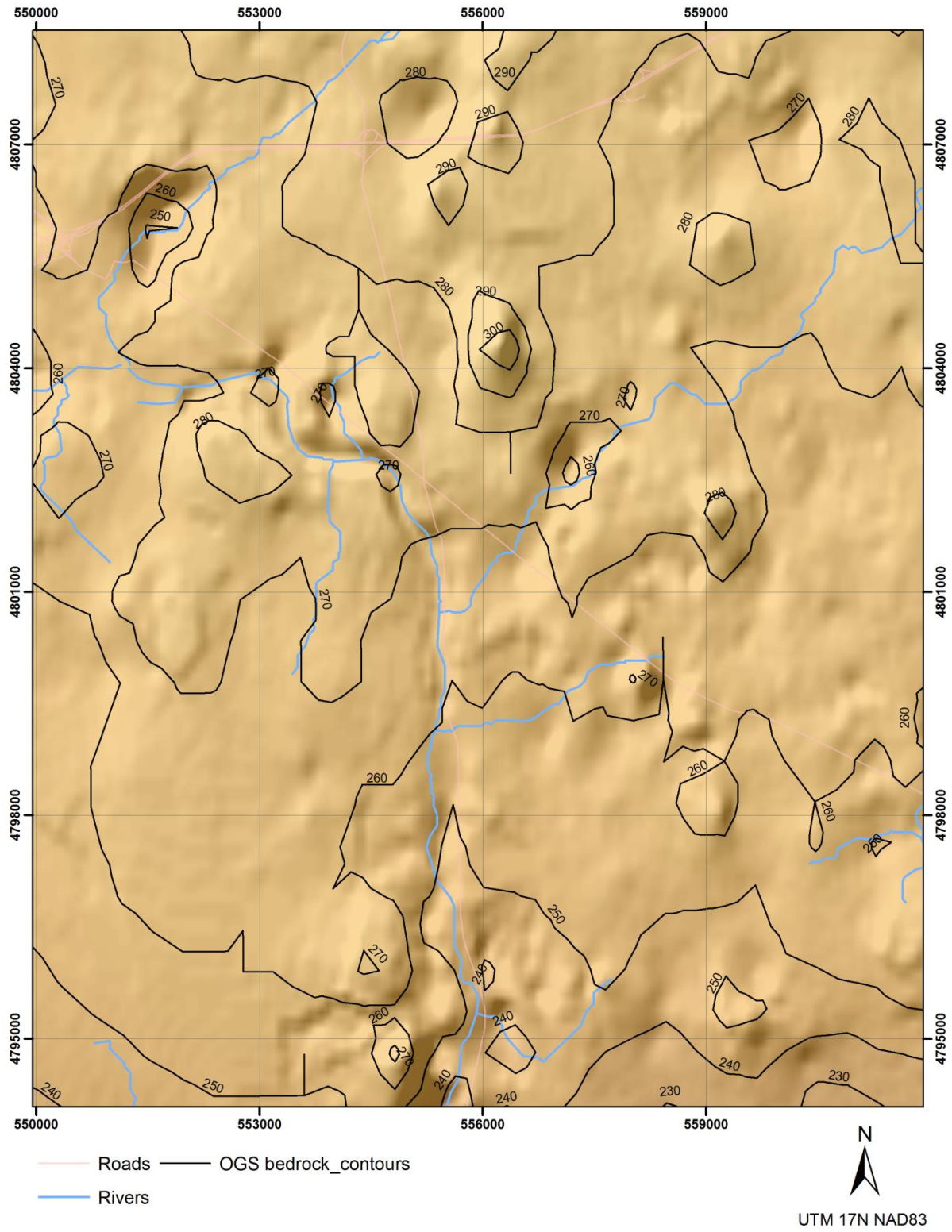
D:\Projects\SSP1234 Cambridge model\data\Classified data\Geology\Bedrock depth\April-09-2010\IWA bedrock\IWA_bedrock_top_contour_meters_detail.mxd

Figure 6-2 Bedrock topography (IWS, 1974)



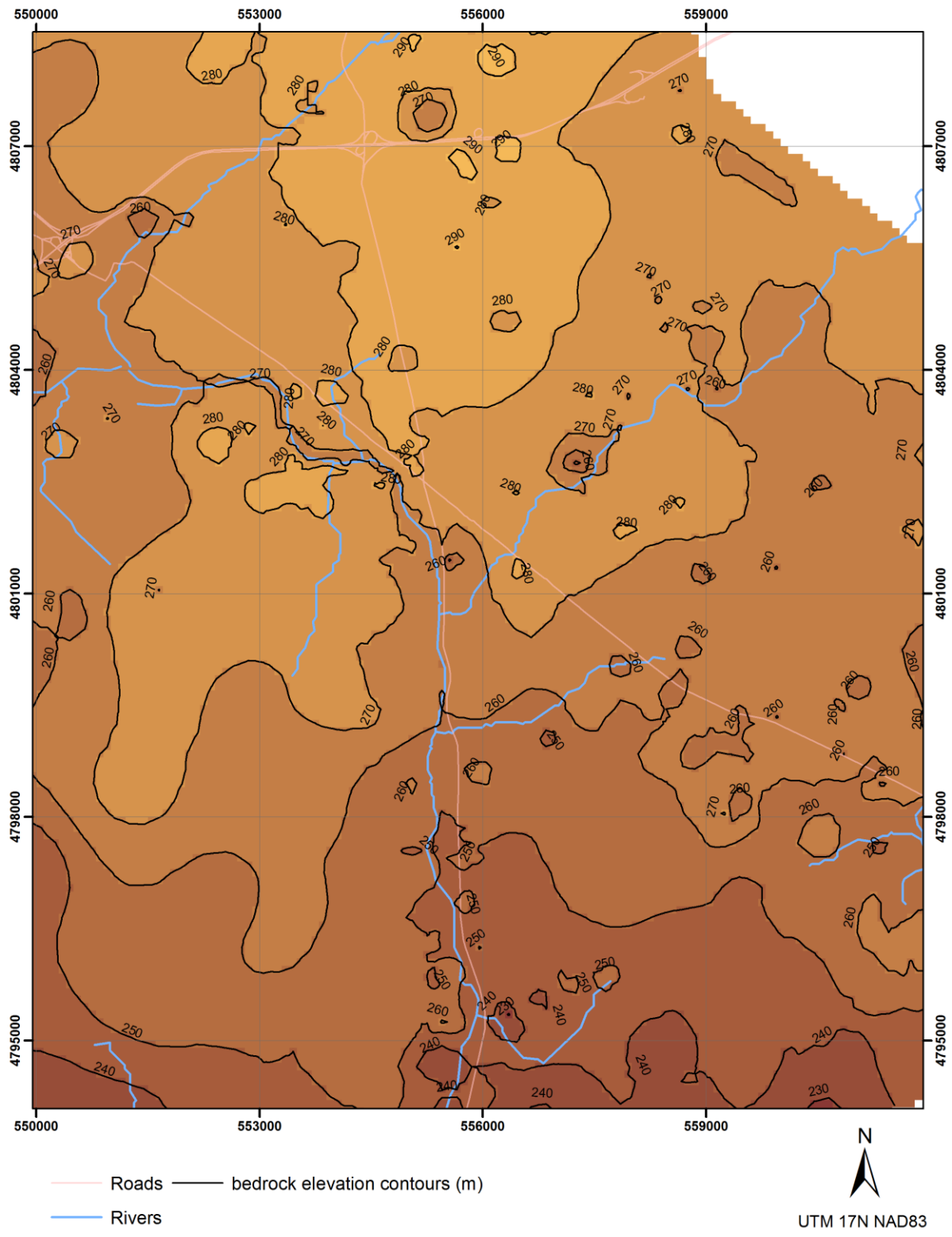
D:\Projects\SSP1234 Cambridge model\data\Classified data\Geology\Bedrock depth\April-09-2010\Lotowater_Bedrock\Lotowater_bedrock_top_contour_detail.mxd

Figure 6-3 Bedrock topography (Lotowater, 1997)



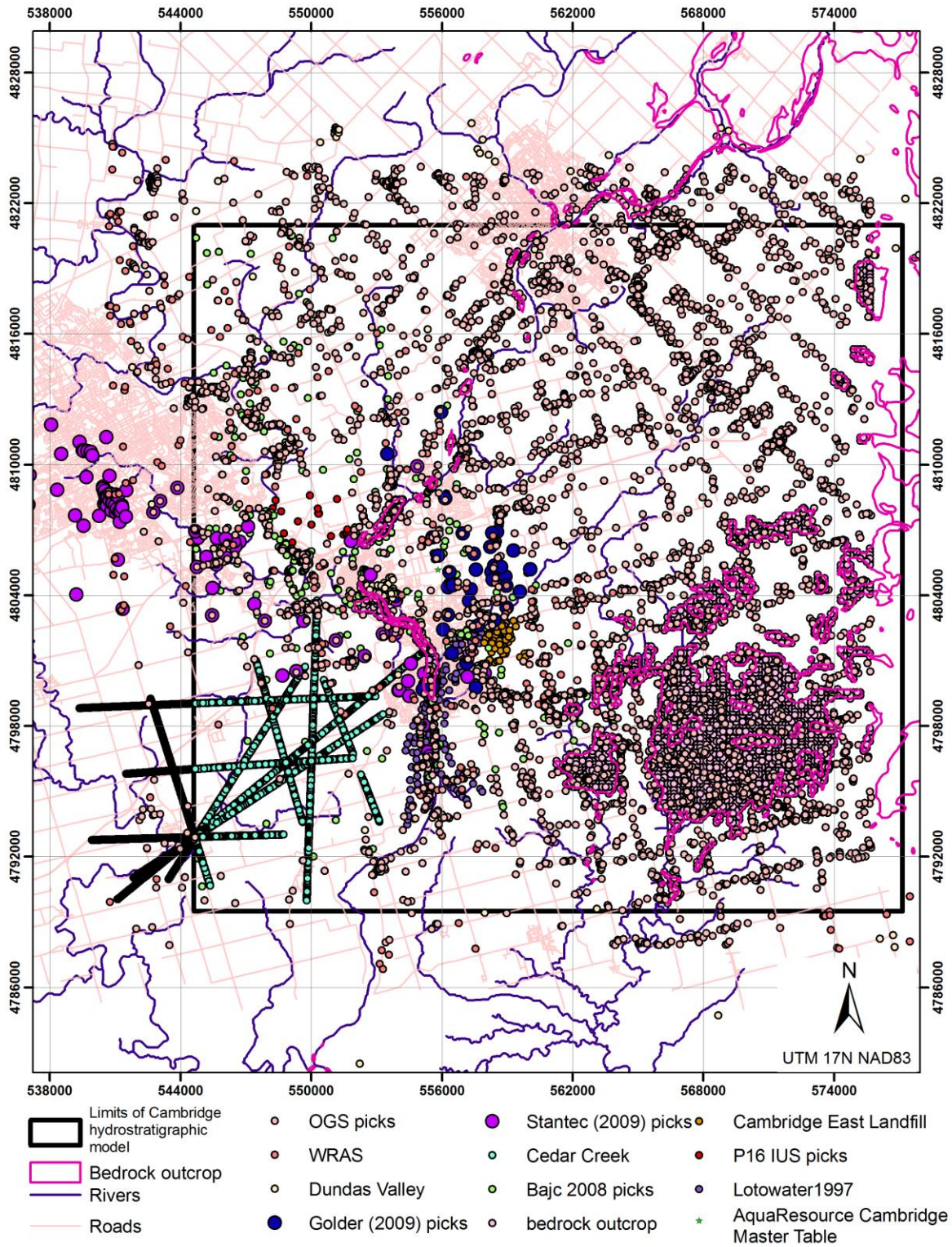
D:\Projects\SSP1234 Cambridge model\data\Classified data\Geology\Bedrock depth\OGS top of bedrock\OGS bedrock surface_detail.mxd

Figure 6-4 Bedrock topography (Gao et al., 2006)



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Figure 6-5 Bedrock topography (Bajc and Shirota, 2007)



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Figure 6-6 Locations of the top of bedrock control points in the final dataset

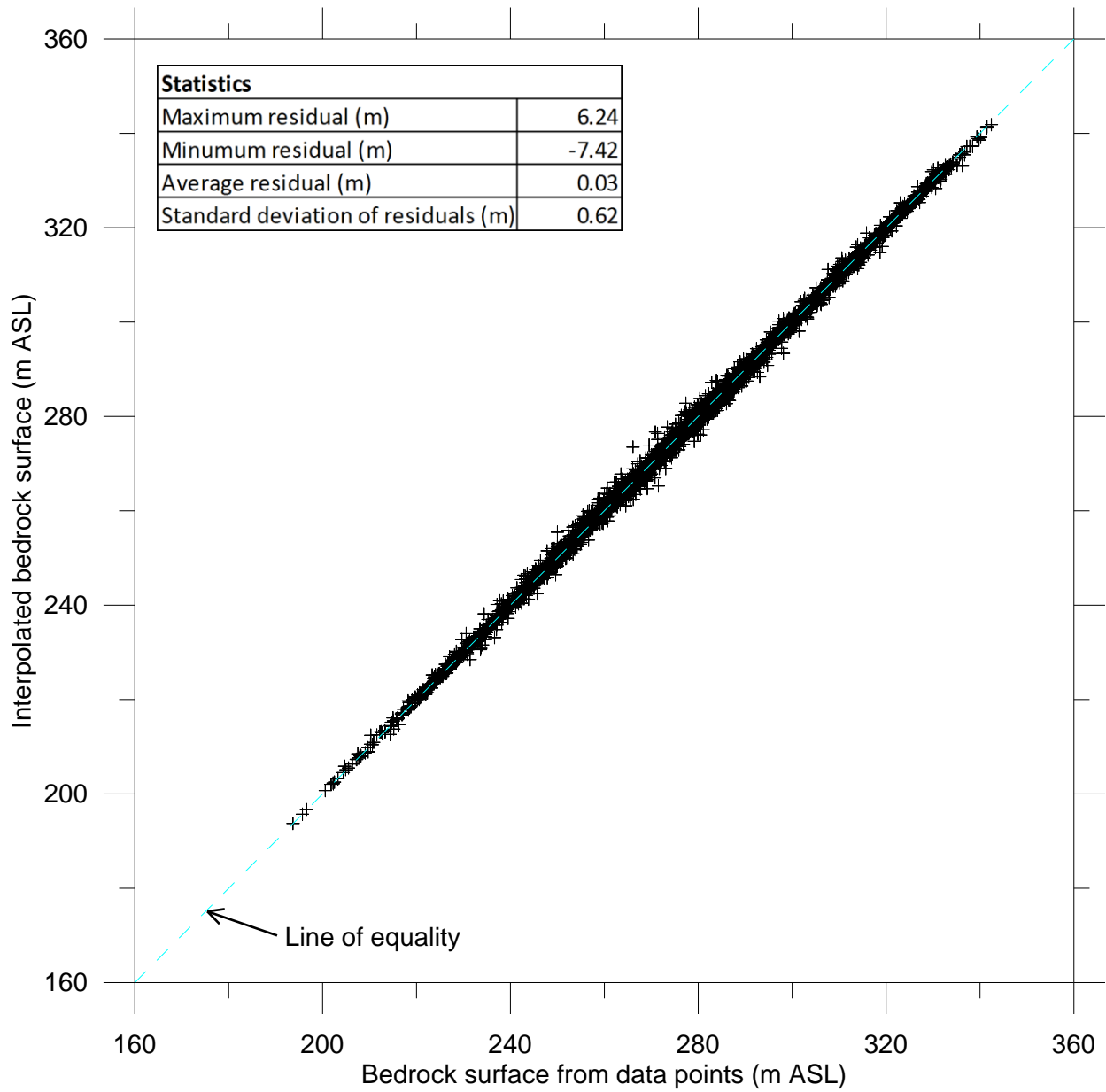


Figure 6-7 Measured and interpolated elevations of the top of bedrock

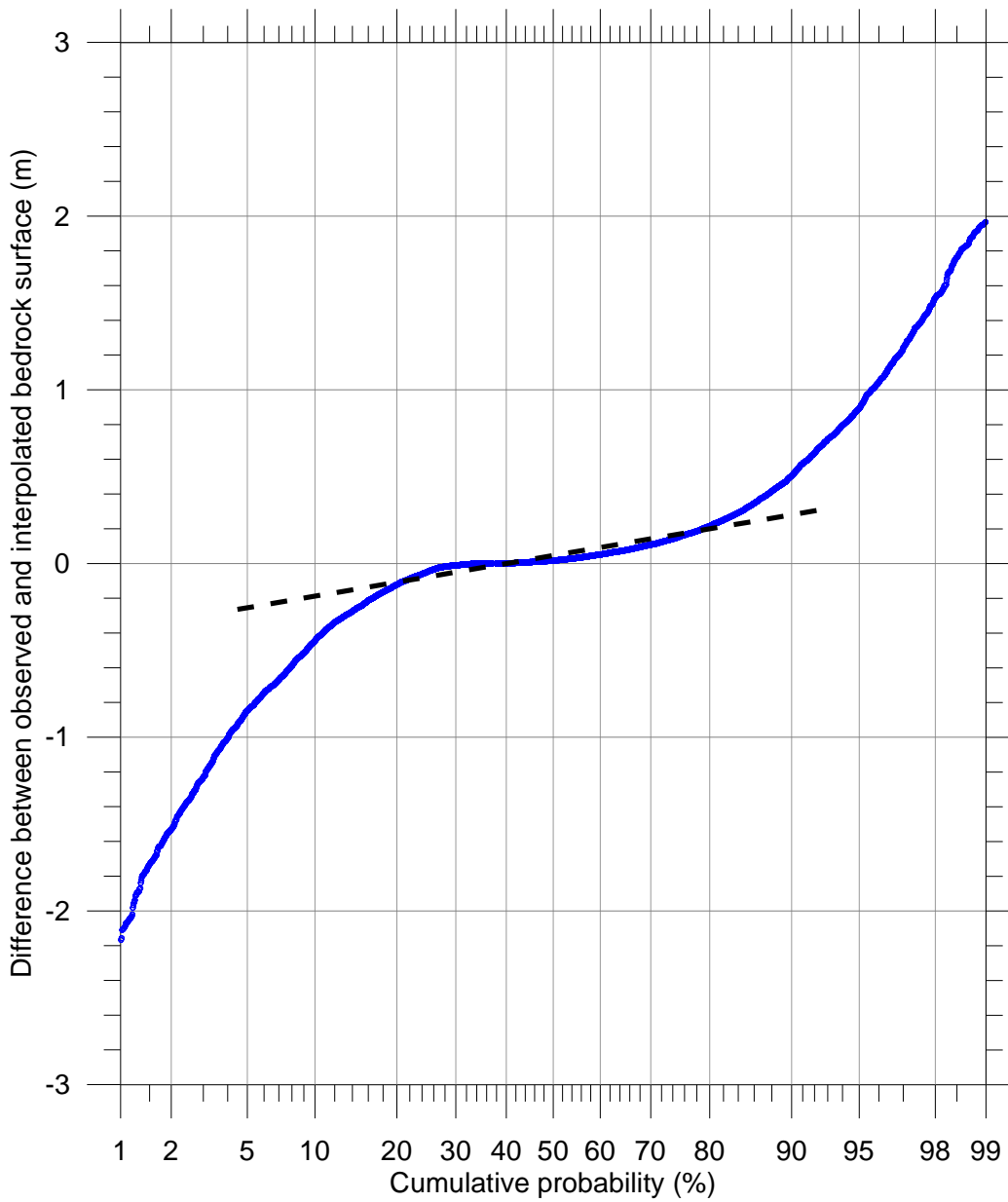
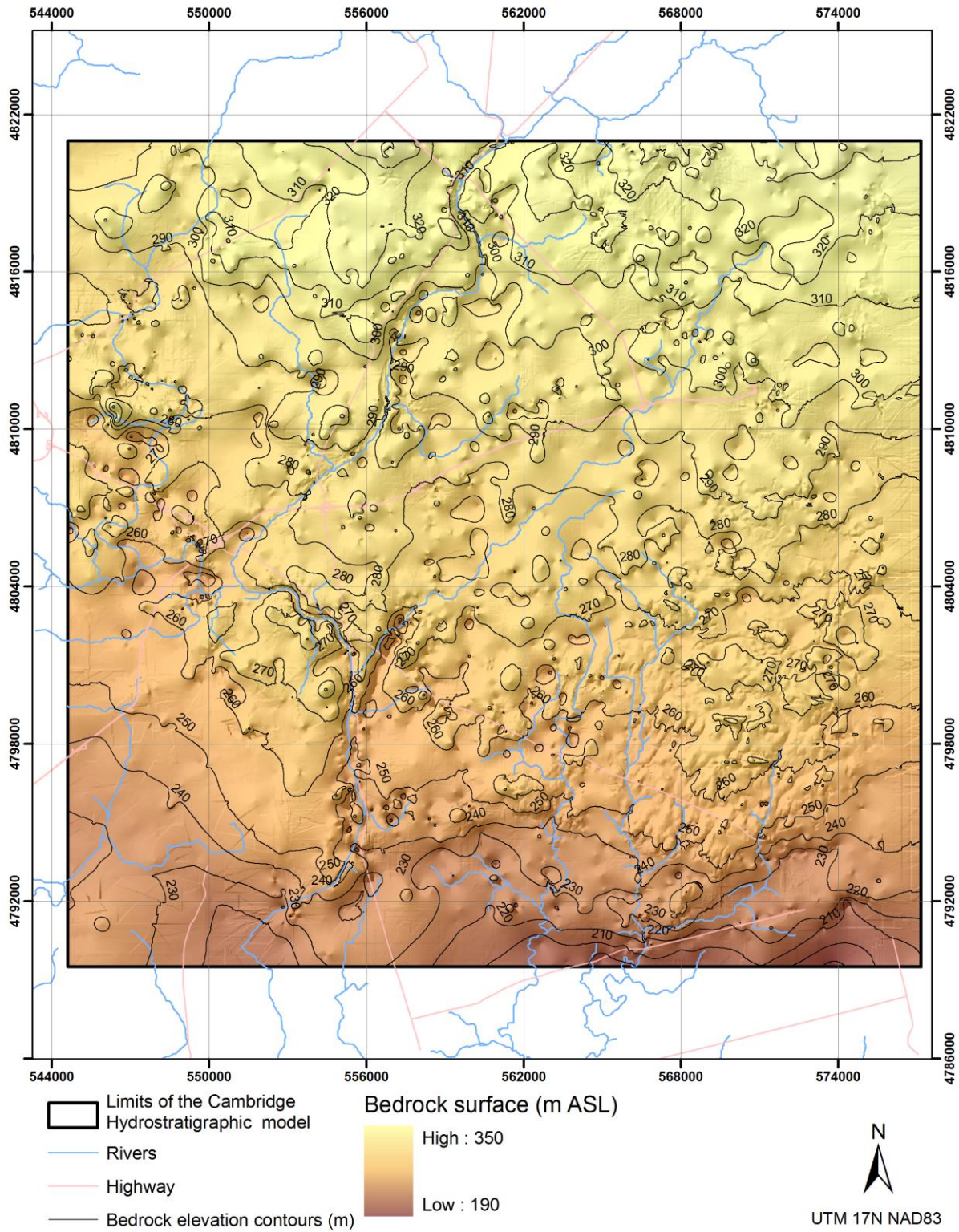
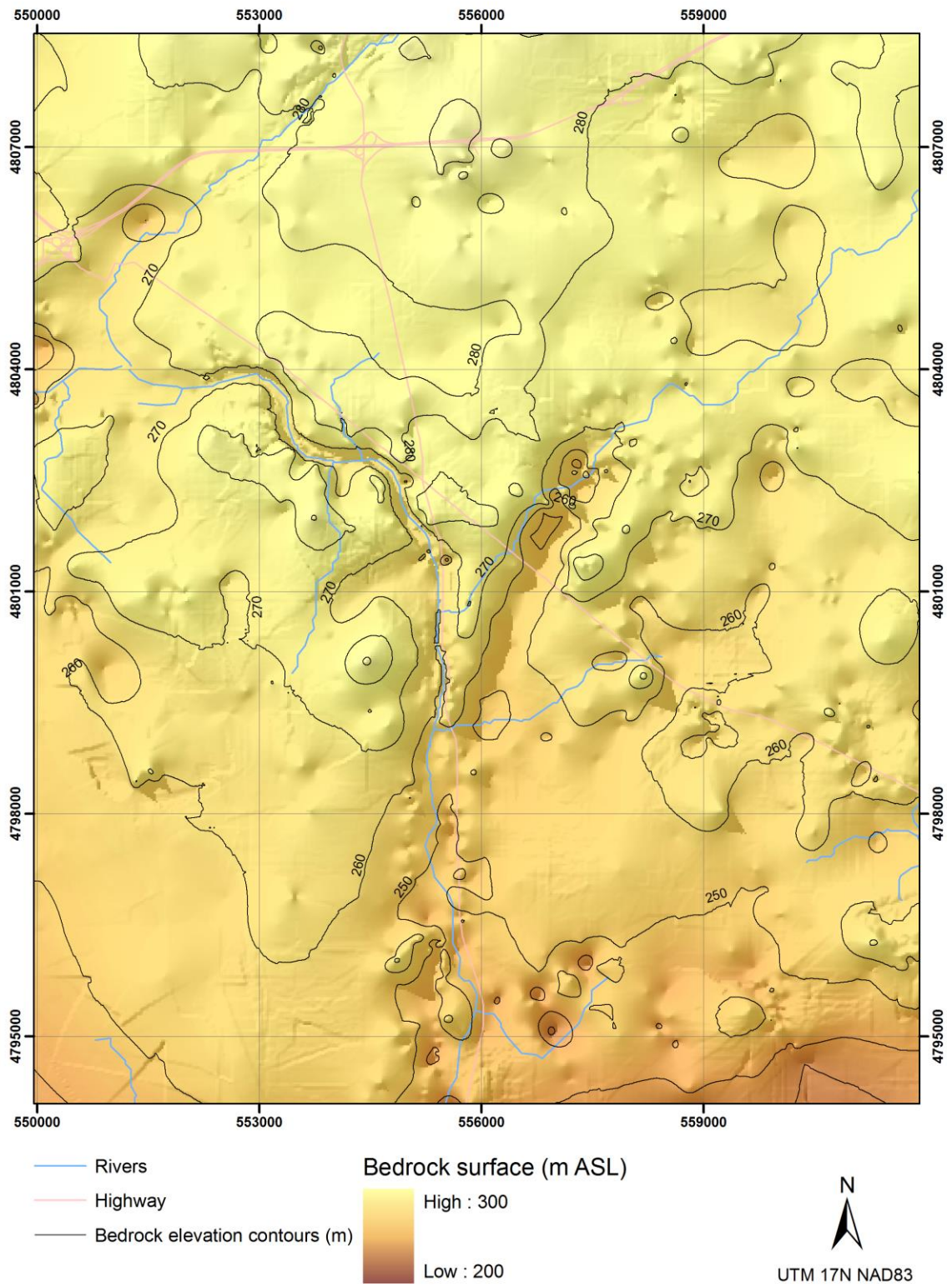


Figure 6-8 Cumulative probability distribution of the differences between the observed and interpolated elevations of the top of bedrock



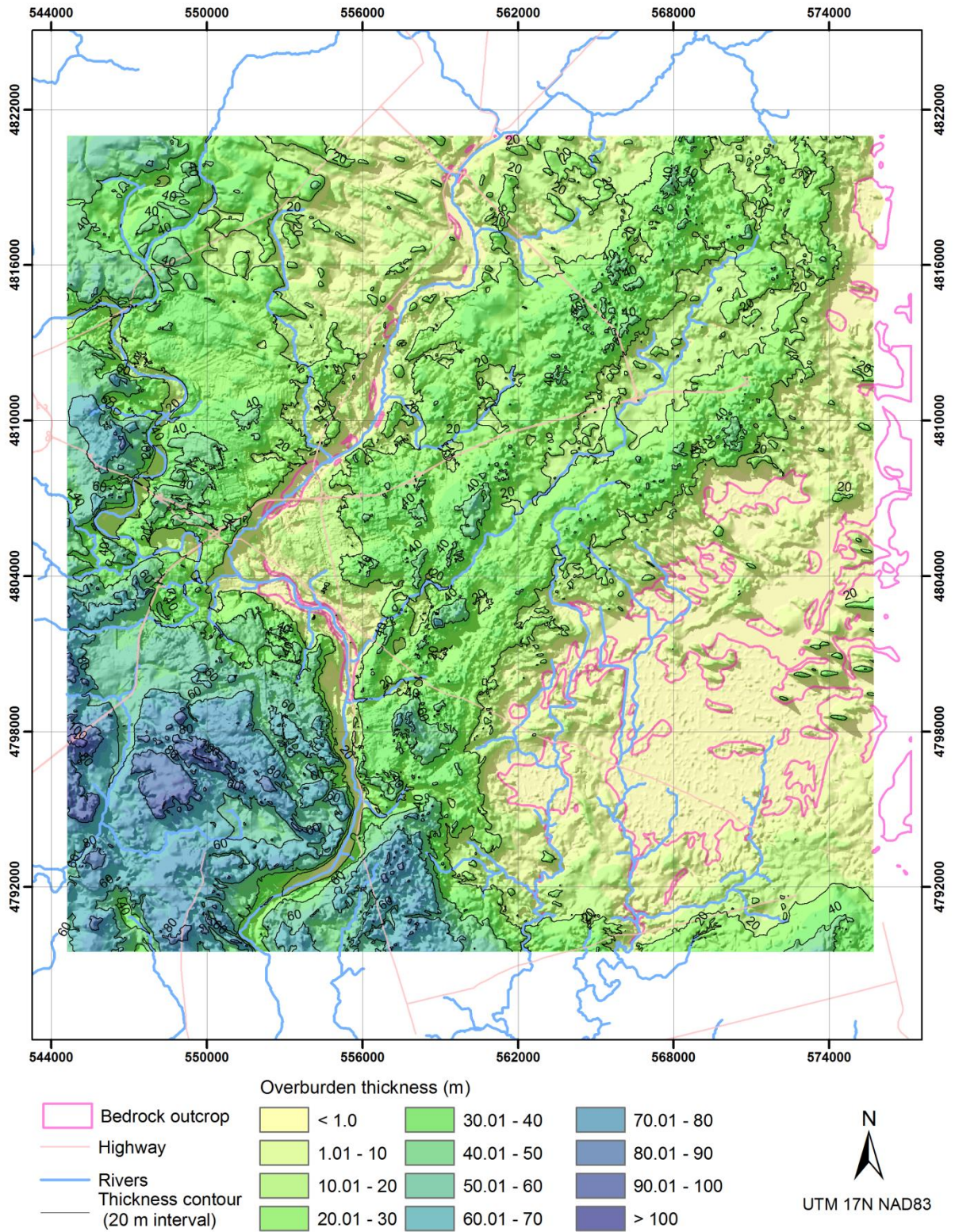
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Figure 6-9 Final interpolated bedrock surface



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Figure 6-10 Final interpolated bedrock surface near Mill Creek



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Figure 6-11 Final interpreted overburden thicknesses

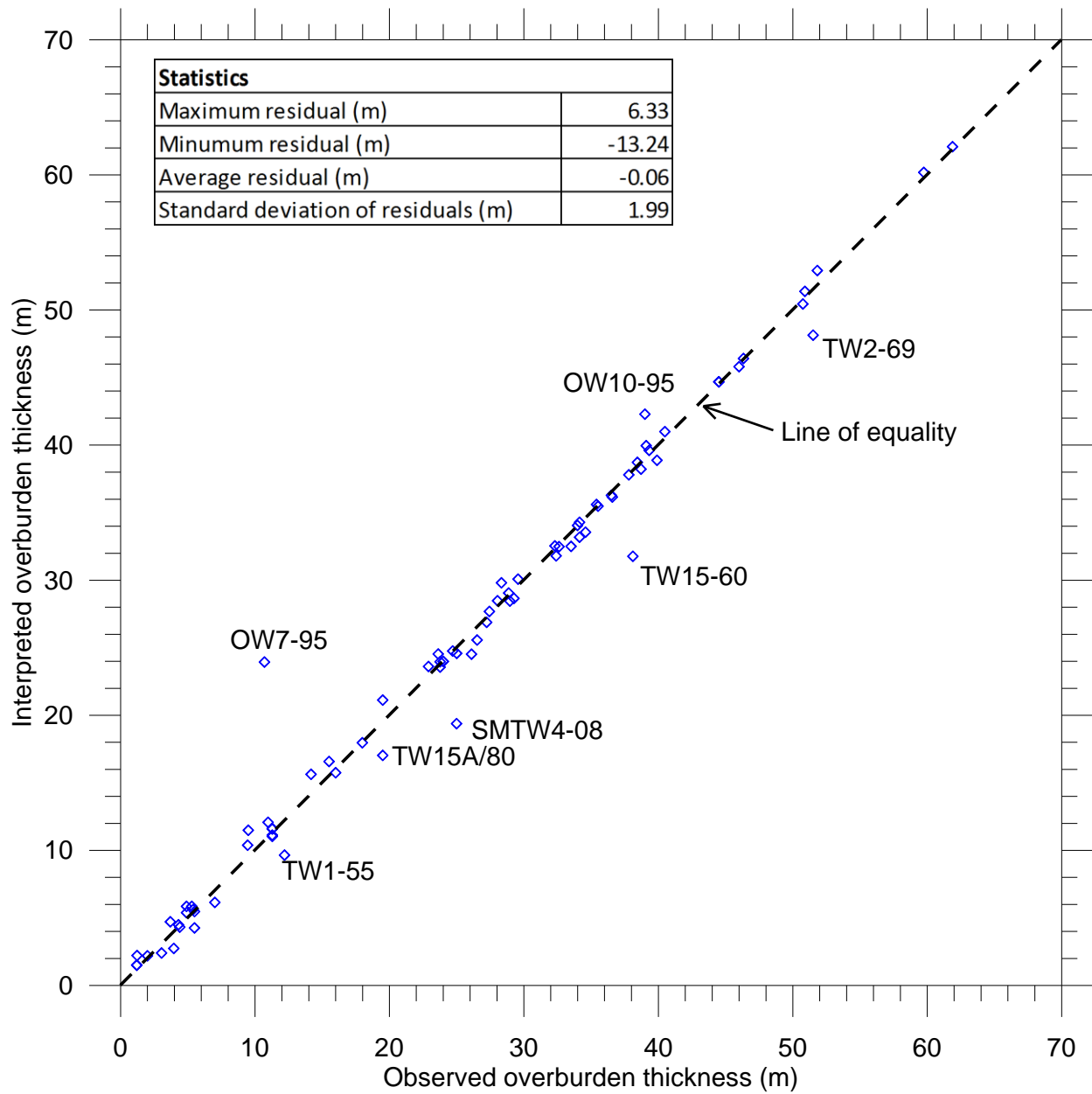


Figure 6-12 Comparison of observed and interpreted overburden thicknesses at locations of boreholes

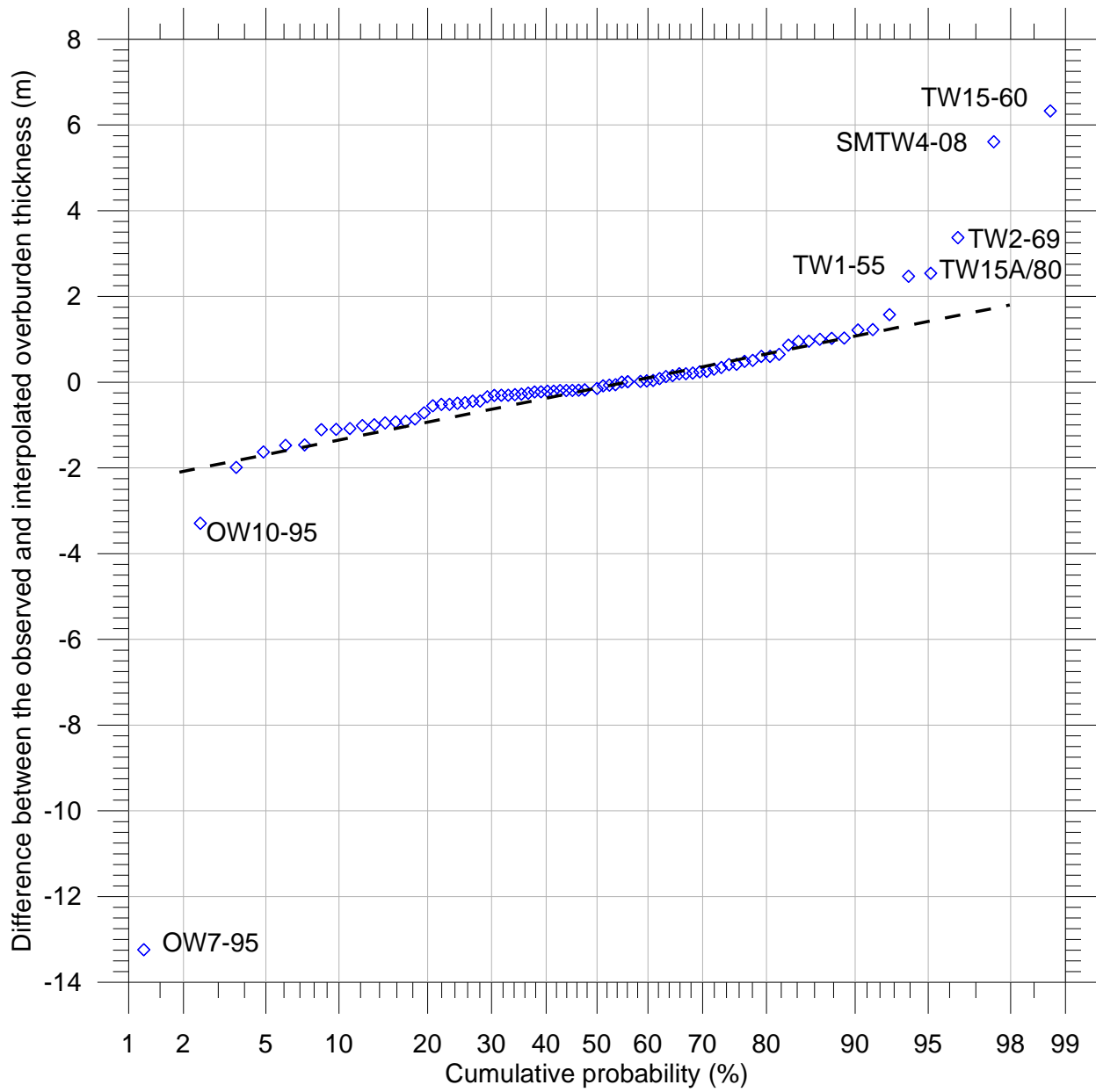


Figure 6-13 Cumulative probability distribution of the differences between the observed and interpolated overburden thicknesses at locations of boreholes

Section 7

The Reformatory Quarry Member of the Eramosa Formation

7.1 Data sources

A key revision of the Silurian stratigraphy of southern Ontario proposed recently by the Ontario Geological Survey was the treatment of the rocks identified previously as the Eramosa Member of the Guelph Formation (Johnson et al., 1992; Brunton, 2008, 2009). Brunton (2008, 2009) proposed that the Eramosa be reinterpreted as a formation with distinct members. In the Guelph area, three members were proposed for the Eramosa Formation: the Stone Road Member, the Reformatory Quarry Member, and the Vinemount Member. Brunton identified the latter two members in the Cambridge area.

The revisions proposed by Brunton were particularly important with respect to groundwater flow in the Guelph and Cambridge areas, as they provided a framework for understanding the variable role that the Eramosa unit appeared to play as an aquitard. For example, geologic logs at the Syngenta site in Cambridge indicated that the Eramosa is present; however, water level and water quality data and the results of hydraulic testing did not suggest that the unit acts as an aquitard (Turner, 2001; Burns, 2005; Plett, 2006). The reinterpretation of the geology of this area suggested that, although the Eramosa was present at the site, it was limited to the Reformatory Quarry Member. Where the Vinemount Member of the Eramosa Formation was absent, the Eramosa Formation was not necessarily a confining unit.

The following data were used to construct the top surface of the Reformatory Quarry Member for the Cambridge East groundwater model:

- Bedrock picks from Golder Associates Ltd.: 43 points ;
- Bedrock picks from Stantec: 14 points; and
- Data points used for the Guelph Tier 3 study: 173 points (including 72 artificial control points).

During the development of the top surface of the Reformatory Quarry Member, Brunton (2009, written communication) suggested that the bedrock present at surface in the east and southeast portions of the model area were the Reformatory Quarry Member. Along the Speed River, it was primarily the Guelph Formation that was present at surface. Following Brunton's suggestion, additional points with elevations equal to the ground surface were added at the locations of bedrock outcrops in the eastern and southern portions of the model area.

7.2 Data organization and quality control

All data sets were checked and duplicate points removed based on their reliability ranking and consistency with the surrounding points. The numbers of data points left were:

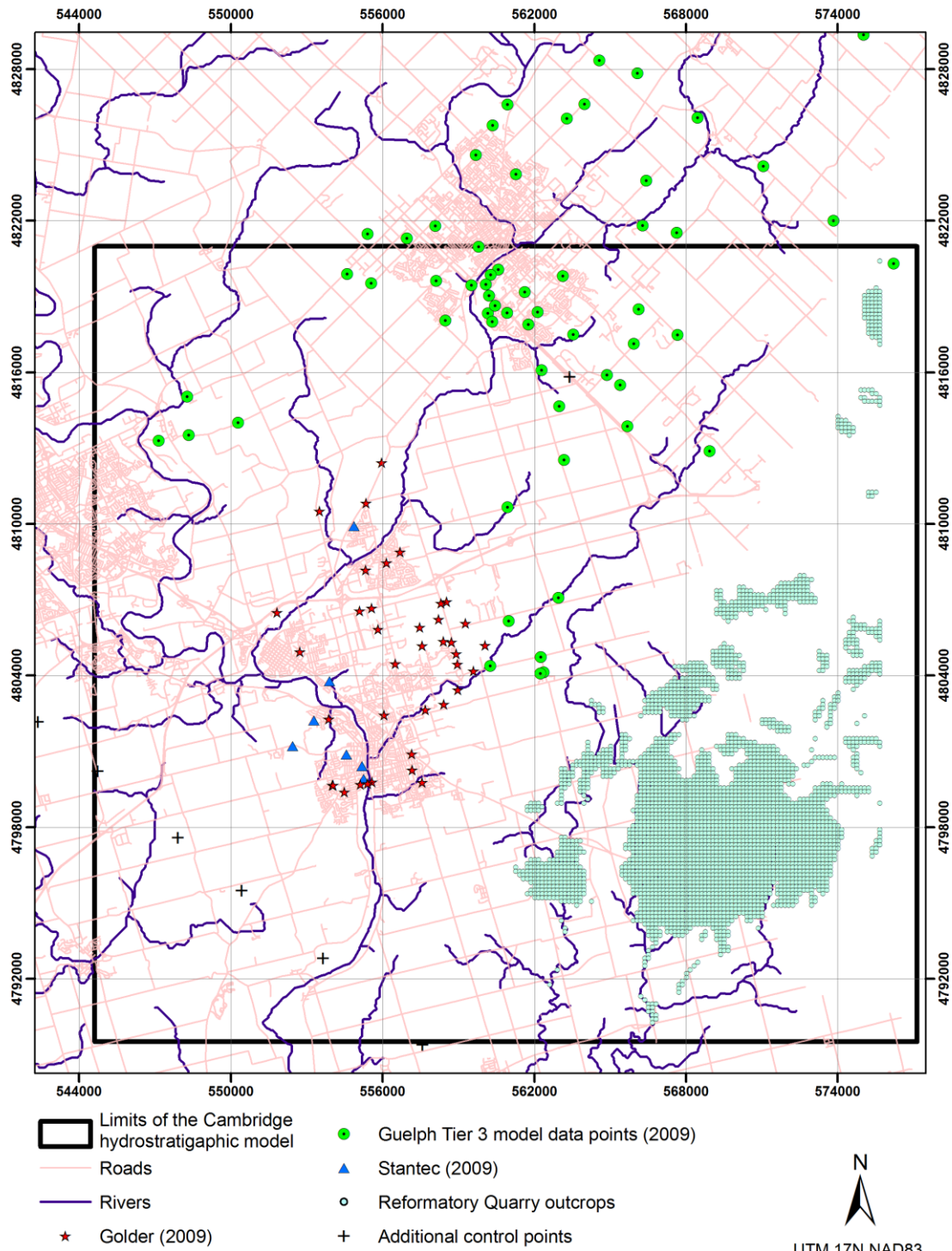
- Bedrock picks from Golder Associates Inc.: 38 points;
- Bedrock picks from Stantec: 7 points; and
- Data points for the Guelph Tier 3 study: 127 points (including 72 artificial control points).

A complete listing of the borehole picks, including an indication of those points that were excluded from the interpretation, is included as part of the metadata for this study. A total of 3,289 data points were added to represent the outcrops of the Reformatory Quarry Member in the east and southeast portion of the study area. Another 8 control points were added at the southwest corner of the model domain to reflect the south-westward dipping of the surface. The locations of all control points used to define the top surface of the Reformatory Quarry are shown in **Figure 7-1**. During the development of the surface, 877 additional control points with surface elevations extracted from the Guelph Tier 3 surface for the top of the Reformatory Quarry Member were added along the eastern domain boundary to ensure consistency between the Guelph and Cambridge surface models.

7.3 Methodology and results

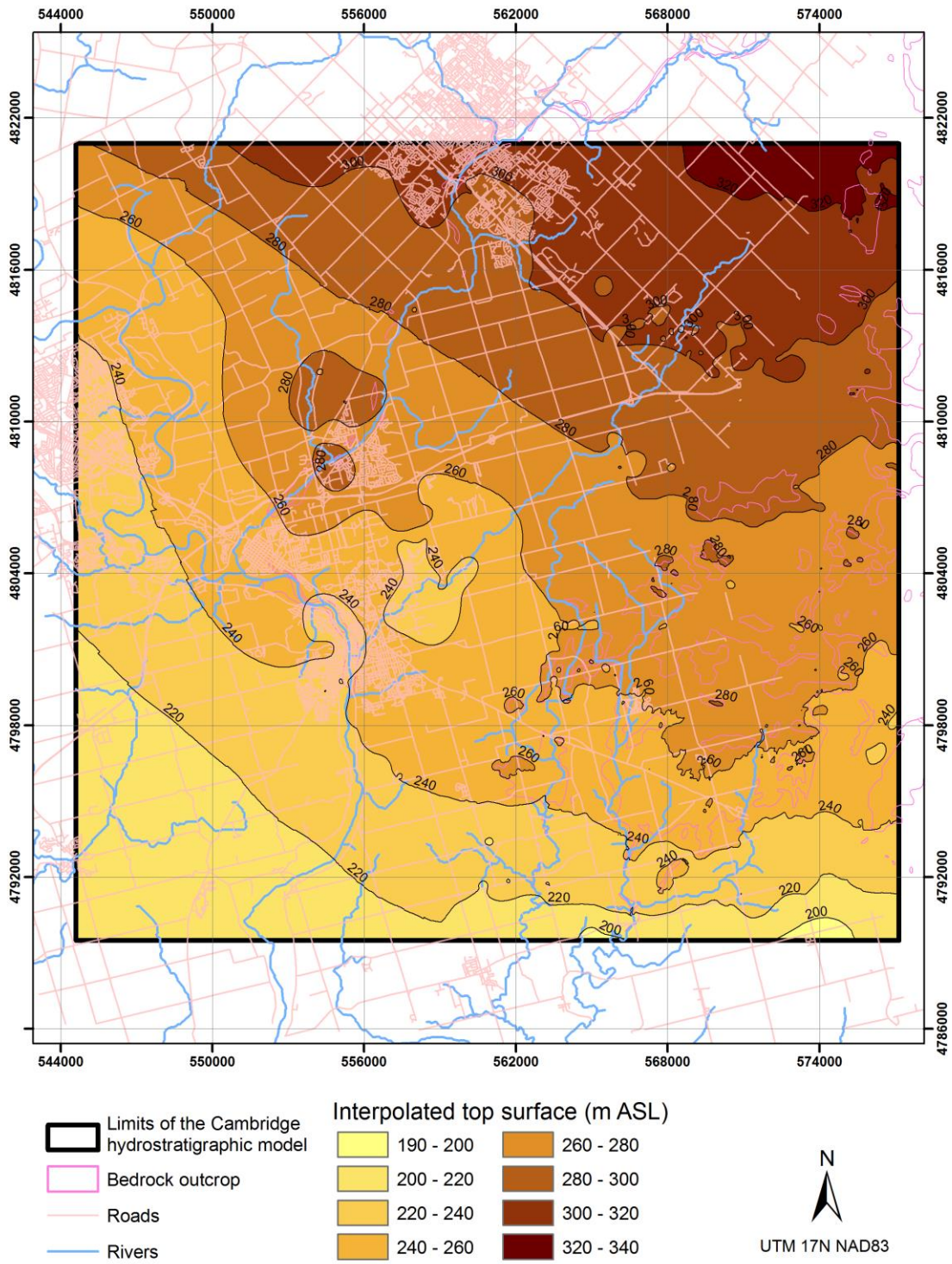
The top surface of the Reformatory Quarry Member was developed using interpolation with an Ordinary Kriging algorithm and a linear variogram. The resulting surface was compared with both the bedrock surface and the top of the underlying Vinemount Member to ensure that no surfaces overlapped. Control points were added to the data sets in those areas where surface overlaps were due to interpolation artefacts, and the interpolation was repeated through successive iterations. The final interpolated surface of the top of the Reformatory Quarry Member is shown in **Figure 7-2**. The observed and interpolated elevations of the top of the Reformatory Quarry Member at the locations of the control points are compared in **Figure 7-3**. The scatterplot shown in **Figure 7-3** suggests that the interpolated values are consistent with the observations. A plot of the cumulative probability distribution of the differences between the observed and interpolated elevations is presented in **Figure 7-4**. The mean absolute error in the interpolation was 0.16 m. The differences at over 98% of the control points were less than 1.0 m.

The thickness of the Guelph Formation was calculated by subtracting the top surface of Reformatory Quarry Member from the final bedrock surface. The resulting map of the thickness of the Guelph Formation is presented in **Figure 7-5**. The areas of bedrock outcrops are also indicated in **Figure 7-5**. Brunton (2009, written communication) indicated that the Reformatory Quarry Member is the uppermost bedrock unit over these areas. Therefore, the outcrop areas also corresponded to areas where the Guelph Formation was absent.



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Figure 7-1 Available control points for the top of the Reformatory Quarry Member of the Eramosa Formation



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Figure 7-2 Interpolated elevations of the top of the Reformatory Quarry Member

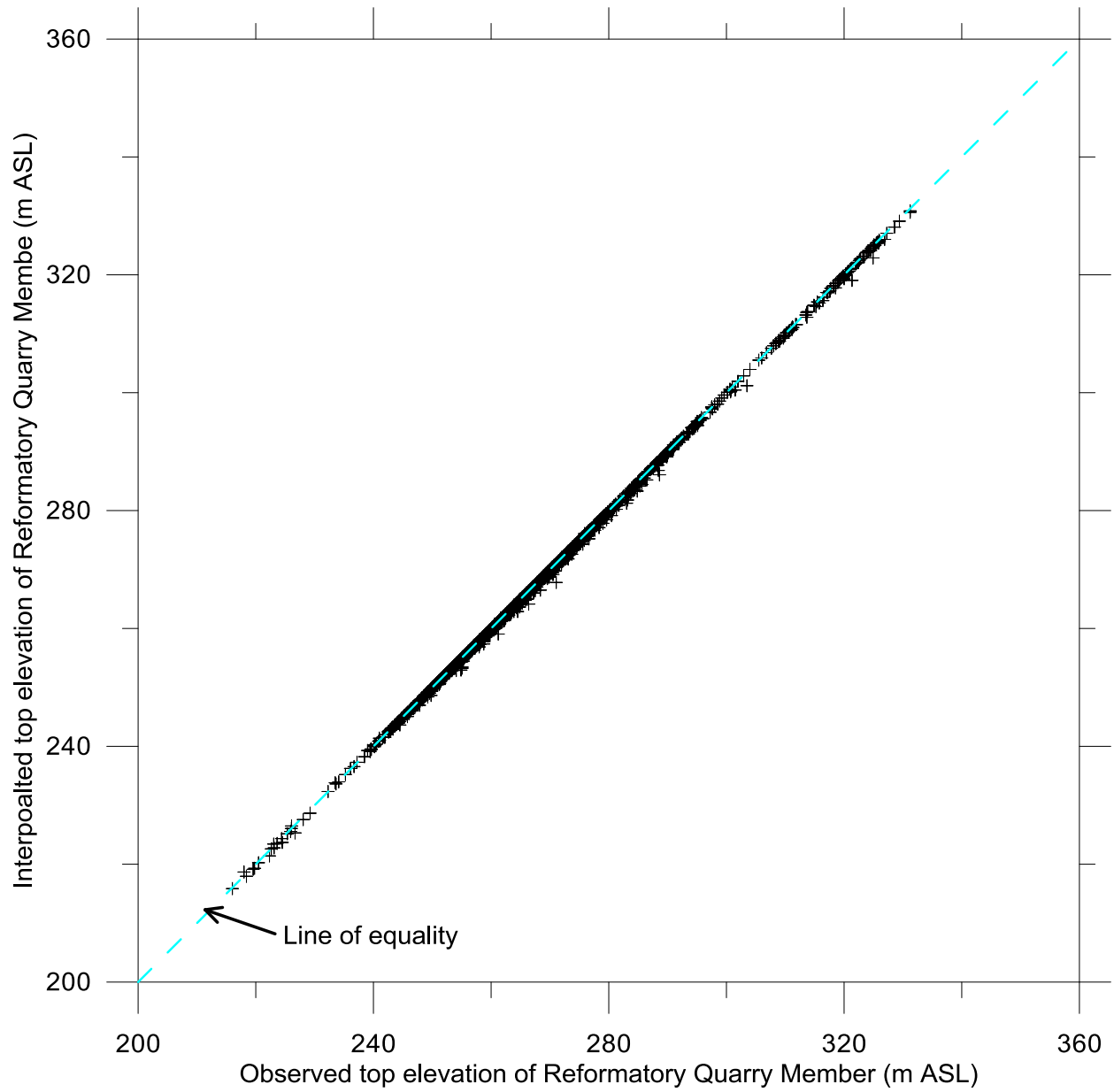


Figure 7-3 Observed and interpolated elevations of the top of the Reformatory Quarry Member

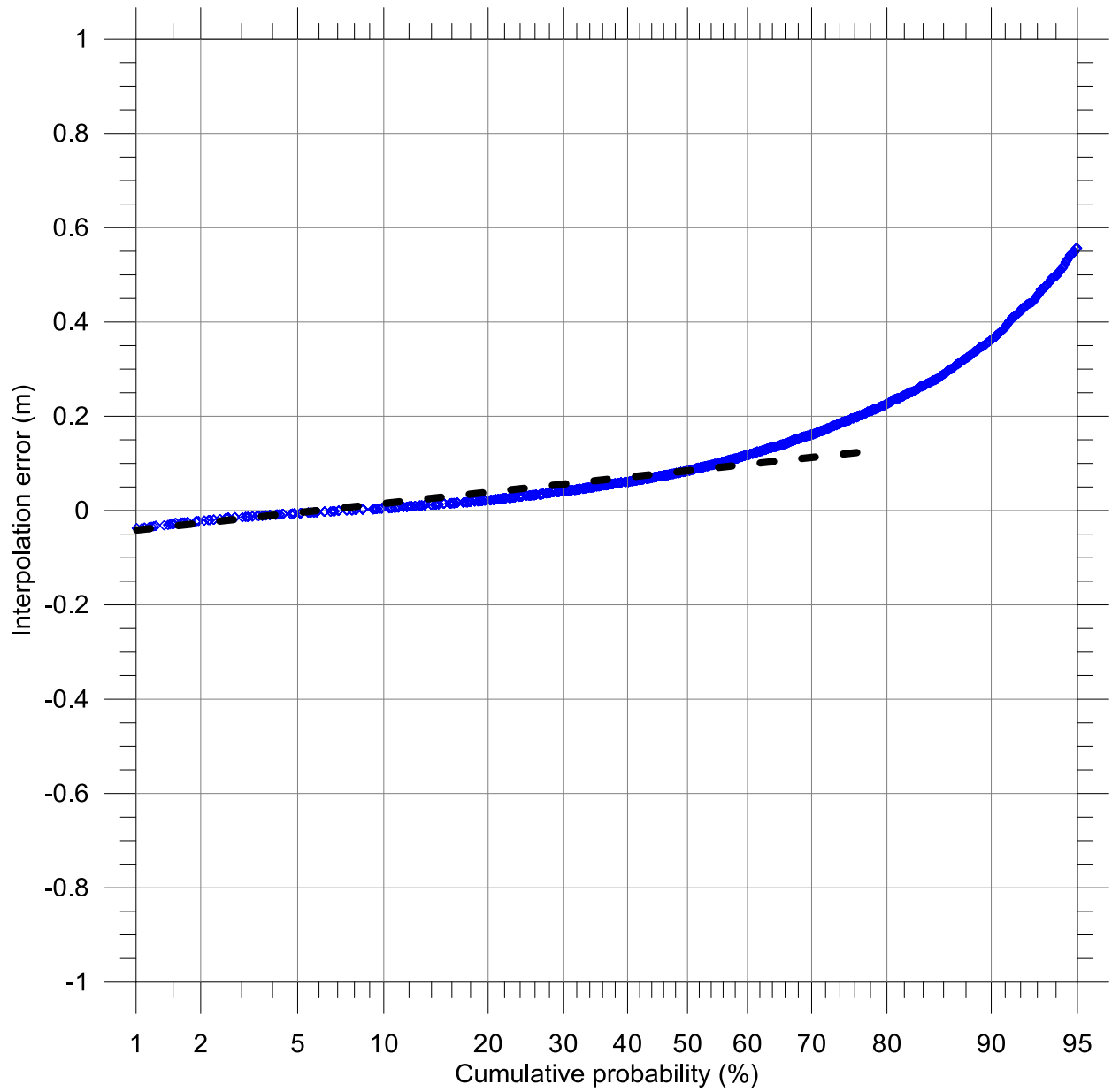
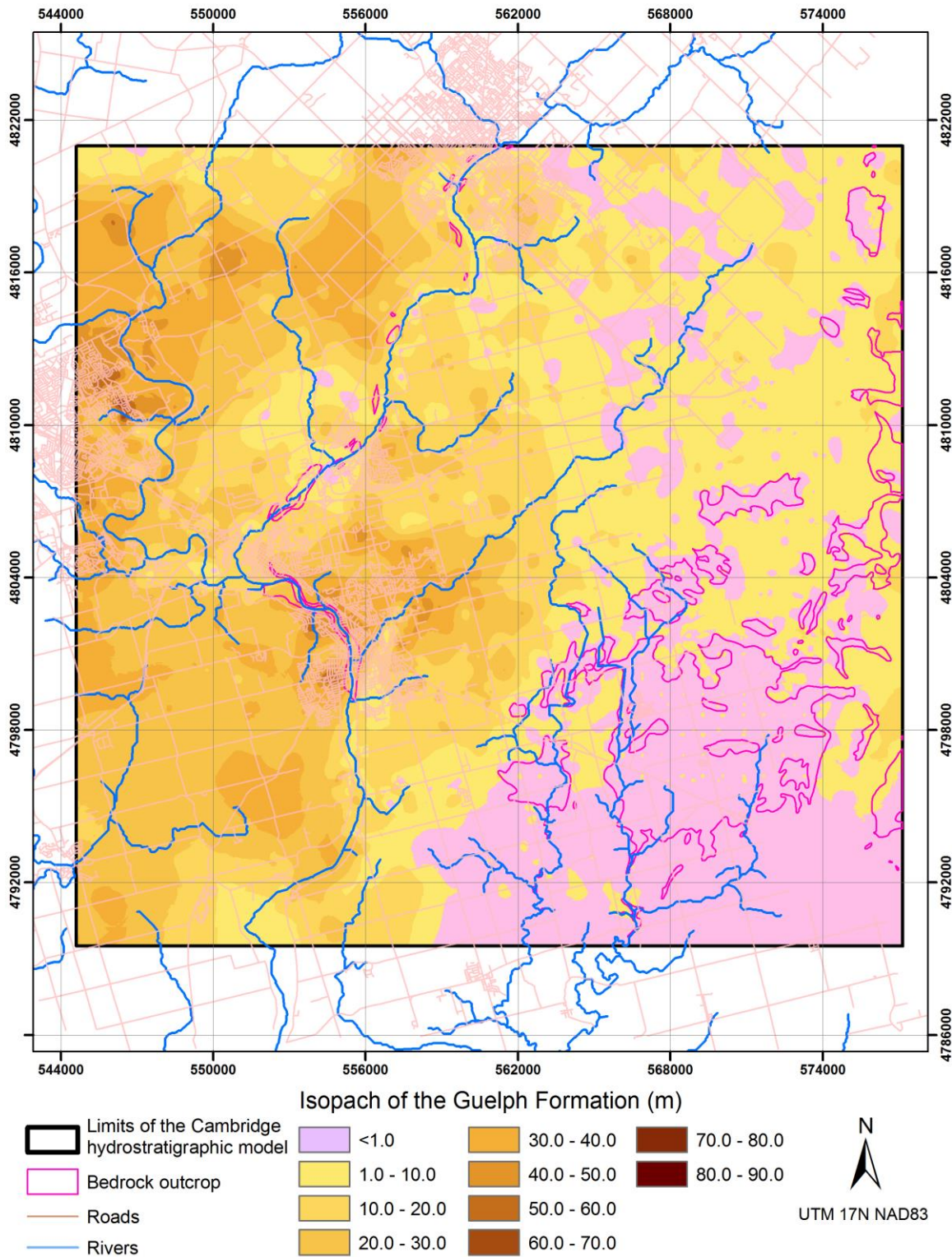


Figure 7-4 Cumulative probability distribution of errors in the interpolated elevations of the top of the Reformatory Quarry Member



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Figure 7-5 Interpreted thicknesses of the Guelph Formation

Section 8

The Vinemount Member of the Eramosa Formation

8.1 Data sources

The Vinemount Member of the Eramosa Formation was a critical hydrogeologic unit for the study area. Hydrogeologic data from the Guelph area indicated that, where present, the Vinemount Member acted as an aquitard between the Guelph Formation and the Gasport Formation (Golder, 2011a). The development of the model layer representing the Vinemount Member differed from the overlying units because it was not present over the entire model area. It was particularly important to represent the areas where the Vinemount Member was absent as correctly as possible. To increase accuracy, a map of the thicknesses of the Vinemount Member was initially developed and the elevations of its top surface were estimated by adding the thicknesses of the member to the elevations of the top of the underlying Goat Island Formation. The following data were used to map the thicknesses of the Vinemount Member:

- Bedrock picks from Golder Associates Ltd.: 29 points;
- Bedrock picks from Stantec: 5 points; and
- Data points used for the Guelph Tier 3 study: 151 points (including 93 artificial control points).

8.2 Data organization and quality control

All data sets were checked and the duplicate data points removed based on their relative reliability ranking and consistency with surrounding data points. The numbers of data points that were retained from the different sources were:

- Bedrock picks from Golder Associates Ltd.: 28 points;
- Bedrock picks from Stantec: 3 points; and
- Data points that are used for the Guelph Tier 3 study: 151 points (including 93 artificial control points).

The Stantec picks, Golder picks, and the picks made for the Guelph Tier 3 study were assigned equal weighting. In general, the locations of all data points are presented in **Figure 8-1**.

The inference of the extent of the Vinemount Member was a crucial element of the hydrostratigraphic model; however, as suggested in **Figure 8-1**, borehole control for this unit was relatively sparse.

To ensure that the Vinemount Member remained present after interpolation from the borehole data, additional control points were incorporated in the southeastern corner of the study area. The Vinemount Member was assigned a thickness of 4 m in the area, based on the approach adopted for the Guelph Tier 3 model. The approach for the Guelph Tier 3 model was in turn based on the suggestion that the thickness of the Vinemount Member in that area ranges between 0 and 10 m (Brunton, personal communication, 2009).

8.3 Methodology and results

A continuous distribution of thickness of the Vinemount Member was developed using interpolation with Ordinary Kriging and a linear variogram. The top surface of the Vinemount Member was calculated by adding its interpolated thickness to the top surface of the underlying Goat Island Formation. The resulting surface was checked to ensure that it lay at or below the surface of the overlying Reformatory Quarry Member. During the checking, it was found that cross-cutting occurred occasionally along the eastern boundary. After the top surface of the Vinemount Member was finalized, the thicknesses of the Vinemount Member were re-calculated by subtracting the elevations of the bottom of the Vinemount Member from the top.

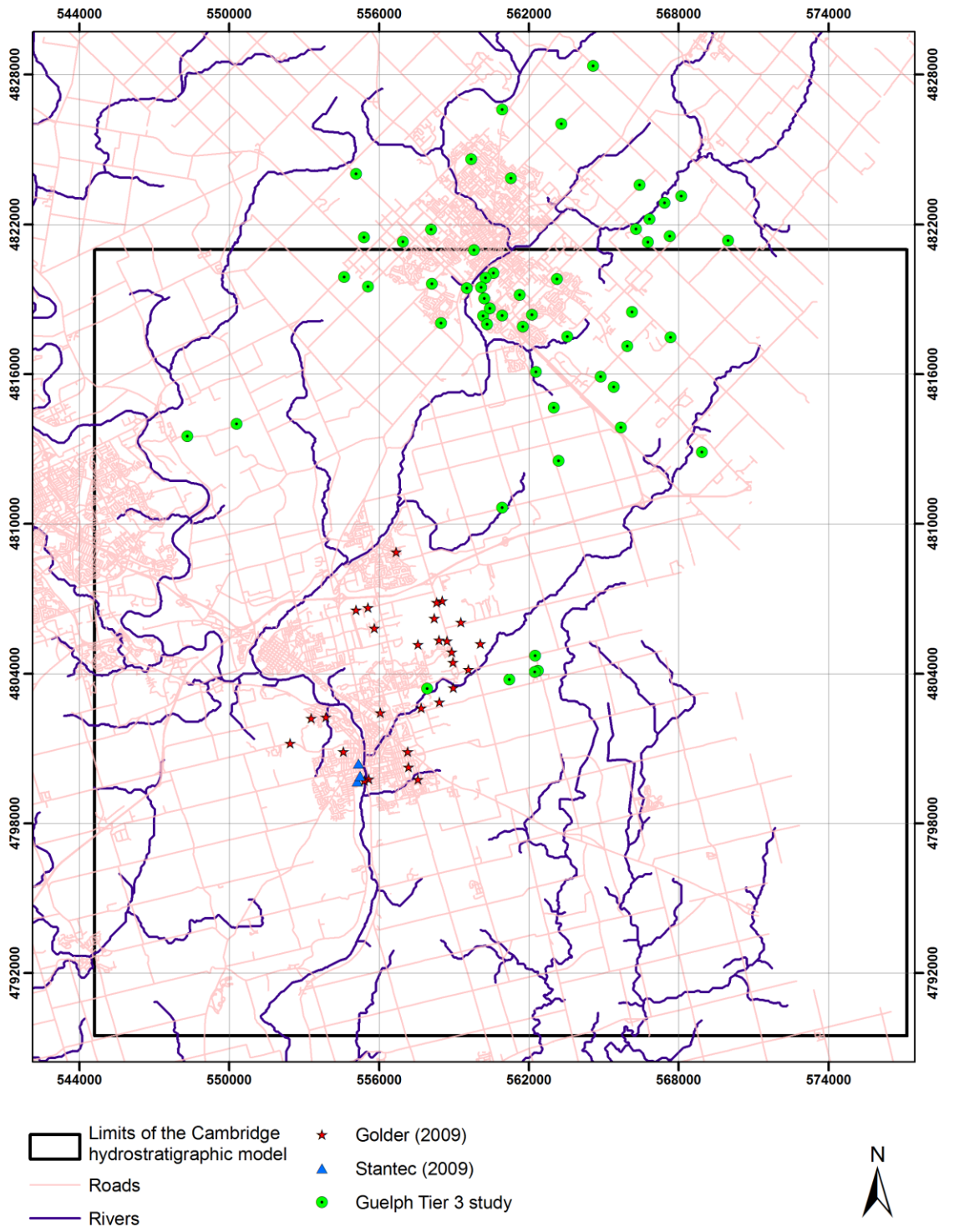
The final interpreted thicknesses of the Vinemount Member are shown in **Figure 8-2**. The thicknesses ranged from 0 to 16 m. The Vinemount Member was predicted to be absent over the extreme eastern portion of the interpolation area. This prediction was consistent with the interpretation from the Guelph Tier 3 study of the same area.

The line of zero thickness developed through interpolation corresponds to the boundary between the pink and yellow areas in **Figure 8-2**. Subsequent to the present analyses, Golder Associates provided a line of zero thickness, indicated by the thick blue line in the figure. The relatively close correspondence between the two interpretations of the extent of the Vinemount Member provided an independent confirmation of the reliability of the analyses.

The observed and interpolated thicknesses of the Vinemount Member at the location of boreholes compared are shown in **Figure 8-3**. As shown in the scatterplot, the thicknesses observed in boreholes and interpolated at the same locations match closely. A plot of the cumulative probability distribution of the differences between the observed and interpolated thicknesses is shown in **Figure 8-4**. The differences approximated a straight line when plotted on a normal probability axis, which suggested that the differences were distributed randomly. The mean absolute error in the interpolation was less than 0.03 m. The differences at over 96% of the control points were less than 0.2 m.

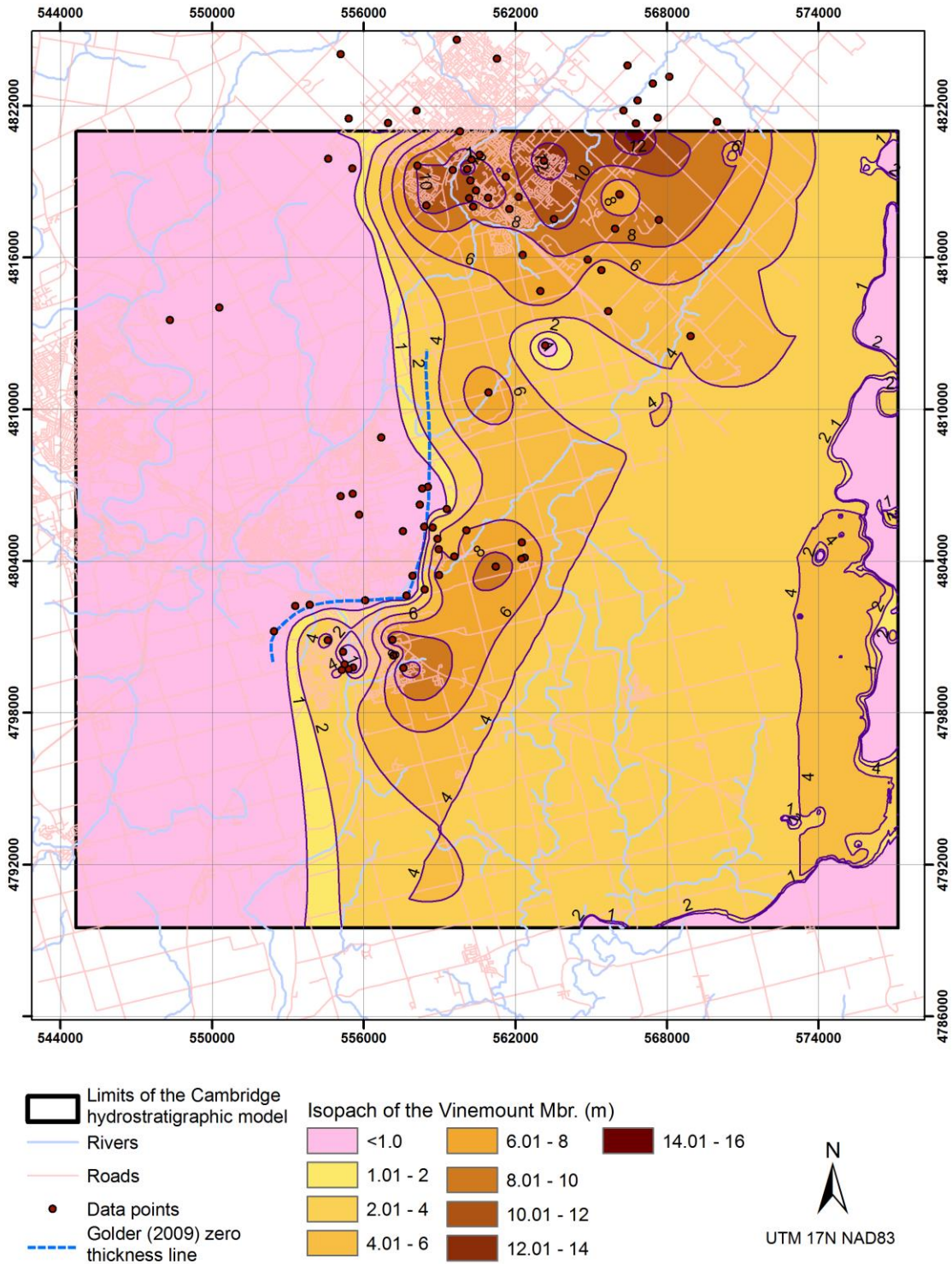
The elevations of the final top surface of the Vinemount Member are shown in **Figure 8-5**. In general, the elevations decline toward the southwest. To check the quality of the interpreted elevations, the elevations were compared with the data from the borehole picks. This comparison is presented in **Figure 8-6**. The calculated elevations of the top of the Vinemount Member approximate closely the data from the borehole picks.

The thickness of the Reformatory Quarry Member was calculated by subtracting the final top surface of the Vinemount Member from the final top surface of the Reformatory Quarry Member. The results are presented in **Figure 8-7**. Some thin areas (less than 1 m) near the eastern boundary of the study area are shown in this figure. These areas reflect a lack of borehole control in that area; the interpolated elevations of the tops of both the Reformatory Quarry and Vinemount Members generally exceeded the elevations of the top of the bedrock. During the development of the final model surfaces, the elevations of the tops of the Reformatory Quarry and Vinemount Members were shifted downward to ensure that no surfaces overlapped. This adjustment was essentially cosmetic with respect to the development of groundwater model, as this area is beyond the limits of the groundwater model.



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Figure 8-1 Available data points for the thicknesses of the Vinemount Member of the Eramosa Formation



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Figure 8-2 Interpolated thicknesses of the Vinemount Member

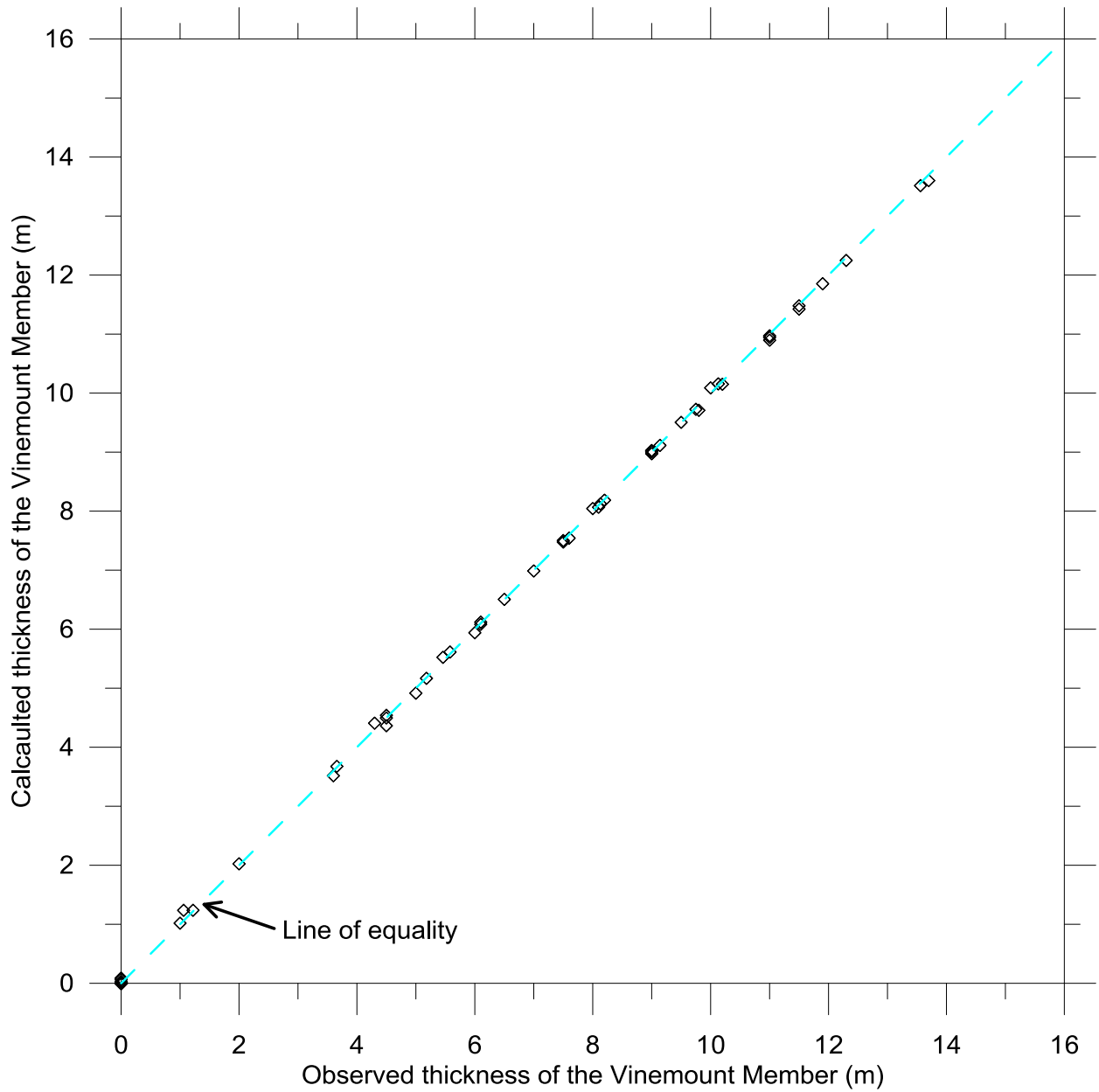


Figure 8-3 Comparison of observed and calculated thicknesses of the Vinemount Member

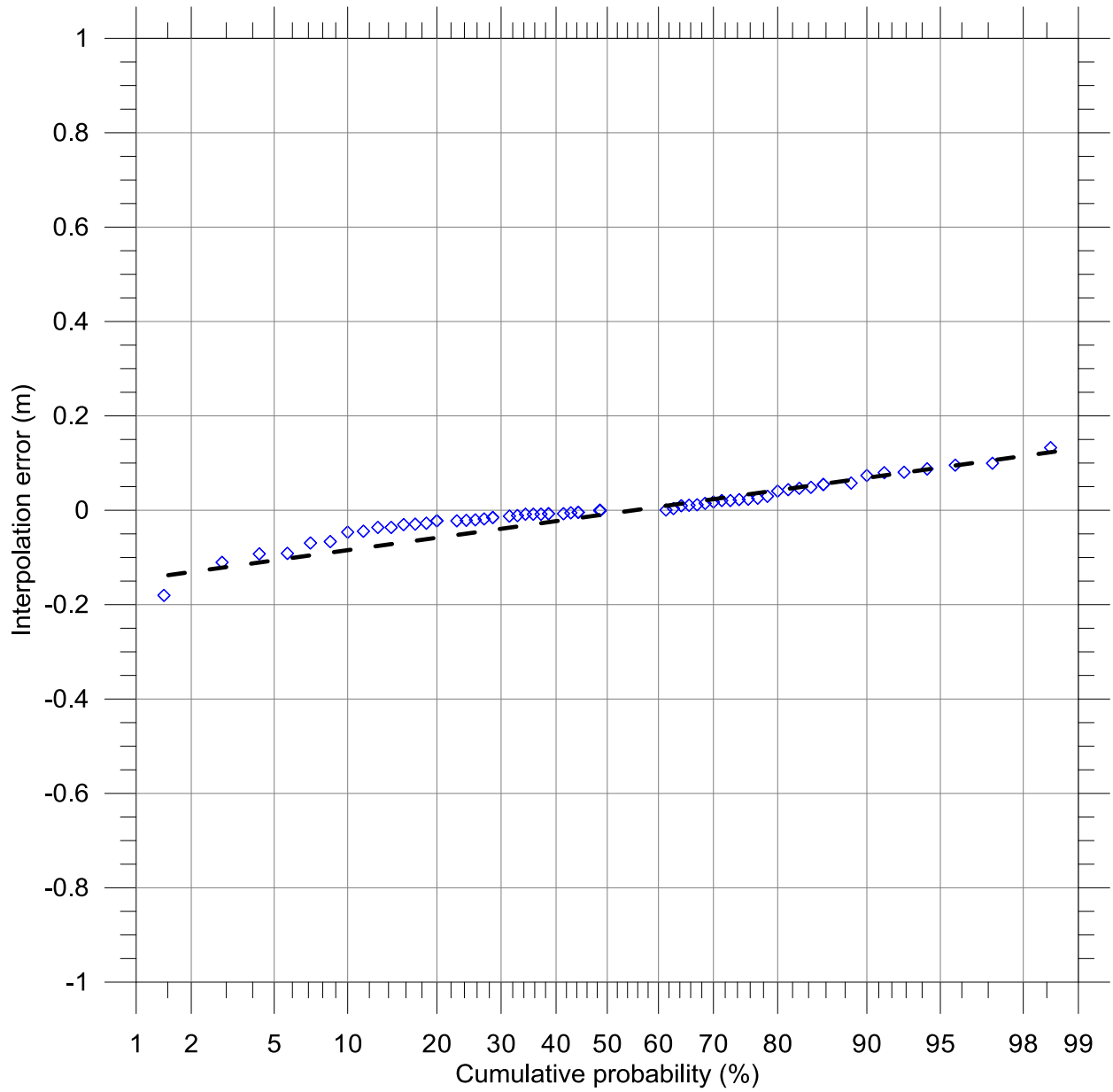
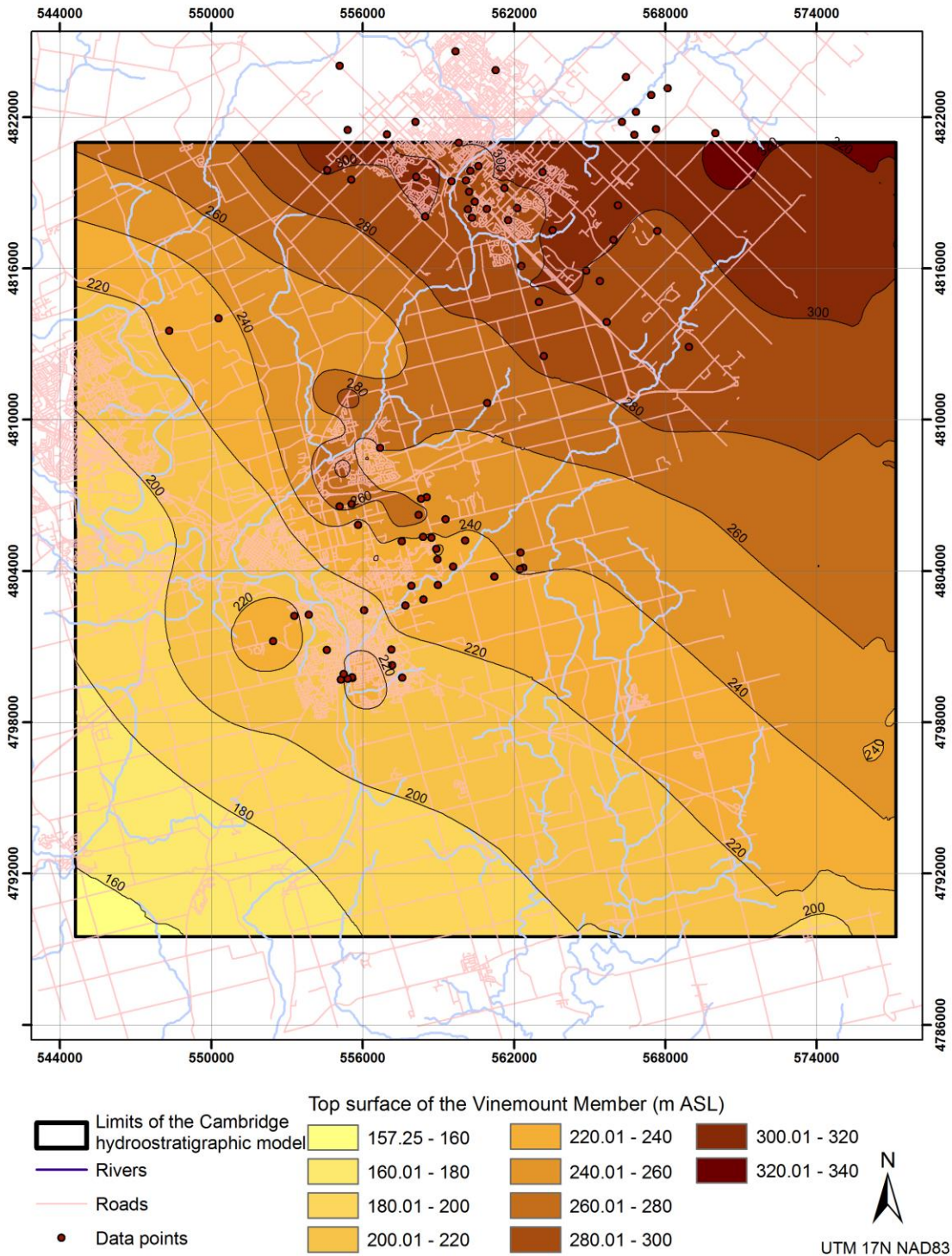


Figure 8-4 Cumulative probability distribution of differences between observed and interpolated thicknesses of the Vinemount Member



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Figure 8-5 Calculated elevations of the top of the Vinemount Member

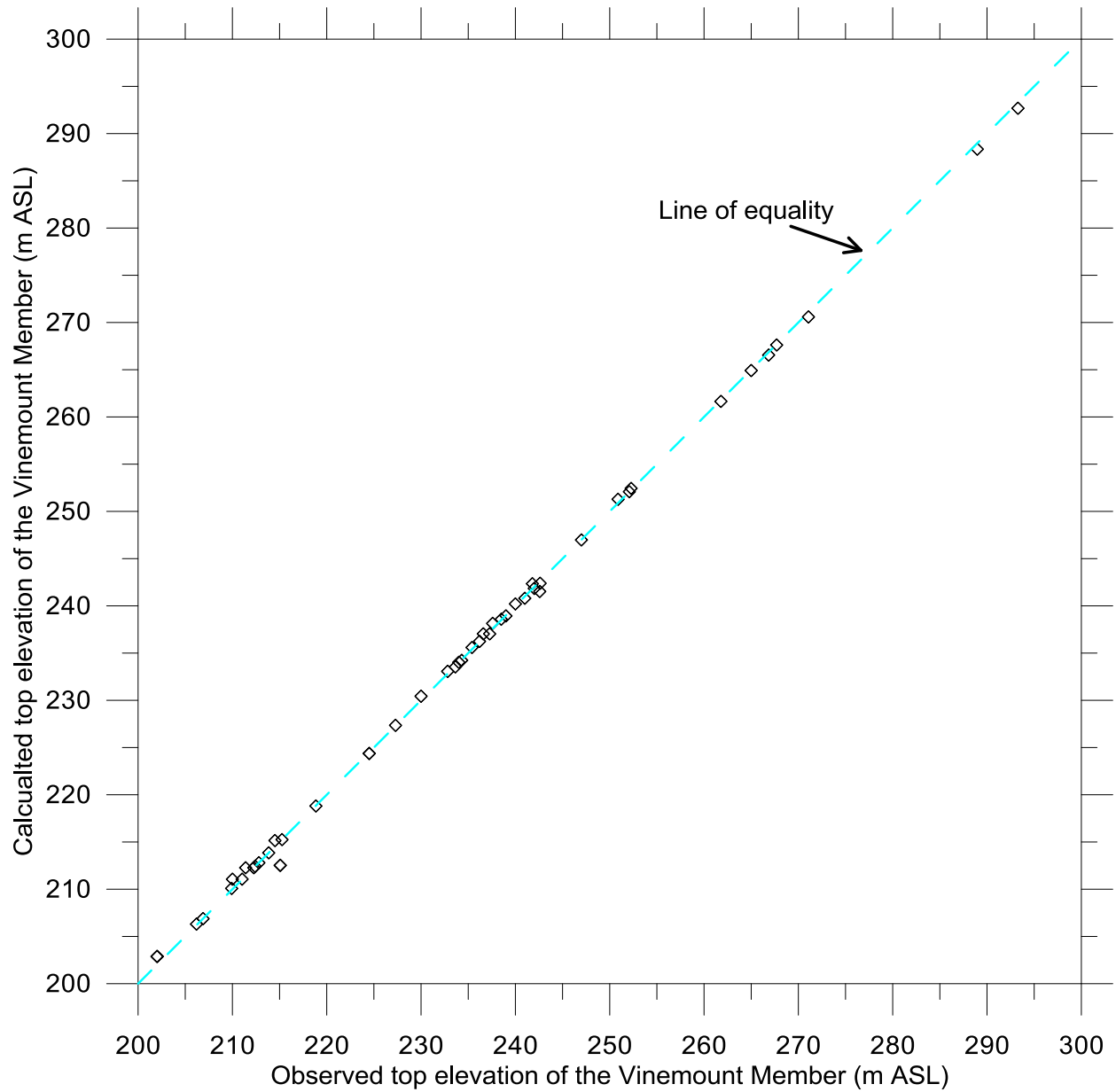
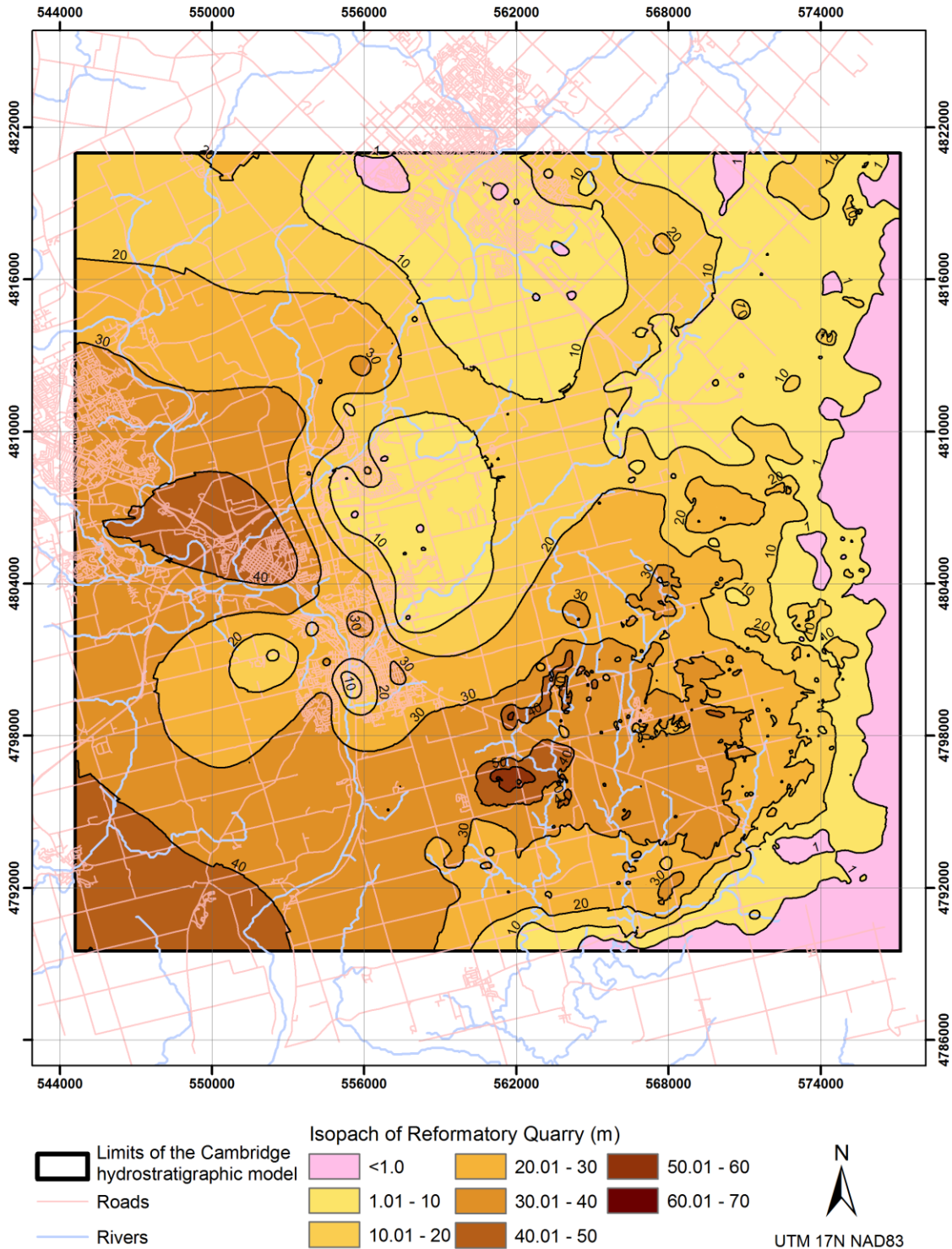


Figure 8-6 Comparison of observed and calculated elevations of the top of the Vinemount Member



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Figure 8-7 Thicknesses of the Reformatory Quarry Member of the Eramosa Formation

Section 9

The Goat Island Formation

9.1 Data sources

The development of the model layer representing the Goat Island Formation was treated the same way as the Vinemount Member. The thicknesses of the unit were estimated and then its top surface developed. This approach was adopted because data characterizing the elevations of the top of the Goat Island Formation are relatively sparse, while a rough idea of its thickness is available. The following data were available to map the thickness of the Goat Island Formation:

- Bedrock picks from Golder: 38 points;
- Bedrock picks from Stantec: 11 points; and
- Data points used for the Guelph Tier 3 study: 187 points (including 86 artificial control points).

9.2 Data organization and quality control

All data sets were checked and duplicate data points removed based on their relative reliability and consistency with surrounding data points. The following data were retained from the different sources:

- Bedrock picks from Golder: 29 points;
- Bedrock picks from Stantec: 10 points; and
- Data points used for the Guelph Tier 3 study: 187 points (including 86 artificial control points).

A complete listing of the borehole picks, including an indication of those points were been excluded from the interpretation, is included in the metadata for this project. The locations of all data points are shown in **Figure 9-1**. Additional points with a thickness of 5 m were added in the southeast part of the study area where control points were lacking. This thickness is the same as the default thickness as adopted in the Guelph Tier 3 study.

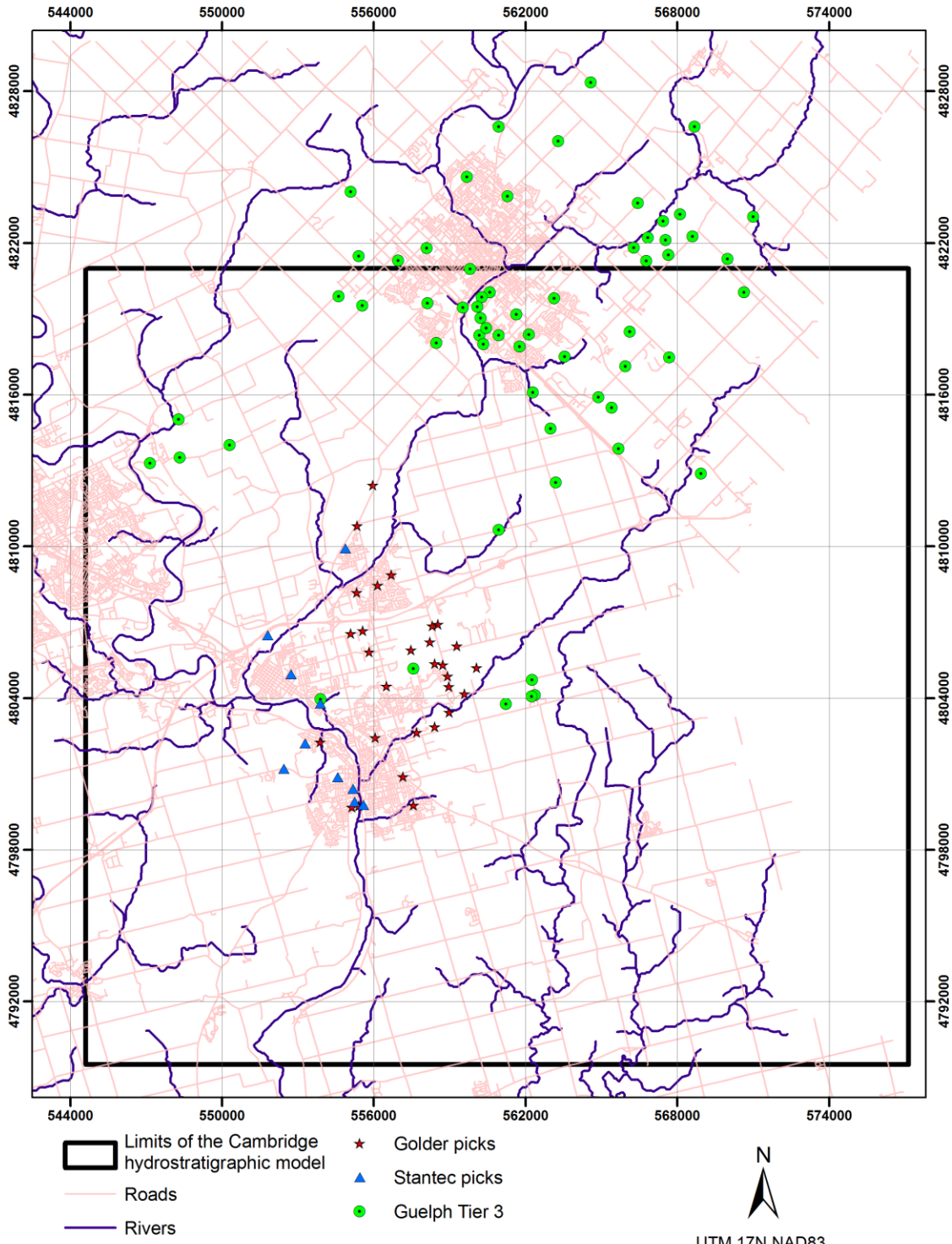
9.3 Methodology and results

A map of the thicknesses of the Goat Island Formation was developed by an Ordinary Kriging interpolation with a linear variogram. The thicknesses ranged from 0 to 55 m. In general, the unit thickened westward, consistent with the general impression of Frank Brunton (personal communication, 2009).

The surface of the top of the Goat Island Formation was calculated by adding its thickness to the surface of the top of the Gasport Formation. The elevations of the top of the Goat Island Formation were adjusted in areas where they were initially calculated to exceed the elevations of the top of the overlying Vinemount Member. This overlay occurred near the eastern boundary of the study area. The final thicknesses of the Goat Island Format are shown in **Figure 9-2**.

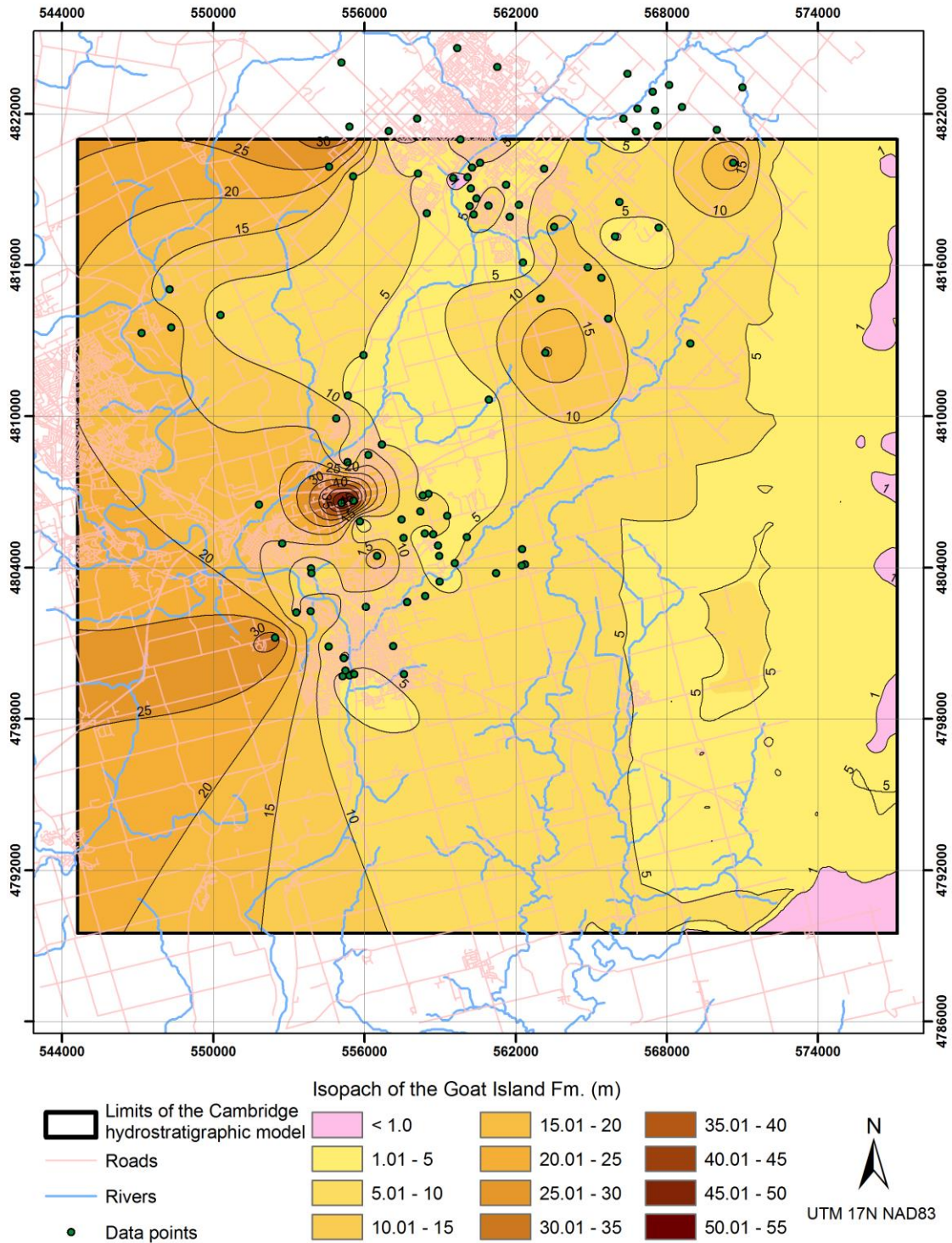
The observed and interpolated thicknesses of the Goat Island Formation are compared in **Figure 9-3**. As shown in the scatterplot, there is a close correspondence between the thicknesses observed in boreholes and the interpolated thicknesses at the same locations. A plot of the cumulative probability distribution of the differences between the observed and interpolated thicknesses is shown in **Figure 9-4**. The differences approximate a straight line when plotted on a normal probability axis, which suggests that the differences are distributed randomly. In general, the differences between the observed and interpolated thicknesses at the locations of the boreholes are relatively small. The mean absolute error in the interpolation is about 0.07 m.

The final top surface of the Goat Island is presented in **Figure 9-5**. In general, the surface dips toward the southwest, which is consistent with the regional geologic setting. To check the calculated surface, the interpolated top elevations of the Goat Island Formation at the locations of borehole picks were compared with the borehole picks. The comparison is presented in **Figure 9-6**. The results shown in the scatterplot confirm that the interpolated elevations match the observed elevations at the boreholes.



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Figure 9-1 Available data points for the thicknesses of the Goat Island Formation



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Figure 9-2 Interpolated thicknesses of the Goat Island Formation

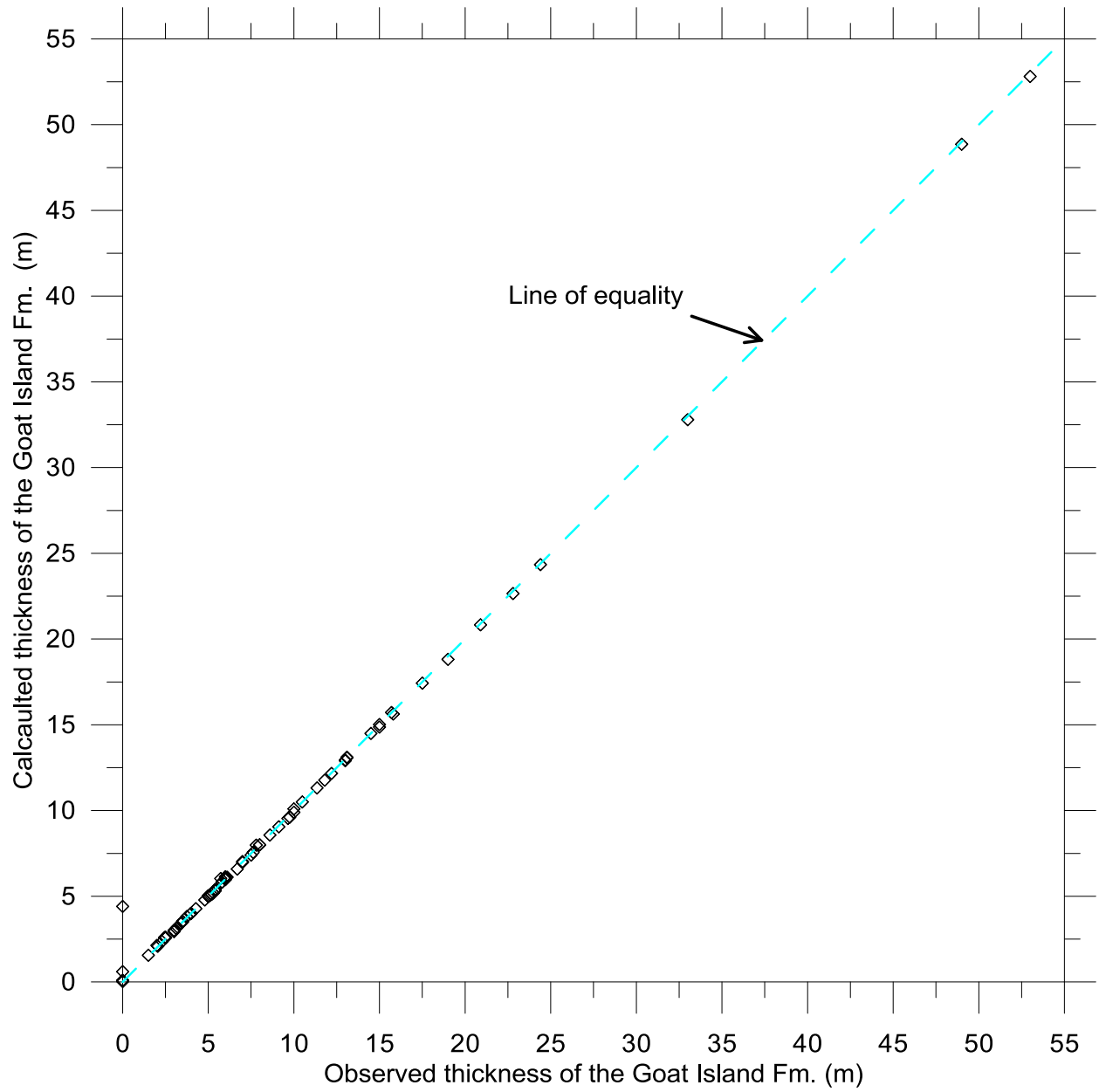


Figure 9-3 Observed and calculated thicknesses of the Goat Island Formation

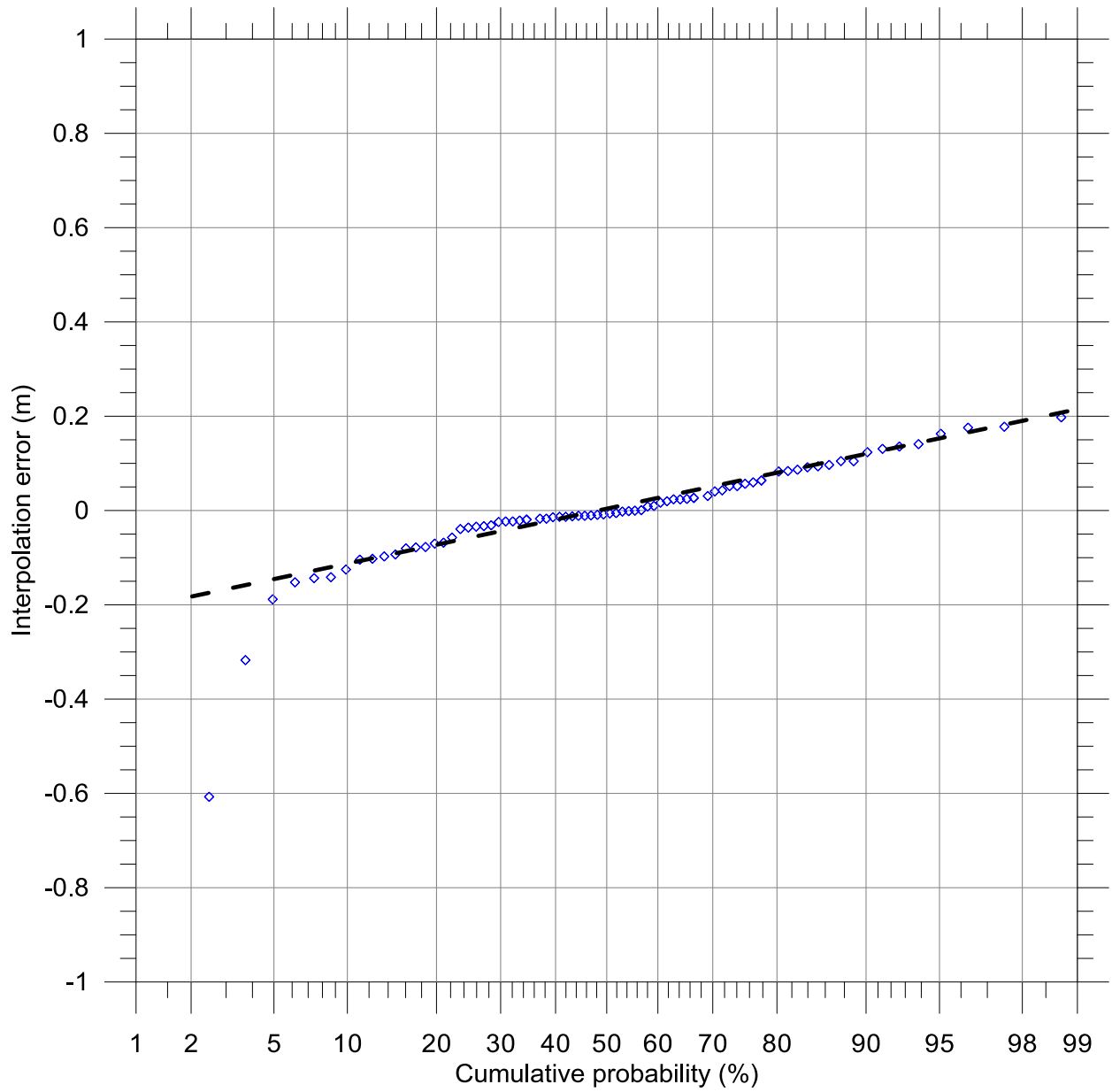
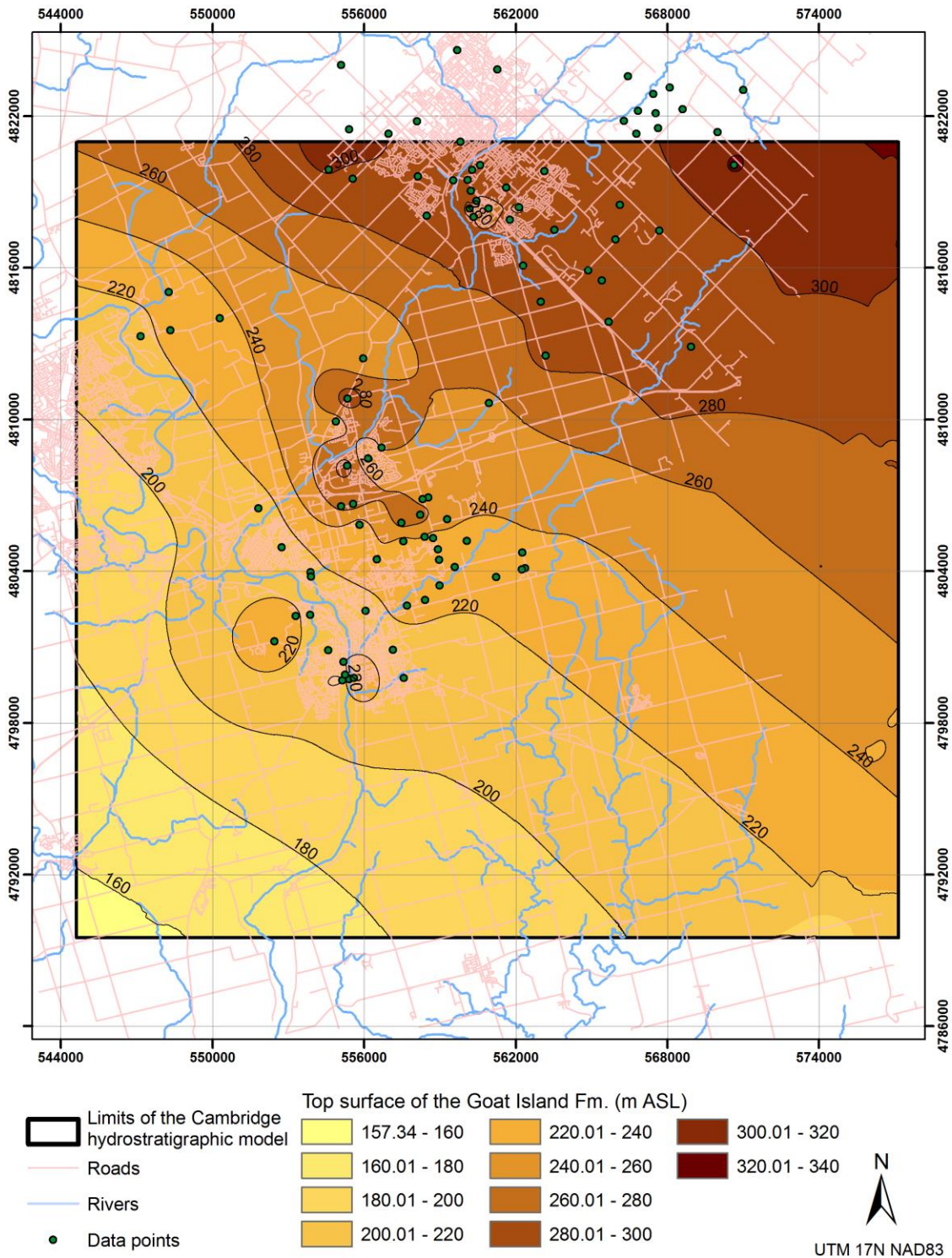


Figure 9-4 Cumulative probability distribution of errors in the interpolated thicknesses of the Goat Island Formation



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Figure 9-5 Interpolated elevations of the top of the Goat Island Formation

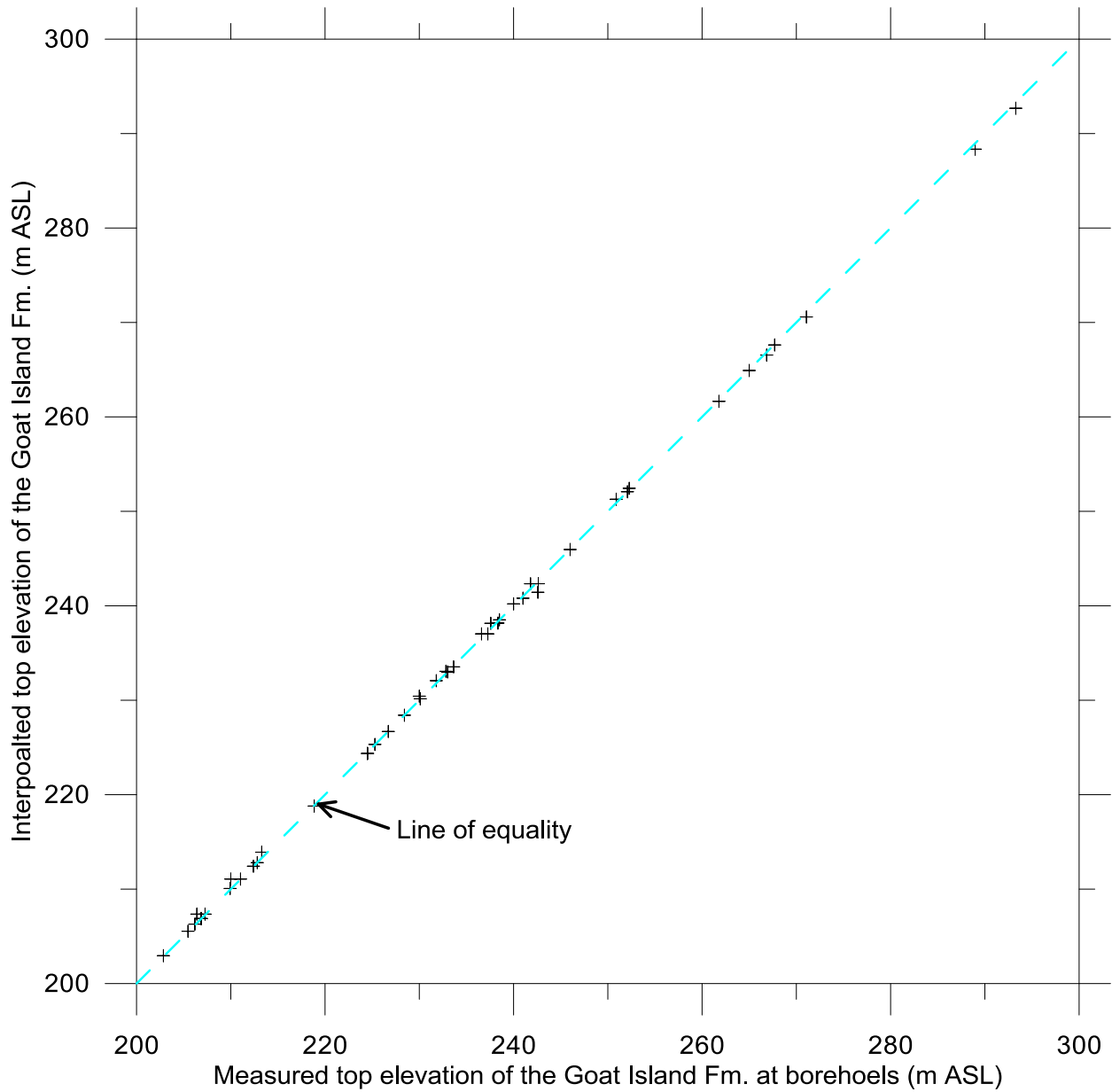


Figure 9-6 Comparison of observed and interpolated elevations of the top of the Goat Island Formation

Section 10

The Gasport Formation

10.1 Data sources

The definition of the Gasport Formation is critical for the development of the Cambridge hydrostratigraphic model. The Gasport Formation and Guelph Formation are the primary bedrock aquifers for the Cambridge area (Golder, 1991). The following data were used to develop the surface of the top of the Gasport Formation:

- Bedrock picks from Golder: 31 points;
- Bedrock picks from Stantec: 9 points; and
- Data points that are used for the Guelph Tier 3 study: 313 points (including 206 artificial control points).

10.2 Data organization and quality control

All data sets were checked and duplicate data points removed based on their reliability and consistency with the surrounding data points. The numbers of data points that were retained from the different sources were:

- Bedrock picks from Golder: 29 points;
- Bedrock picks from Stantec: 9 points; and
- Data points that are used for the Guelph Tier 3 study: 312 points (including 206 artificial control points).

A complete listing of the borehole picks, including an indication of those points that were excluded from the interpretation, is included as part of the metadata for this project. The locations of all data points are presented in **Figure 10-1**. There are relatively large portions of the model area over which there are no well controls for the elevations of the top of the Gasport Formation.

The control points listed above were supplemented with data from well ASRTW6-08, located outside of the model area, and information obtained from the recent drilling of two boreholes in the vicinity of the Middleton Street well field, OW1-09 and OW2-09. The initial development of the top surface of the Gasport Formation did not include information from these two locations. However, in light of their significance, the finalized picks from these two boreholes were incorporated into the complete data set and used to develop the final surface of the top of the Gasport Formation.

Borehole OW2-09 is located in an area where the elevations of the units change abruptly corresponding to the inferred edge of a reef mound in the Gasport Formation. An impression of the predictive capabilities of the geologic model was gained by comparing the elevations of the top of the units predicted prior to the receipt of any information on borehole OW2-09 with the data from the finalized geologic logs that Stantec provided. These elevations are listed on **Table 9-1**. As indicated on the table, the predicted elevations of the bedrock units are relatively close to the geologic logs. A key difference between the initial interpretation at OW2-09 and the data from the geologic log is the thickness of the Vinemount Member. In the initial interpretation, a thickness of 2.72 m was estimated through interpolation. The final geologic log of OW2-09 shows the Vinemount Member is absent at this location. The data from the OW2-09 were used in the development of final thickness of the Vinemount Member.

Table 10-1 Comparison of predicted and observed elevation at OW2-09

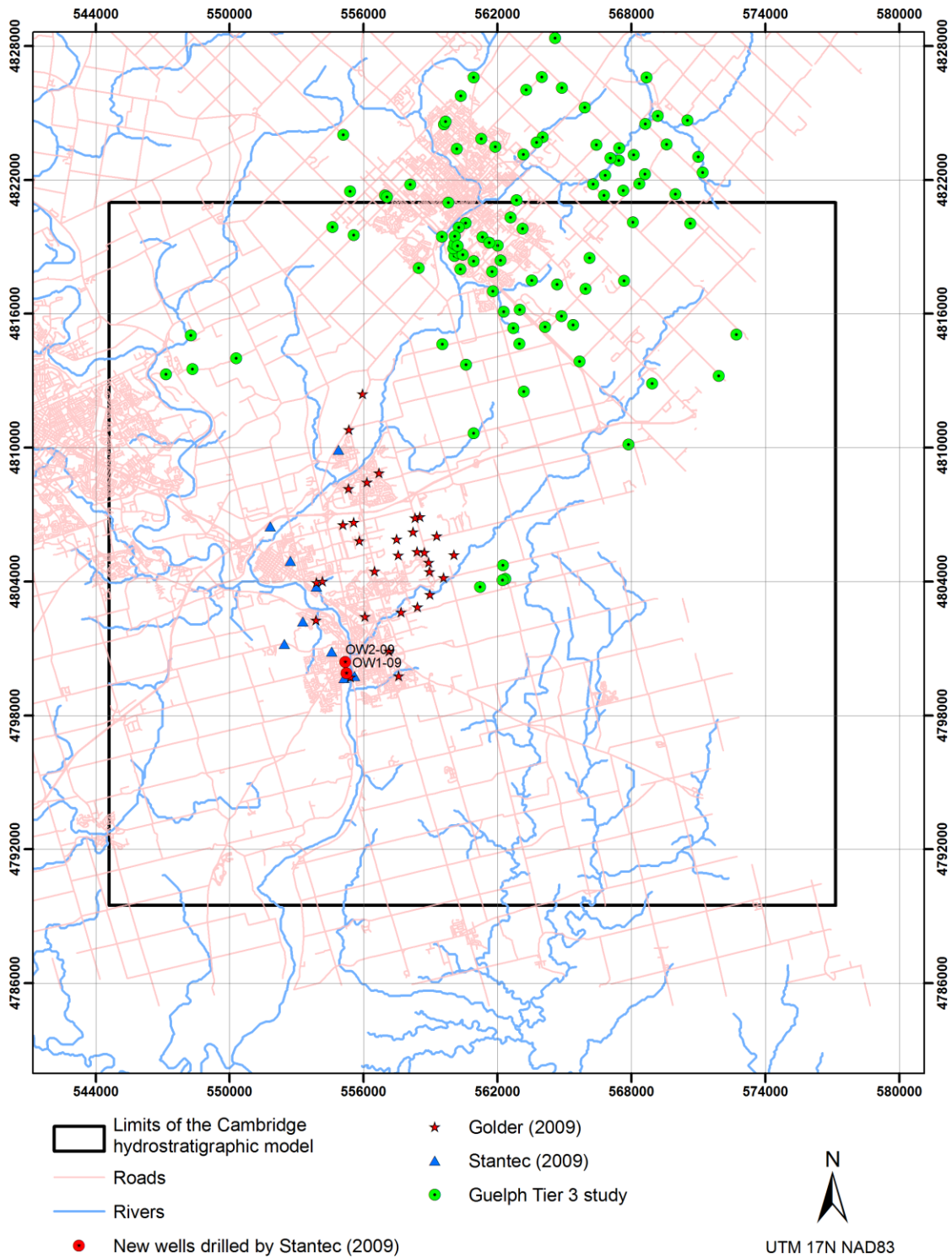
Surface	Predicted elevation at OW2-09 (m ASL)	Observed elevation at OW2-09, from finalized geologic logs (m ASL)
Top of bedrock	267.59	265.28
Top of Reformatory Quarry Mbr.	230.17	226.06
Top of Vinemount Mbr.	215.03	218.85
Top of Goat Island Fm.	212.31	218.52
Top of Gasport Fm.	208.42	207.50

10.3 Methodology and results

The top surface of the Gasport Formation was developed using interpolation with Ordinary Kriging with a linear variogram. To check for cross-cutting, the resulting surface was compared with the final surface of the top of the overlying Goat Island Formation. The elevations at points where the top of the Gasport Formation exceeded the top of the Goat Island Formation were adjusted. This occurred primarily near the eastern boundary of the study area. These locations are beyond the limits of the Cambridge groundwater flow model area, and do not affect the analyses.

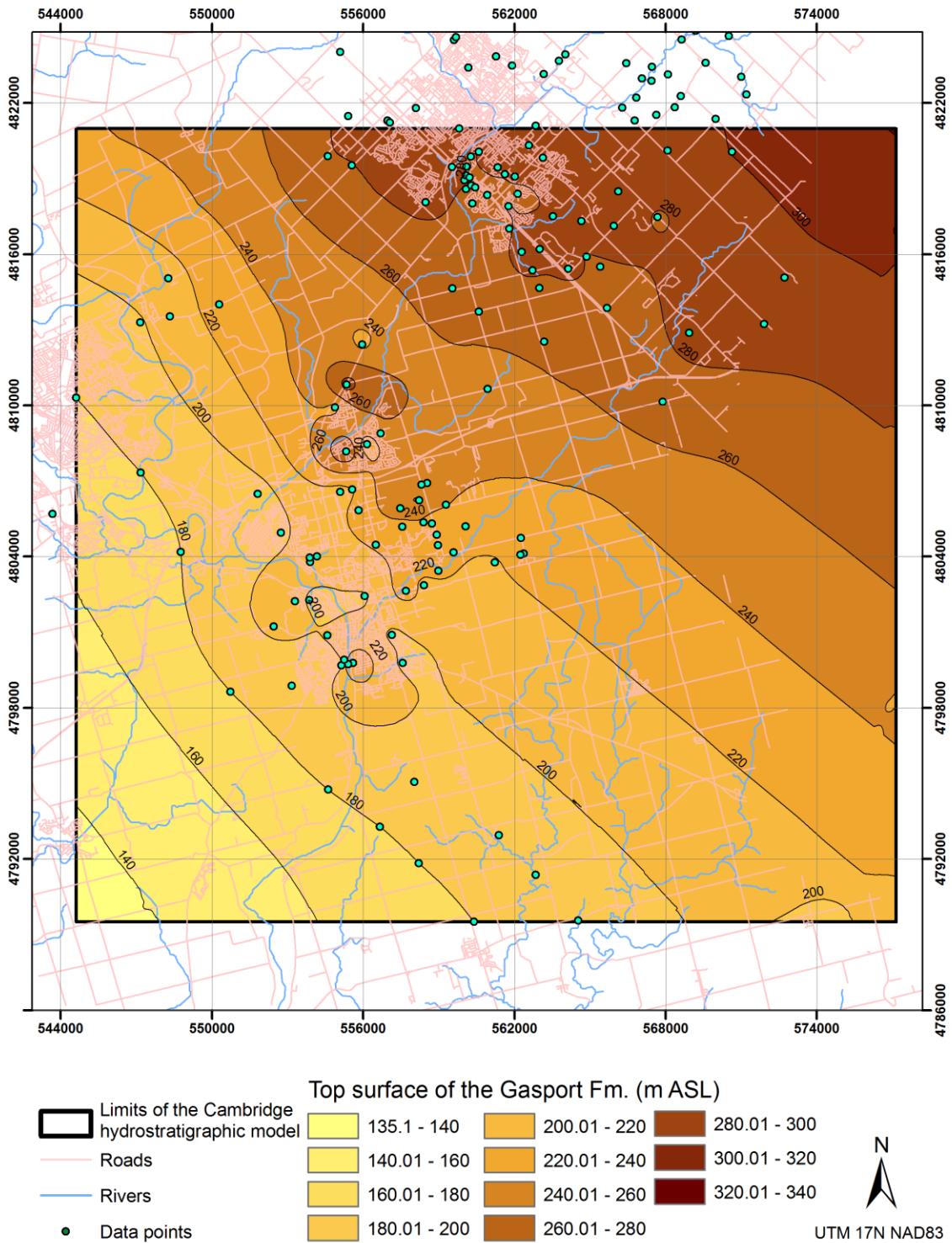
The final top surface of the Gasport Formation is presented in **Figure 10-2**. In general, the surface dips toward the southwest. The contours reveal mounds in the top of the Gasport Formation at the Middleton Street well field and at other locations. The measured and calculated elevations of the top of the Gasport Formation are compared in **Figure 10-3**. The scatterplot confirms the consistency between the calculated and observed surface elevations at the locations of the borehole picks. The cumulative probability distribution of the differences between the interpolated surface and the logged elevations at the boreholes is presented in **Figure 10-4**. The mean absolute difference is 0.18 m. Over 90 % of borehole picks have an absolute difference of less than 0.5 m. The majority of the data points on the cumulative probability distribution plot approximate a straight line, which suggests that the differences are random.

The Cambridge model was simplified slightly by combining the lower portion of the Gasport Formation with the underlying four thin units: Rochester Formation, Irondequoit Formation, the Rockway Formation and Merriton Formation (referred to collectively as the R-I-R-M Formations). Limited information is available for these units, apart from the regional interpretation that they are relatively thin [see for example Golder (1991); Figure 2.6]. The available borehole picks in the study area indicate that the average total thickness of the four units is about 4.5 m. In comparison, the thickness of the Gasport Formation itself ranges from 20 to 100 m. These units have relatively low hydraulic conductivity and are not hydrogeologically significant compared to the Gasport Formation (Golder, 1991; Blair and McFarland, 1992). The thicknesses of the Gasport Formation plus the four underlying units were calculated by subtracting the top surface of the Cabot Head Formation from the final top surface of the Gasport Formation. The resulting thicknesses are shown in **Figure 10-5**.



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Figure 10-1 Available data points for the top of the Gasport Formation



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Figure 10-2 Interpolated surface of the top of the Gasport Formation

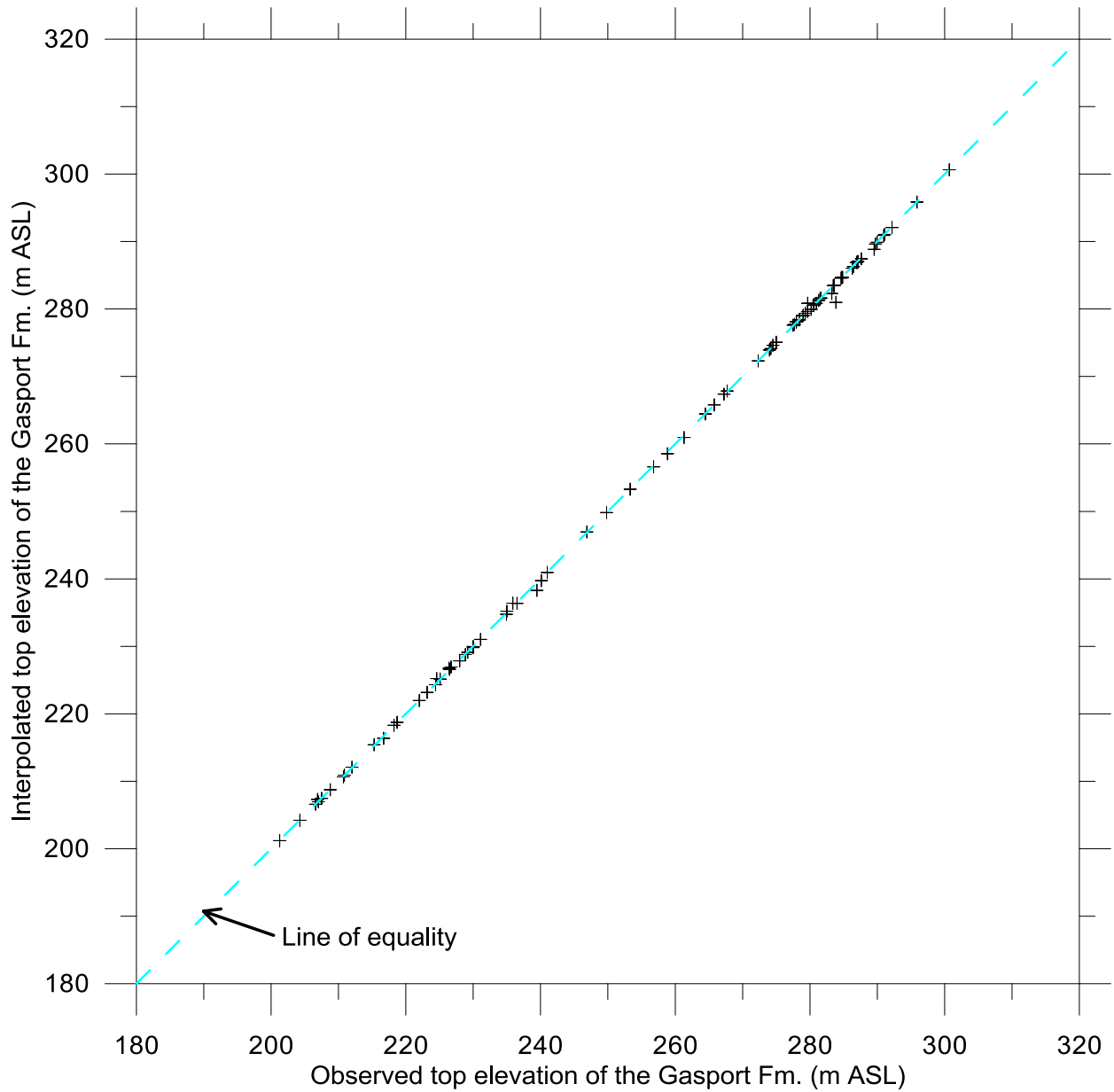


Figure 10-3 Comparison of observed and calculated top elevations of the Gasport Formation

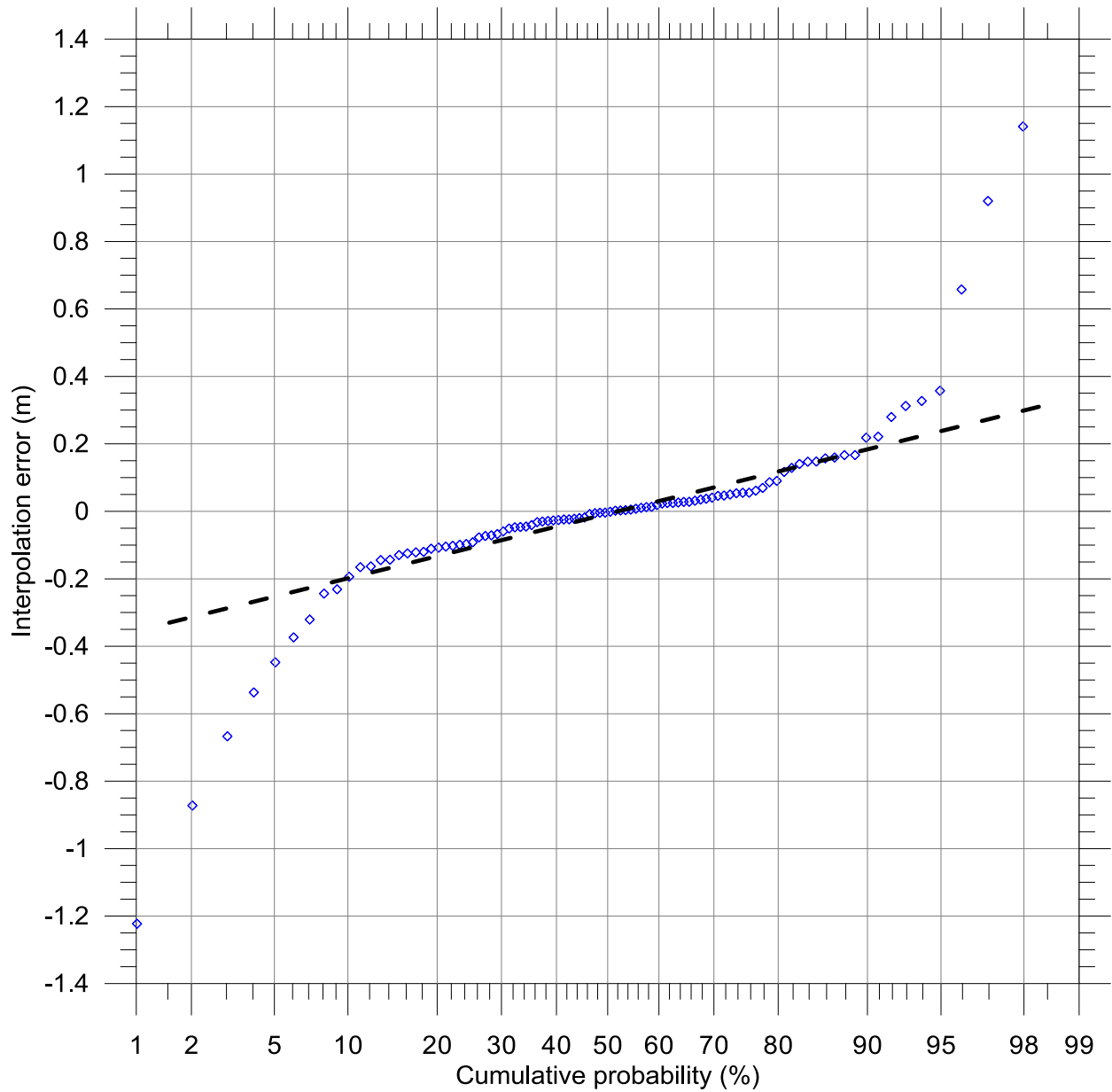
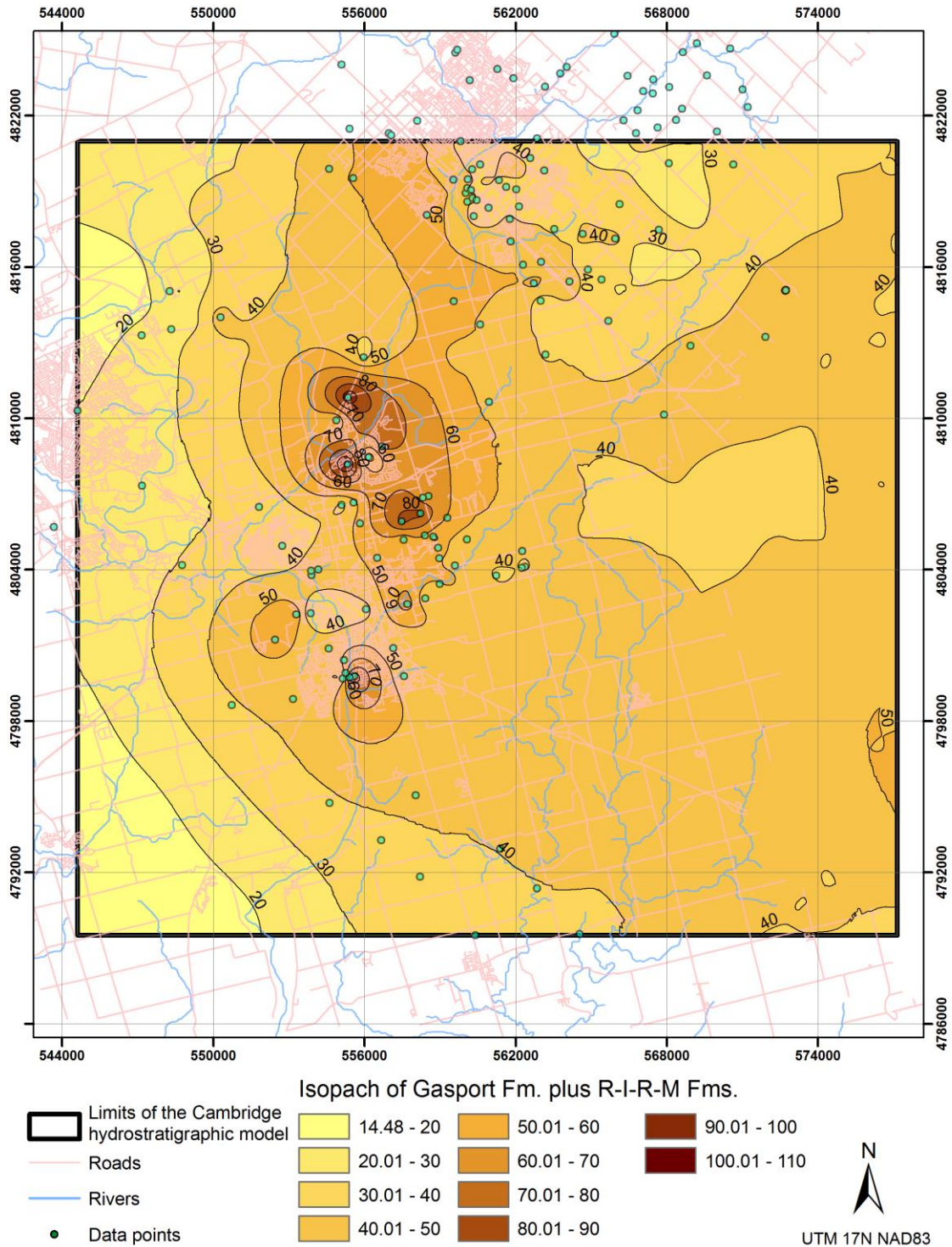


Figure 10-4 Cumulative probability distribution of differences between the interpolated and observed top elevations of the Gasport Formation



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Figure 10-5 Thicknesses of the Gasport Formation plus R-I-R-M Formations

Section 11

Potential production zone in the Gasport Formation in the Cambridge area

11.1 Introduction

In the Guelph area, a zone of high transmissivity has been observed in the middle of the Gasport Formation. This zone is referred to as the Guelph Production Zone, and is a feature being targeted for additional high-capacity production wells (Brunton et al., 2007). The evidence for a continuous zone of elevated transmissivity in the middle of the Gasport Formation in the Cambridge area is not clear cut. A review of borehole flowmeter profiles across the Cambridge area suggests that there are distinct zones of concentrated flow, but they are not aligned in a single zone.

To afford flexibility in the assignment of the properties of the Gasport Formation, the Gasport Formation was subdivided into three layers in the Cambridge model. The delineation of the second layer followed an approach similar to that adopted to represent the Gasport Production Zone in the Guelph area. In this section, development of the top and bottom surfaces of the Production Zone of the Gasport Formation in the Cambridge area is described.

11.2 The “Guelph Production Zone”

Based on borehole logs and testing in the Guelph area, Golder Associates suggested that the Gasport Production Zone could be idealized as a layer with a uniform dip toward the southwest and an average thickness of about 12 m. The thickness of the Gasport Production Zone decreases close to the Niagara Escarpment, as the Gasport Formation pinches out. The thickness of the Gasport Production Zone in the Guelph Tier 3 model is shown in **Figure 11-1**. Linear regression using elevations from 3 selected locations of the top of the Gasport Production Zone in the Guelph Tier 3 model revealed that the elevations of the top of the Gasport Production Zone can be approximated with the following plane:

$$Z = 2.74 \times 10^{-3} \cdot X + 3.3 \times 10^{-3} \cdot Y - 17166.01 \quad (1)$$

Where Z is the elevation of the top of the Gasport Production Zone in metres above mean sea level, and X and Y are the UTM Easting and Northing coordinates (m).

A similar linear regression yielded the following equation for the bottom of the Production Zone in the Guelph Tier 3 model:

$$Z = 2.74 \times 10^{-3} \cdot X + 3.3 \times 10^{-3} \cdot Y - 17178.57 \quad (2)$$

The difference between the two estimated surfaces was 12.6 m, which was consistent with the previous suggestion of an average thickness of about 12 m. The difference was likely due to round-off error. The elevations of the top and bottom of the Gasport Production Zone retrieved from the Guelph model are compared in **Figure 11-2** and **Figure 11-3** with the elevations calculated using Equations (1) and (2). In general, the equations match closely the elevations specified in the Guelph Tier 3 model.

11.3 Layer representing a potential production zone in Cambridge

Golder Associates summarized the results of flow profiling conducted in 2009 at 27 bedrock boreholes from four well fields in the eastern portion of the Cambridge East area. The inferred elevations of the top and bottom of the zone of major inflow in the Gasport Formation are summarized on **Table 11-1**. Strong flows were identified in the middle portion of the Gasport Formation at 12 of the 27 boreholes. The average thickness of the Production Zone in the Gasport Formation inferred from the profiling is about 14 m, about 2 m greater than the thickness assumed for the Gasport Production Zone in the Guelph Tier 3 model.

Table 11-1 Top and bottom elevations of the Production Zone within the Gasport Formation at selected boreholes in Cambridge based on flow profiling (data from Golder Associates)

Borehole ID	UTM Easting	UTM Northing	Elevations of the Top of the Production Zone (m ASL) ¹	Elevations of the bottom of the Production Zone (m ASL) ¹
H3-TW1-09	555322.60	4808181.60	208.9	198.9
H4-TW1-08	556687.00	4808883.00	220.8	200.8
H5-TW1-09	555342.70	4810824.40	210.8	207.8
OW1-95	554877.00	4809921.00	207.9	203.9
OW2-95	556154.00	4808464.00	215.6	201.6
PBPW2-08	557478.20	4805910.80	218.8	203.8
PBPW1-06	558217.20	4806229.10	213.2	198.2
CMPW2-06	558967.10	4804450.60	203.6	189.6
CMPW1-06	558400.00	4805353.20	216.9	201.9
SMTW2-07	557688.70	4802637.60	198.6	183.6
SMTW4-08	558980.00	4803436.00	211.6	191.6
SM3-93	557938.00	4803409.80	206.0	186.0

1. The elevations were calculated by subtracting the depths of the flow interval in the middle Gasport Formation from the ground surface elevations. The depths of the flow intervals were taken from Table E3 of Golder Associates (2010)

In the Cambridge model, three model layers (layers 13, 14, and 15) are used to represent the Gasport Formation and the four formations between the Gasport Formation and the Cabot Head Formation. Layer 14 is included to accommodate a Production Zone in the middle of the Gasport Formation.

As a first approximation, it was assumed that the Gasport Production Zone in the Cambridge area coincided with the zone in the Guelph Tier 3 model and that regression equation (1) could be used to approximate its top. The bottom surface was developed using Equation (2) with a change of the intercept to ensure an average thickness of 14 m. Both surfaces were then compared with the elevations of the tops of the Gasport Formation and Cabot Head Formation. The comparison showed that the initial top surface of the Production Zone was higher than the top of the Gasport Formation in the mid-west portion of the model, and that the bottom surface was lower than the top of the Cabot Head Formation at the south end of the model. Adjustments were needed to the top and bottom surfaces to eliminate cross-cutting with the neighboring model layers. The adjustments to the initial surfaces were based on the following assumptions:

- The lower Gasport Formation (model layer 15) has a minimum thickness of 4.5 m. This thickness is the average thickness of the four intermediate formations between the Gasport Formation and the Cabot Head Formation estimated from the borehole logs; and
- The upper Gasport Formation (model layer 13) has a minimum thickness of 2 m. If the initial top of the Production Zone exceeds the top of the Gasport Formation, it is pushed down 2 m below the top of the Gasport Formation. This ensures that the model layer is continuous even in the western portion of the model where the Gasport Formation is thinner.

The final interpolated top and bottom elevations of the Production Zone at the locations of the 12 boreholes listed on **Table 11-1** are listed on **Table 11-2**. The results shown on **Table 11-2** confirm that the majority of the high productivity intervals in the Gasport Formation fall within model layer 14.

Table 11-2 Comparison of top and bottom elevations of the Production Zone within the Gasport Formation at selected boreholes

Borehole ID	UTM Easting	UTM Northing	Elevations of the Top of the Production Zone (m ASL) ¹	Elevations of the bottom of the Production Zone (m ASL) ¹	Top of the model layer 14 (m ASL) ²	Bottom of model layer 14 (m ASL) ³
H3-TW1-09	555322.60	4808181.60	208.9	198.9	211.91	197.91
H4-TW1-08	556687.00	4808883.00	220.8	200.8	217.97	203.97
H5-TW1-09	555342.70	4810824.40	210.8	207.8	220.68	206.68
OW1-95	554877.00	4809921.00	207.9	203.9	216.43	202.43
OW2-95	556154.00	4808464.00	215.6	201.6	215.12	201.12
PBPW2-08	557478.20	4805910.80	218.8	203.8	210.34	196.34
PBPW1-06	558217.20	4806229.10	213.2	198.2	213.41	199.41
CMPW2-06	558967.10	4804450.60	203.6	189.6	209.60	195.60
CMPW1-06	558400.00	4805353.20	216.9	201.9	211.02	197.02
SMTW2-07	557688.70	4802637.60	198.6	183.6	200.12	186.12
SMTW4-08	558980.00	4803436.00	211.6	191.6	206.29	192.29
SM3-93	557938.00	4803409.80	206.0	186.0	203.35	189.35

1. From Table 1, based on flow profiling data from Golder Associates.

2. Calculated with Equation (1).

3. Elevation of top of model layer 14 minus 14 m.

Limited data from hydraulic testing were available to test the hypothesis that there is a production zone in the Gasport Formation elsewhere in the Cambridge model area. Packer testing was conducted at 6 wells (BH-1, BH-2A, BH-2B, BH-3A, BH-4, and BH-5) in the Middleton Street well field in 1995 (Beak et al., 1995). Only the tests in BH-1 extended across the middle and lower part of the Gasport Formation; the remainder of the tests were conducted only over the upper part of the Gasport Formation (BH-2B, BH-3) or were above the Gasport Formation (BH-2A, BH-4 and BH-5).

The results of the testing in BH-1 are plotted in **Figure 11-4**. The results suggest highly transmissive zones within the Gasport Formation at depths of 86 and 101 m below the top of casing. In the absence of detailed information, it was assumed that the reported depths are from ground surface (some errors were expected in doing so, but it should be less than about 1 m). Using the interpolated ground surface elevation at BH-1 from the DEM, the highly transmissive interval in the Gasport Formation was estimated to extend from 162.8 to 178.7 m ASL (**Figure 11-4**). The largest part of this interval falls within the Gasport Production Zone in the Cambridge model (168.9 -182.9 m ASL).

The final top and bottom elevations of model layer 14 representing the Production Zone in the Gasport Formation are presented in **Figure 11-5** and **Figure 11-6**, respectively. The cross-sections near the Middleton Street well field are presented in **Figure 11-7** and **Figure 11-8**. The inferred reef mound in the Gasport Formation is evident in these figures.

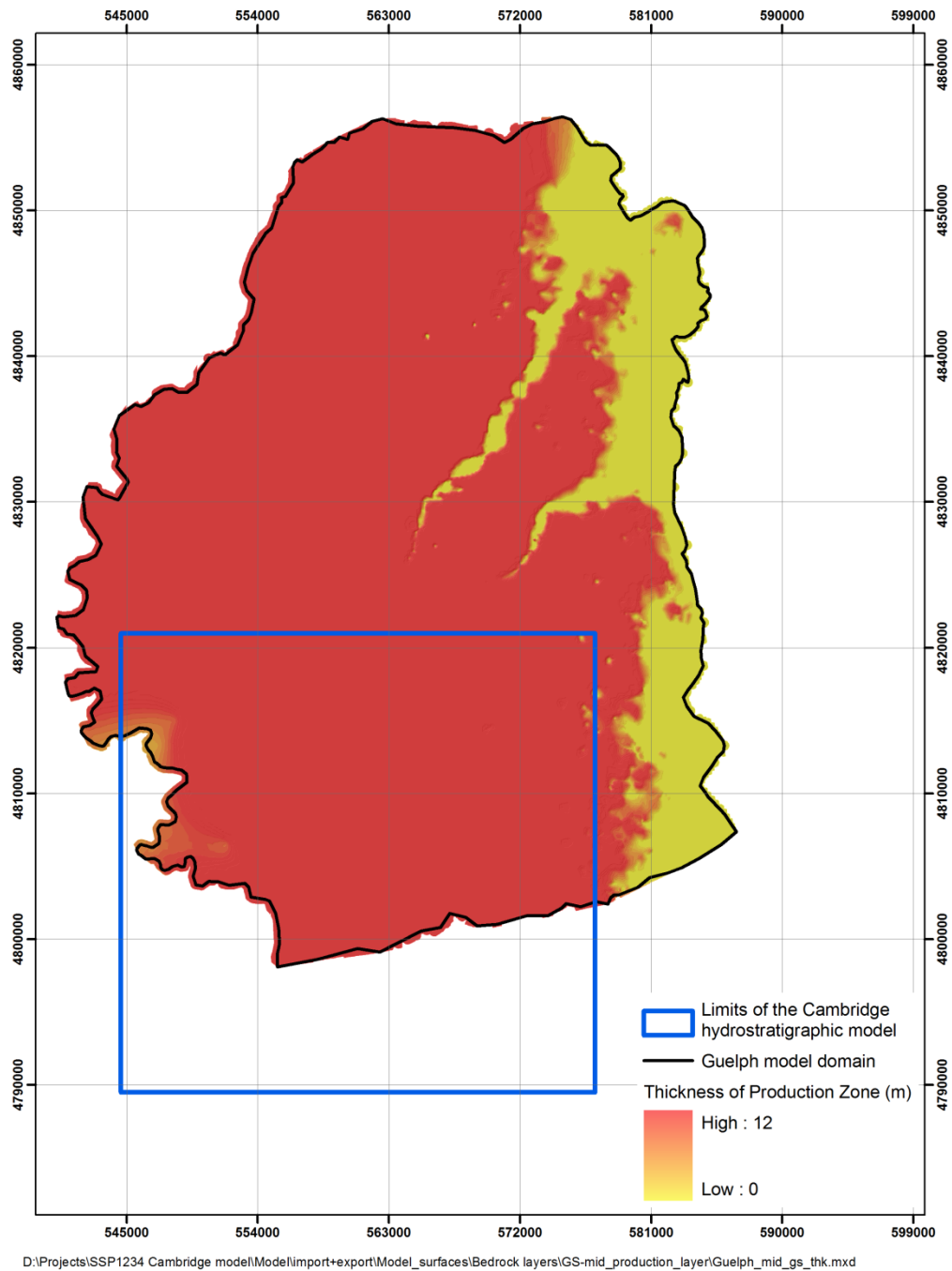
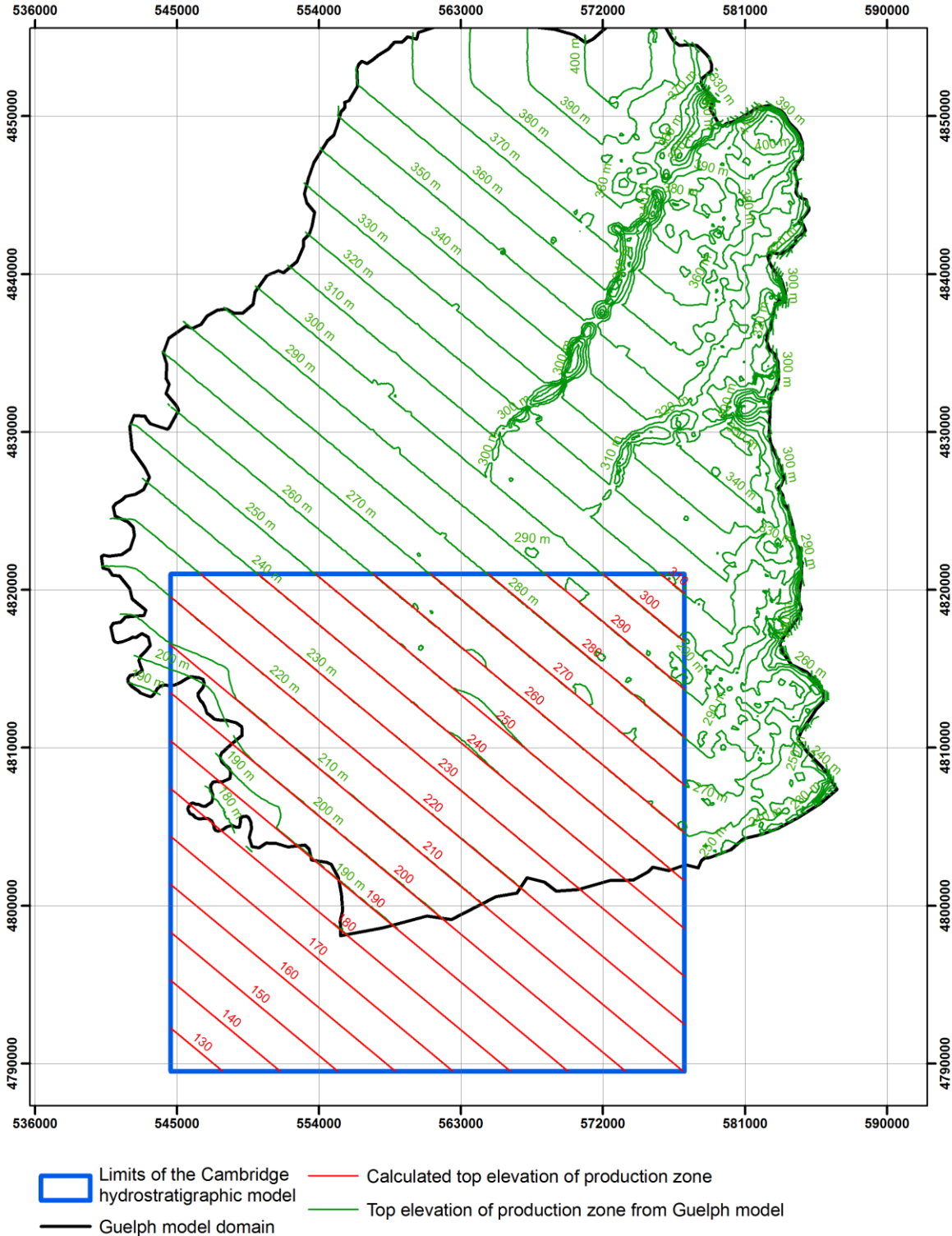
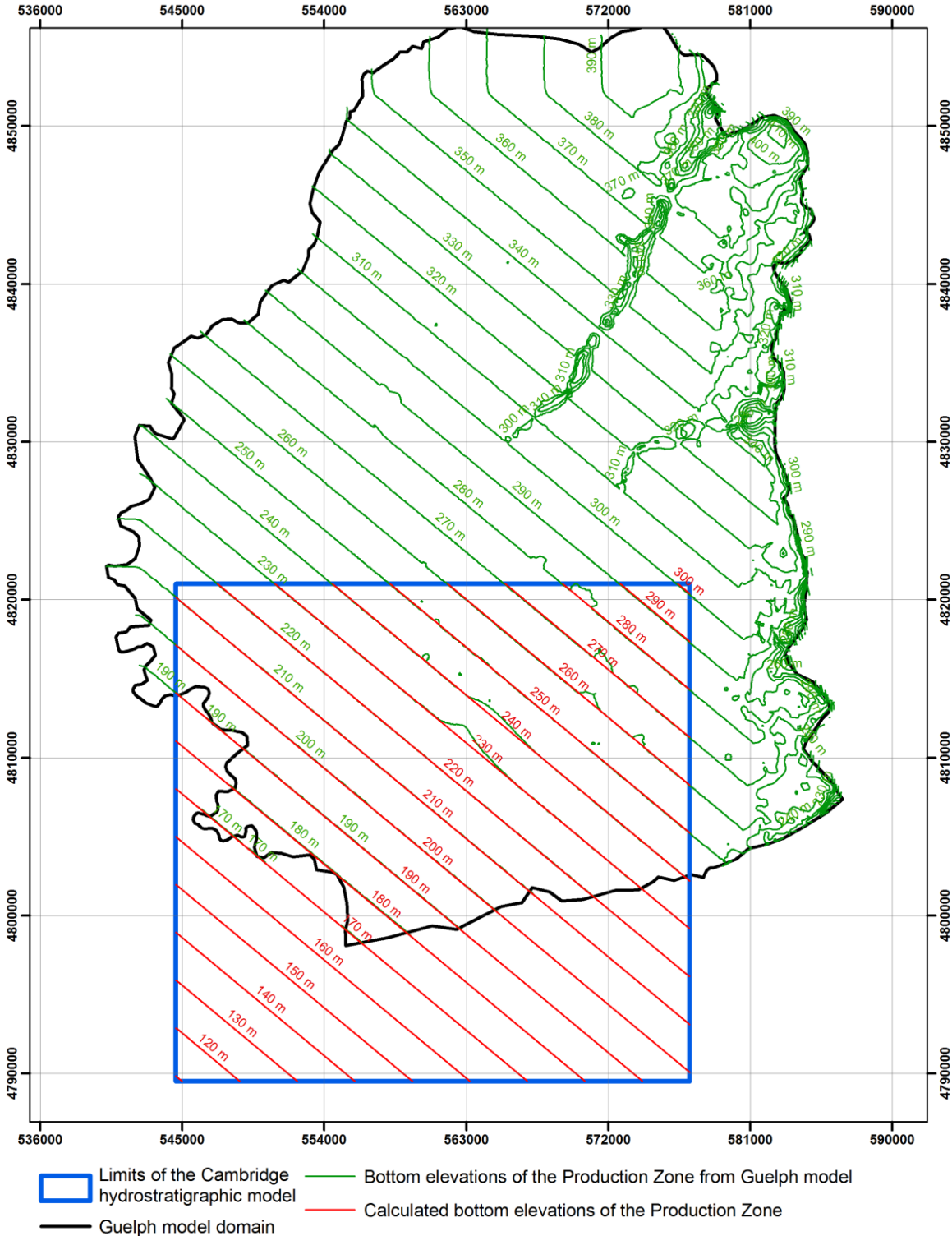


Figure 11-1 Thickness of the Gasport Production Zone in the Guelph Tier 3 Model



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Figure 11-2 Elevations of the top of the Gasport Production Zone from the Guelph Tier 3 model



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Figure 11-3 Elevations of the bottom of the Gasport Production Zone from the Guelph Tier 3 model

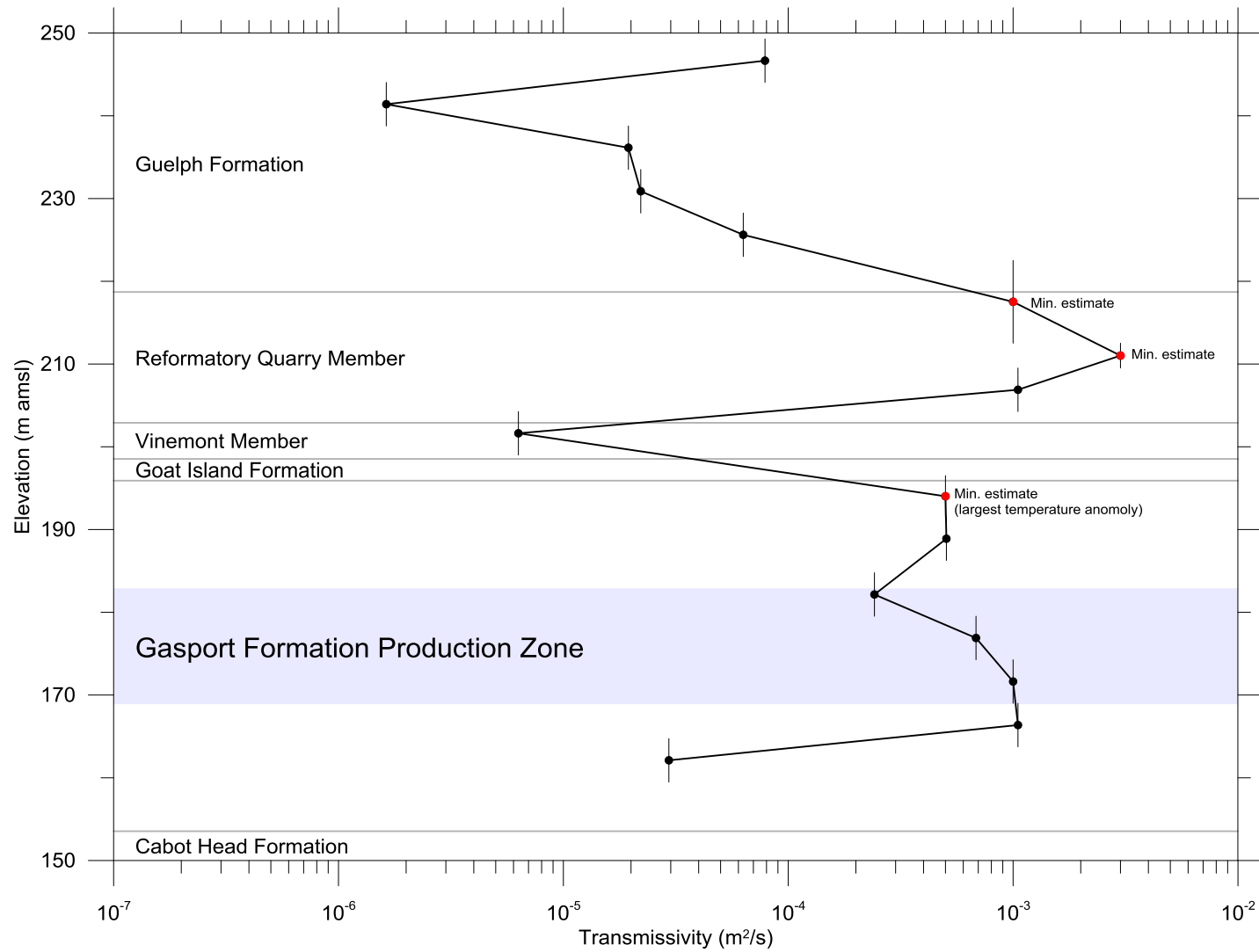
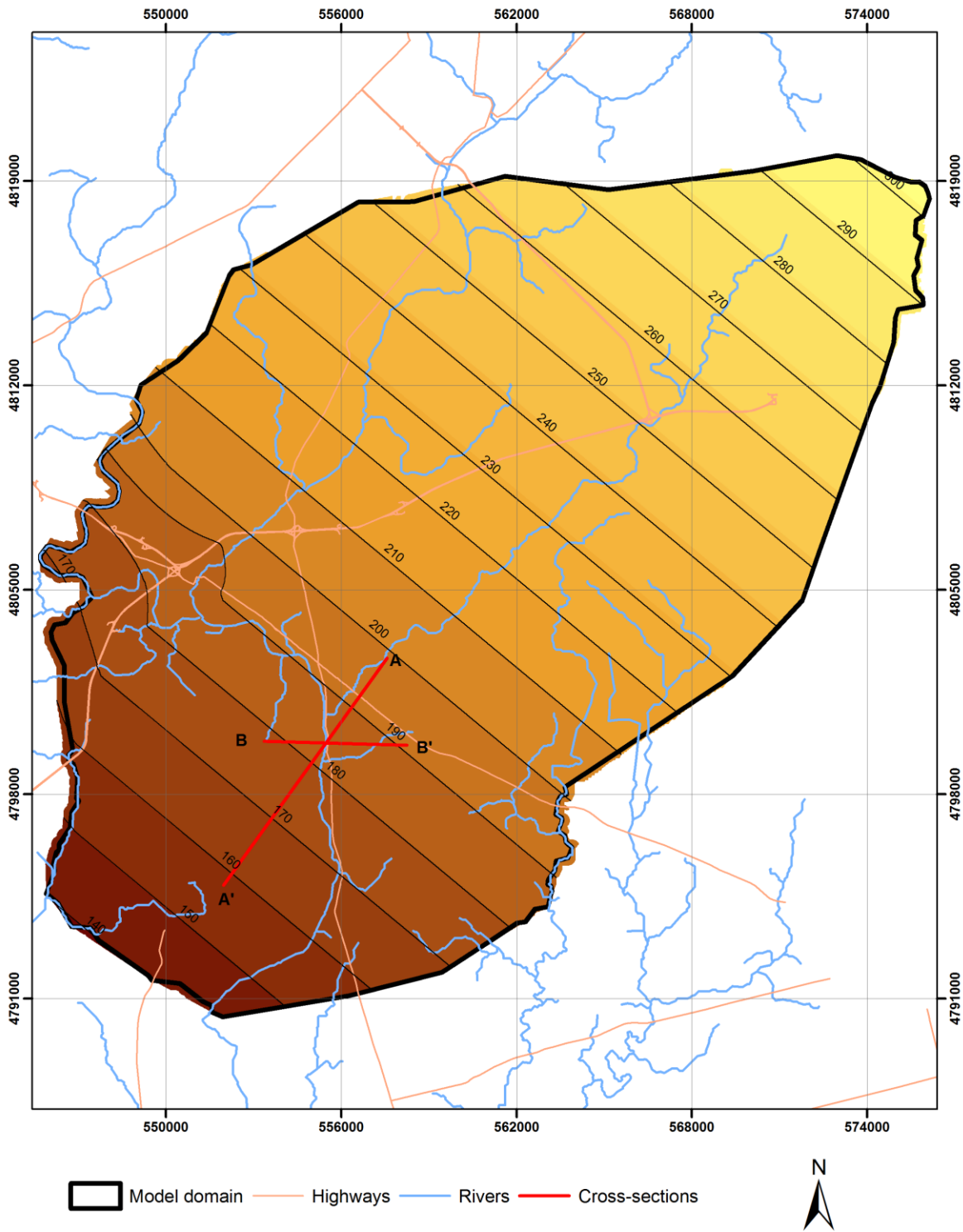
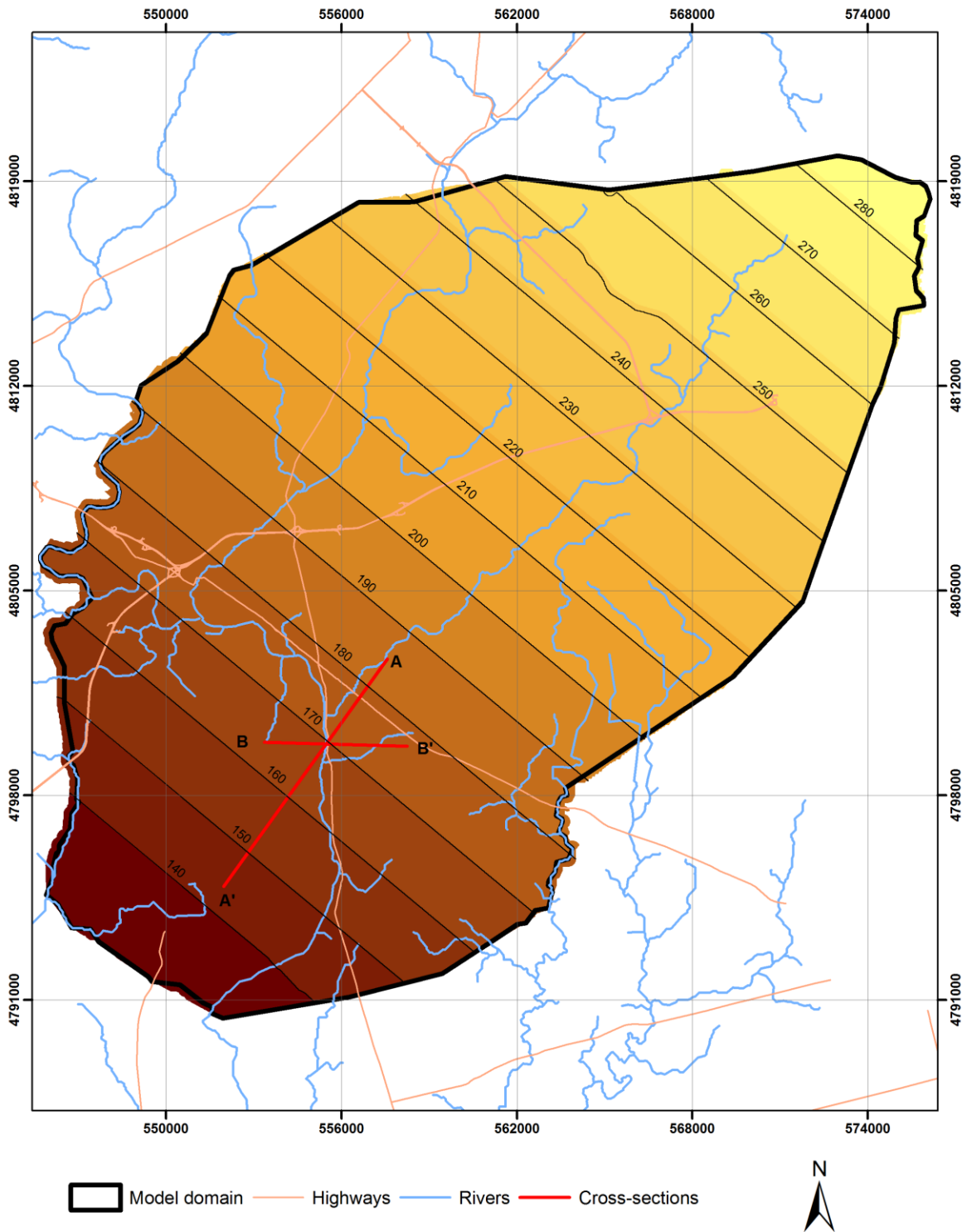


Figure 11-4 Packer testing results at BH-1, Middleton Street well field (data from Beak et al., 1995)



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Figure 11-5 Final elevations of the top of the Gasport Production Zone (model layer 14)



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Figure 11-6 Elevations of the bottom of the Gasport Production Zone (model layer 14)

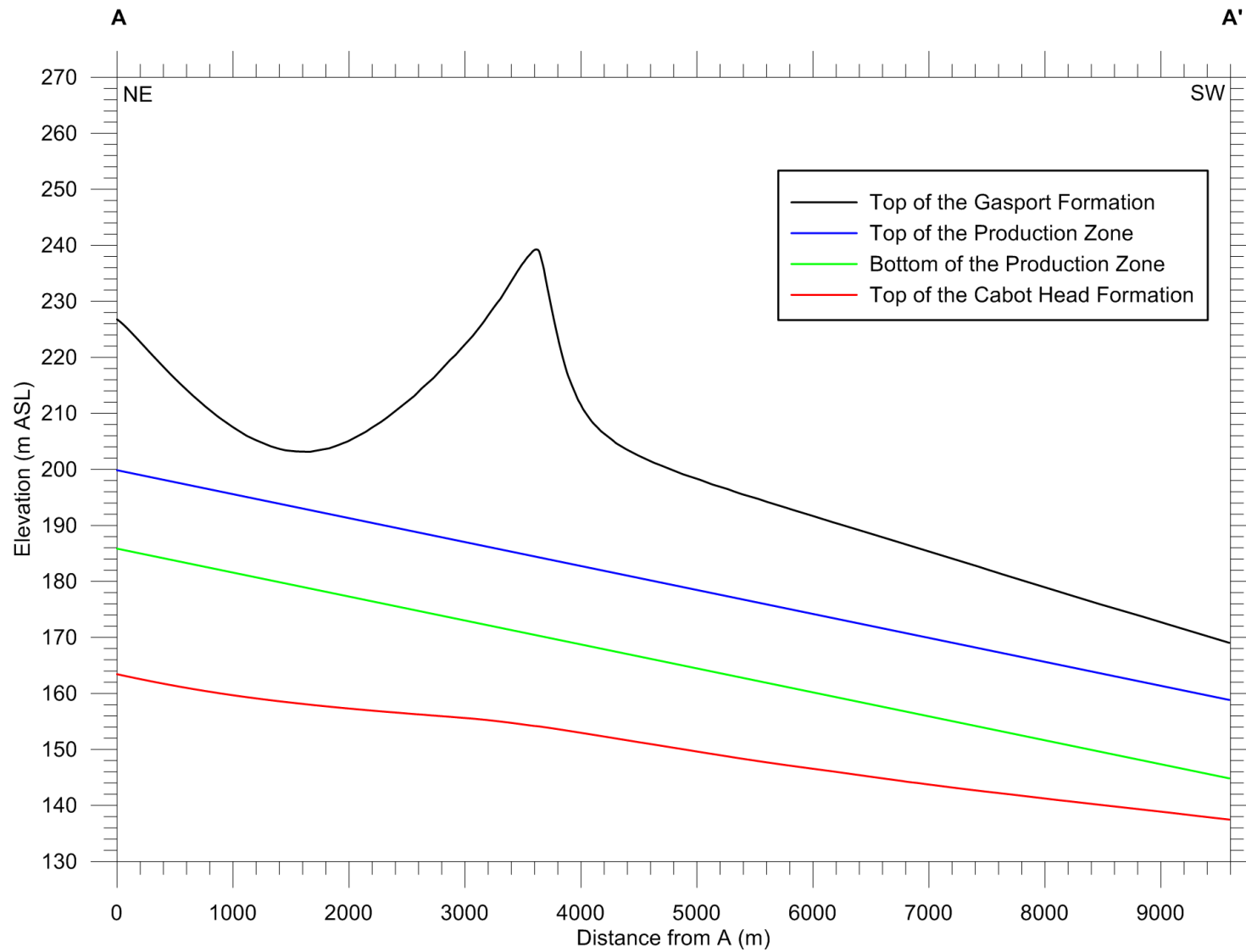


Figure 11-7 Cross section A-A' through the Middleton Street well field

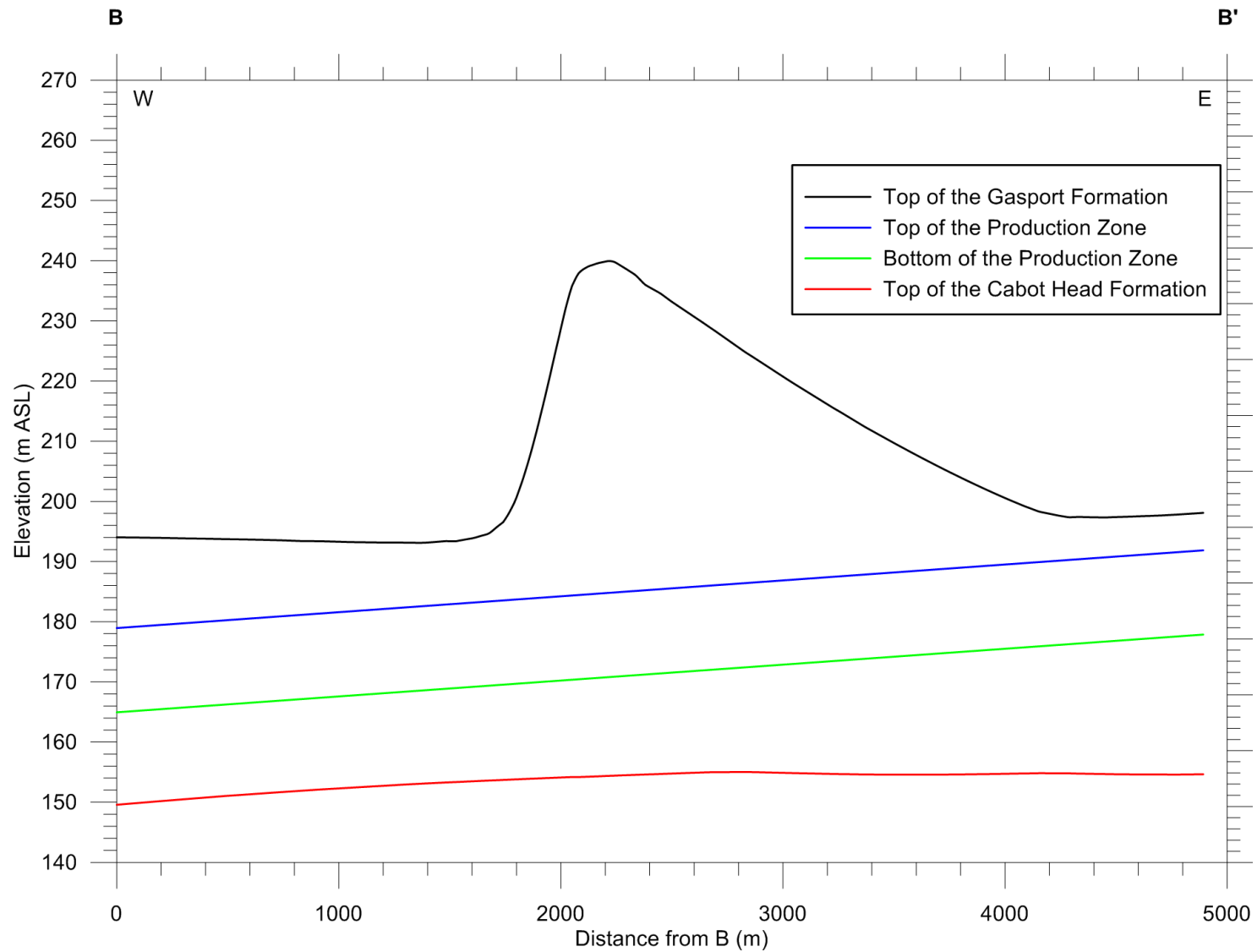


Figure 11-8 Cross section B-B' through the Middleton Street well field

Section 12

The Cabot Head Formation

12.1 Significance of the Cabot Head Formation

The top of the Cabot Head Formation has typically been treated as the lower limit of significant groundwater flow in the Cambridge area (Golder, 1991; Beak et al., 1995; Lotowater, 1997; Duke, 1998; Stantec, 2007; Stantec, 2011a,b; Golder, 2009b,c, 2011a). The results of hydraulic tests conducted in the Cabot Head Formation at various locations are listed on **Table 12-1**. The hydraulic conductivity values range from 1.9×10^{-7} to 5.6×10^{-4} cm/s; these values are two orders of magnitude or less than the hydraulic conductivity values of the Gasport Formation. The data compiled on **Table 12-1** provide support for setting the top of the Cabot Head Formation as the bottom of the Cambridge hydrostratigraphic model.

Table 12-1 Summary of hydraulic conductivity values for the Cabot Head Formation

Source	Mean hydraulic conductivity (cm/s)	Comments
Middleton St well field (Beak et al., 1995)	5.6×10^{-4}	Packer test at BH-1, 2 order of magnitude lower than the overlaying formation
Proposed Flamborough Quarry (SSPA, 2009)	9.1×10^{-7}	Packer test at MWB4-II for a interval that cross the interface between the Cabot Head Formation and the Amabel Formation
	2.5×10^{-5}	Packer test at MWB7-II for a interval that cross the interface between the Cabot Head Formation and Fossil Hill Formation
Acton Quarry (CRA, 2008)	5.0×10^{-5}	Packer test at OW1-86
	$< 3.2 \times 10^{-6}$	Packer test at OW2-86
	4.5×10^{-5}	Packer test at OW3-86
	$< 1.3 \times 10^{-6}$	Packer test at OW4-86
	1.7×10^{-5}	Packer test at OW5-86
	$2.7 \times 10^{-6}, 8.3 \times 10^{-7}$	Packer test at OW22a
	7.5×10^{-6}	Packer test at OW1C1
1.9×10^{-7}	Packer test at OW1C2	

12.2 Data sources, organization and quality control

The following data were available for the development of the top surface of the Cabot Head formation:

- Bedrock picks from Golder: 30 points;
- Bedrock picks from Stantec: 1 point; and
- Data points from the Guelph Tier 3 study: 143 points (including 55 artificial control points).

All data sets were checked and duplicate data points removed based on their reliability and consistency with the surrounding data points. After the checking was completed, the numbers of data points retained from the different sources were:

- Bedrock picks from Golder: 30 points;
- Bedrock picks from Stantec: no points; and
- Data points from the Guelph Tier 3 study: 143 points (including 55 artificial control points).

Data from the borehole ASRTW6-08 were included for control on the top of the Cabot Head Formation in the western part of the study area.

A complete listing of the borehole picks, including an indication of those points that were excluded from the interpretation, are included as part of the metadata for this project.

The locations of the control points for defining the surface of the top of the Cabot Head Formation are shown in **Figure 12-1**. As shown in the figure, there is relatively sparse coverage in the Cambridge area. Many of the control points developed for the Guelph Tier 3 study are a significant distance from the Cambridge model area.

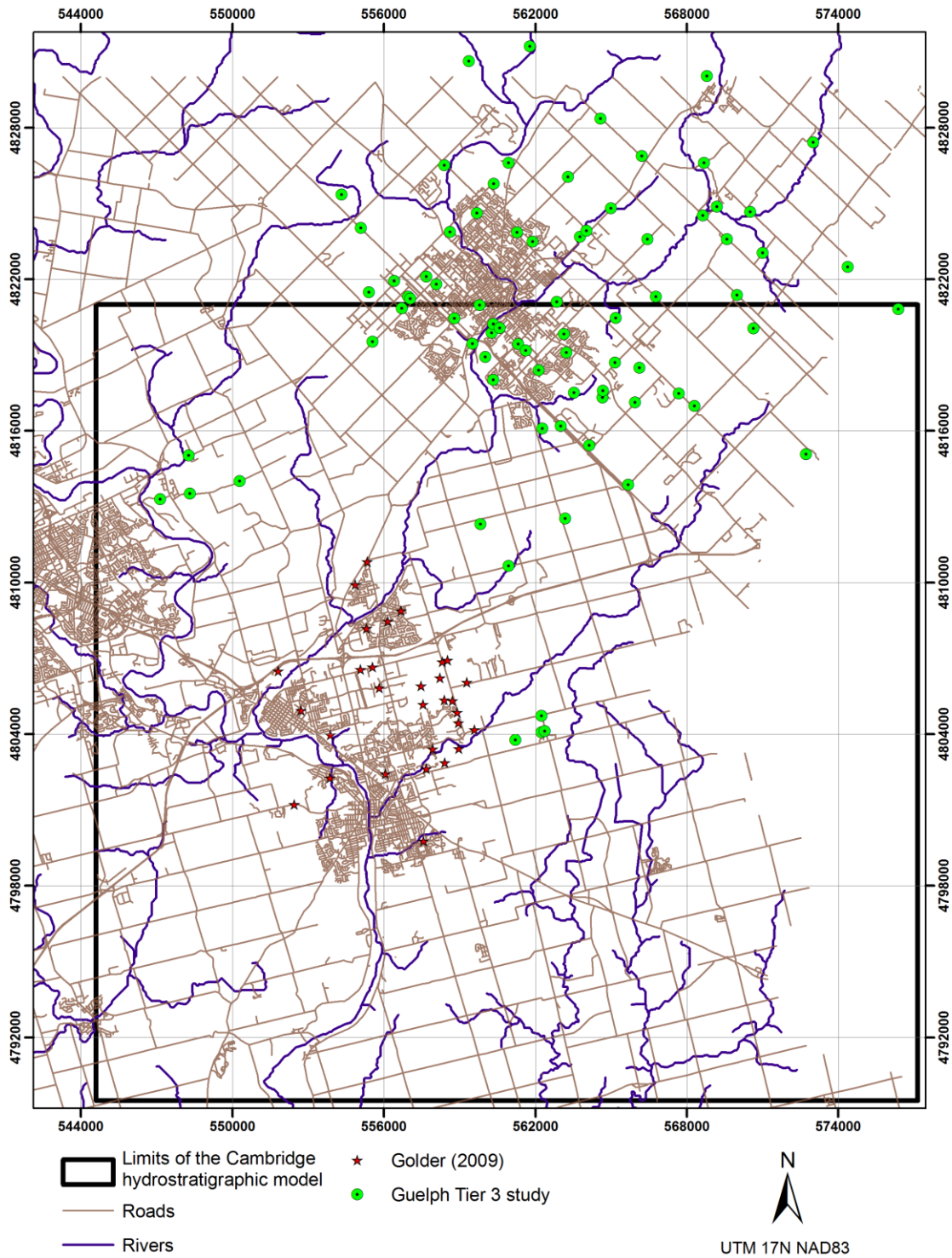
As part of their groundwater protection study for the township of Puslinch, Golder Associates developed an interpretation of the bottom surface of the Amabel Formation (Golder, 2006). To incorporate additional control, the contours from the Golder interpretation were digitized and incorporated into this analysis.

Brunton (personal communication, 2009) has urged caution when interpreting maps of the bottom of the Amabel Formation. Problems may arise because the bottom of the Amabel has not always been interpreted correctly in previous investigations. Brunton indicated in his revisions to the Silurian stratigraphy of southern Ontario that the bottom of the Amabel Formation is correctly interpreted as the bottom of the Gasport Formation. He suggested that several previous investigations have mistakenly interpreted the bottom of the Rochester Formation or the Irondequoit Formation as the bottom of the Amabel Formation.

Brunton indicated that the Golder (2006) interpretation is one of the cases in which the reported bottom of the Amabel Formation lies below the actual bottom of the Gasport Formation. In the Cambridge area, the difference between the two elevations is on the order of 4.5 m. Since the Gasport Formation is relatively thick in the Cambridge area, errors arising from confounding the bottom of the Rochester Formation/Irondequoit Formation with the bottom of the Gasport Formation are not particularly significant. However, to be as consistent as possible, an average thickness of the Rochester Formation/Irondequoit Formation was added to the elevations digitized from the Golder (2006) interpretation. In total, 1081 additional control points were added from the Golder (2006) interpretation. The locations of the additional control points are shown in **Figure 12-2**.

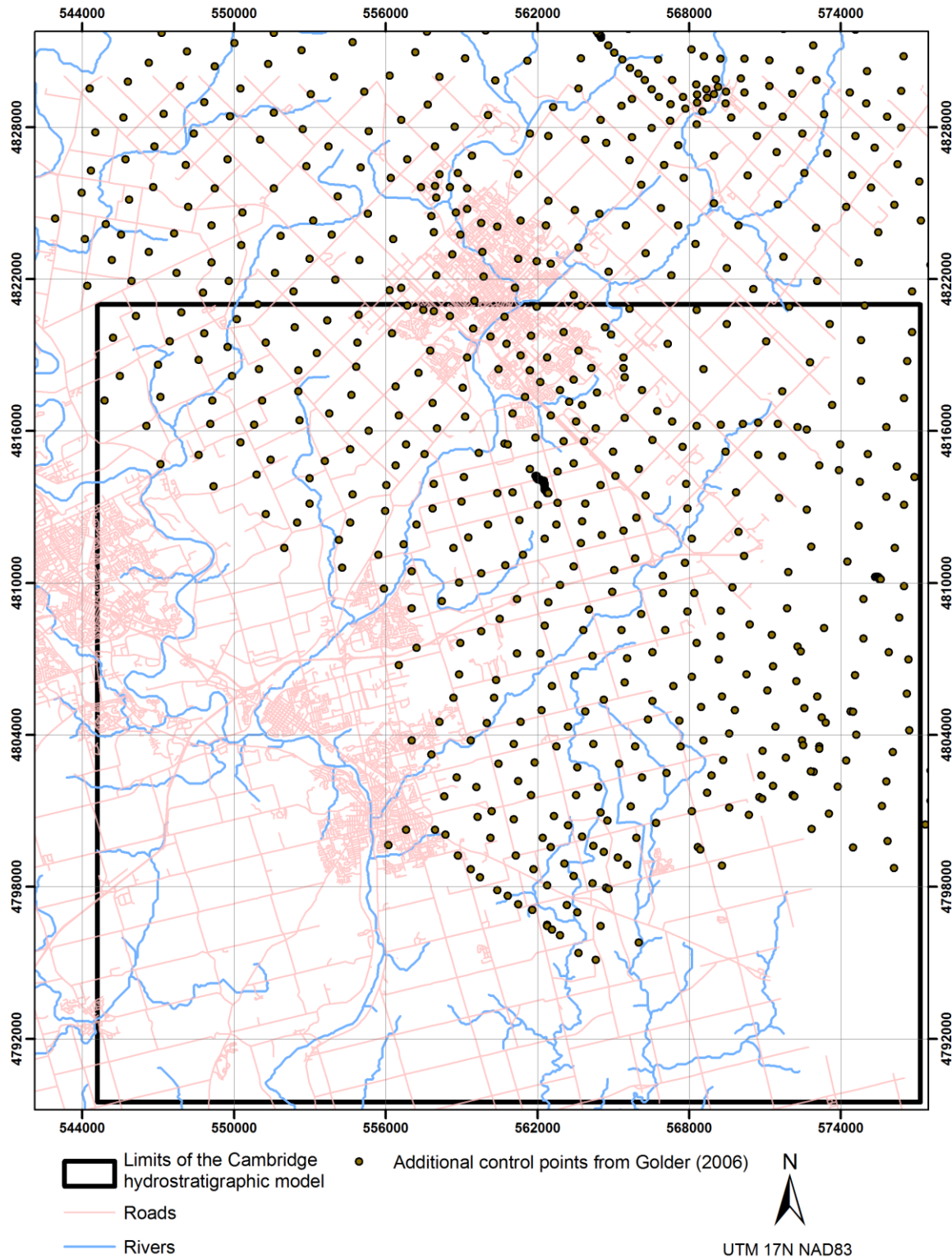
12.3 Methodology and results

The continuous surface of the top of the Cabot Head Formation was developed using Ordinary Kriging with a linear variogram. The interpolated surface is shown in **Figure 12-3**. In general, the surface dips toward the southwest. The measured and interpolated elevations of the top of the Cabot Head Formation at the borehole control points are compared in **Figure 12-4**. As shown in the figure, the interpolated elevations match the observations closely. The cumulative probability distribution of the difference between the interpolated elevation and the elevation at boreholes is shown in **Figure 12-5**. The mean absolute error of the interpolation is about 0.18 m. Over 95% of the borehole picks have an absolute error of less than 0.2 m. Most of the points on the cumulative probability distribution plot approximate a straight line; this suggests that the interpolation errors are distributed randomly.



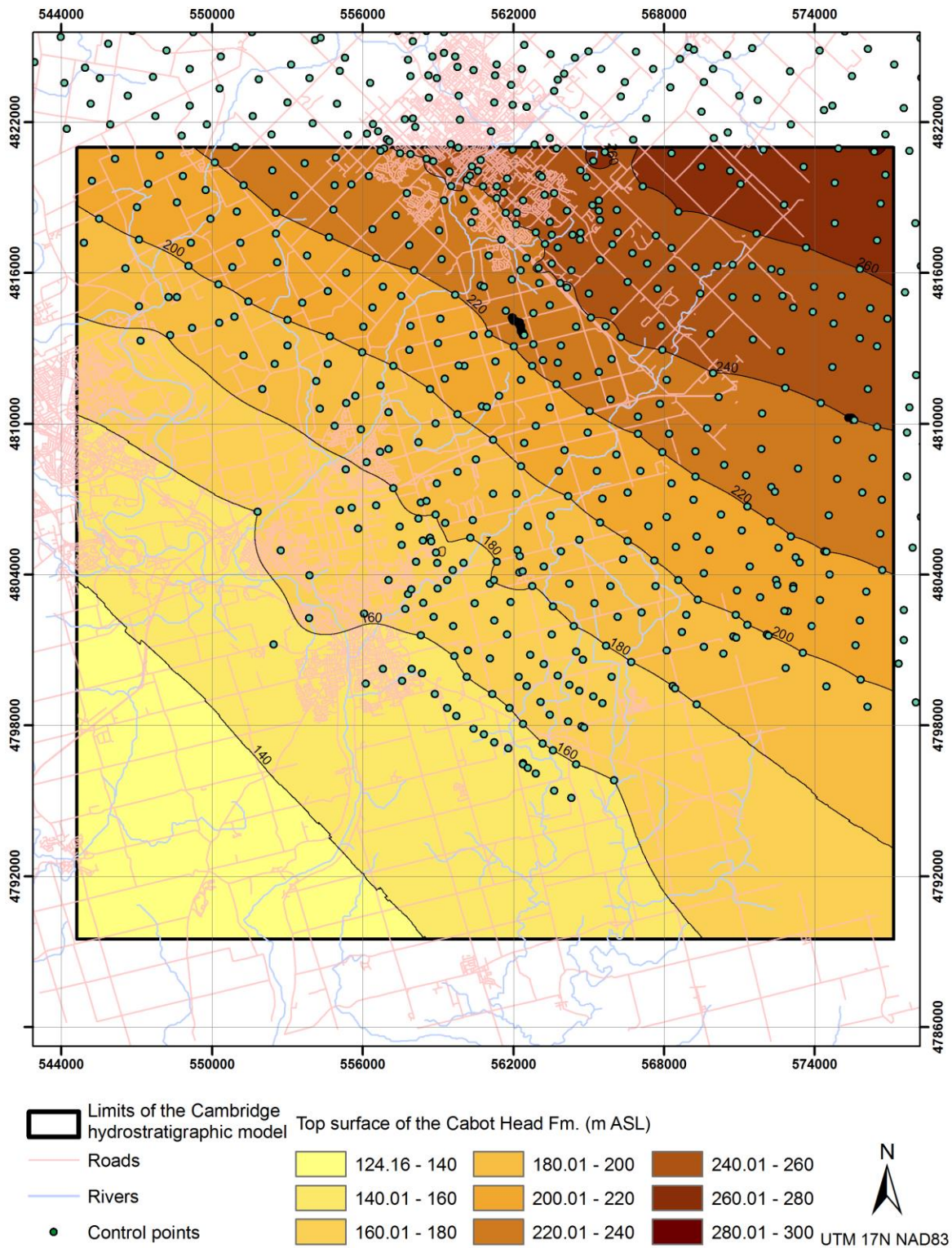
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Figure 12-1 Available borehole control for the top of the Cabot Head Formation



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Figure 12-2 Additional control points for the top of the Cabot Head Formation



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Figure 12-3 Interpolated elevations of the top the Cabot Head Formation

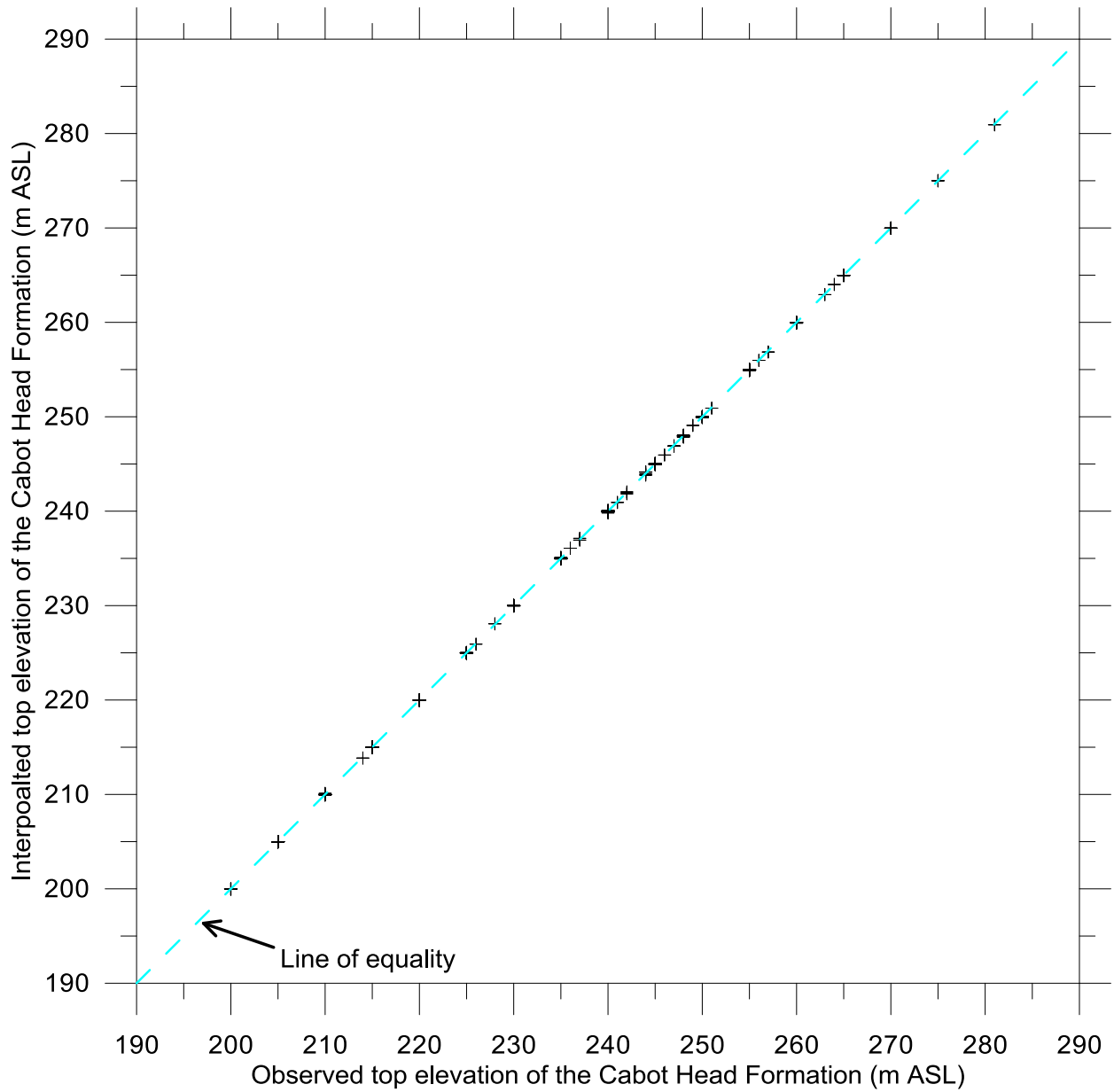


Figure 12-4 Observed and interpolated elevations of the top of the Cabot Head Formation

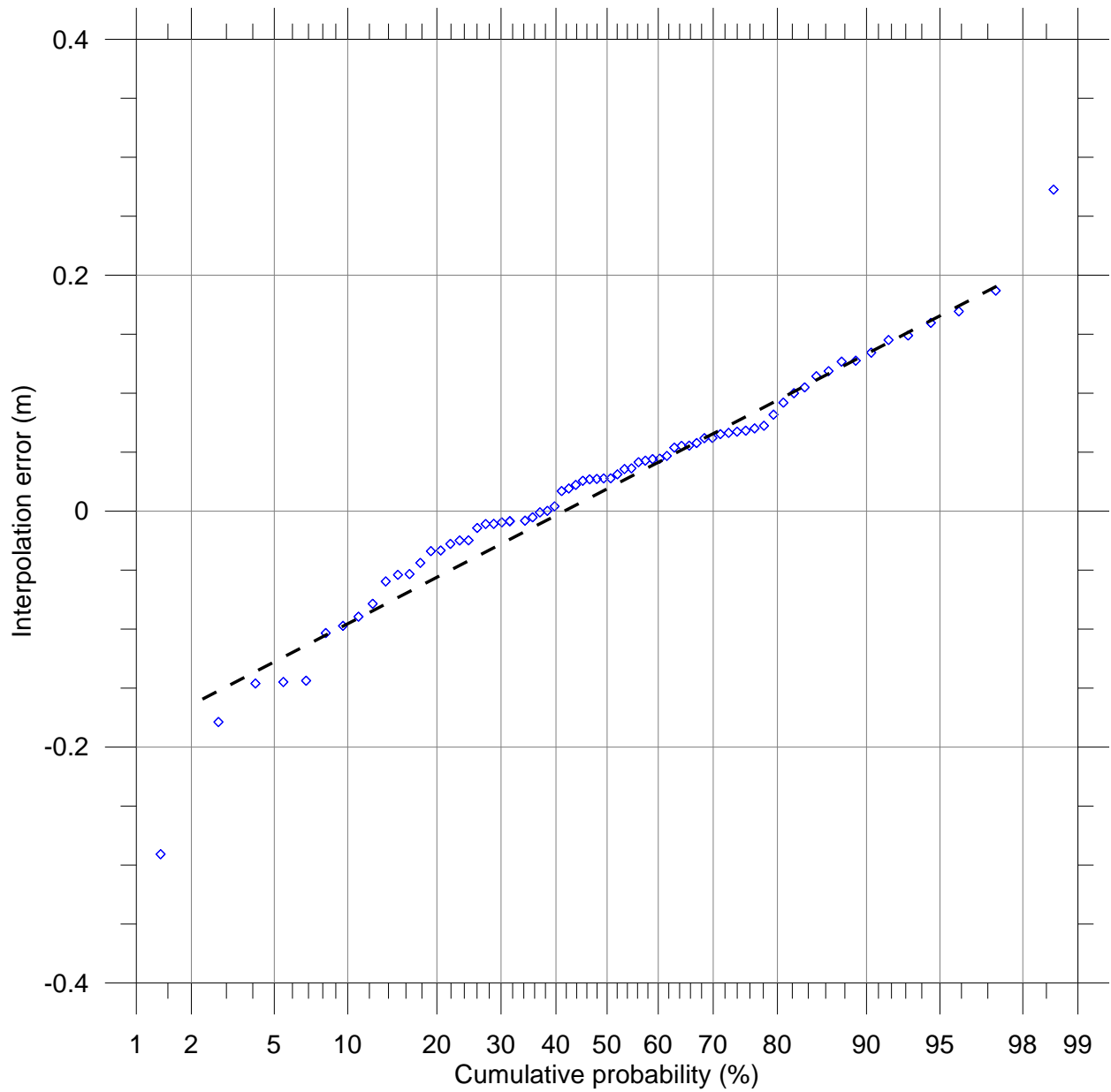


Figure 12-5 Cumulative probability distribution of differences in the interpolated elevations of the top of the Cabot Head Formation

Section 13

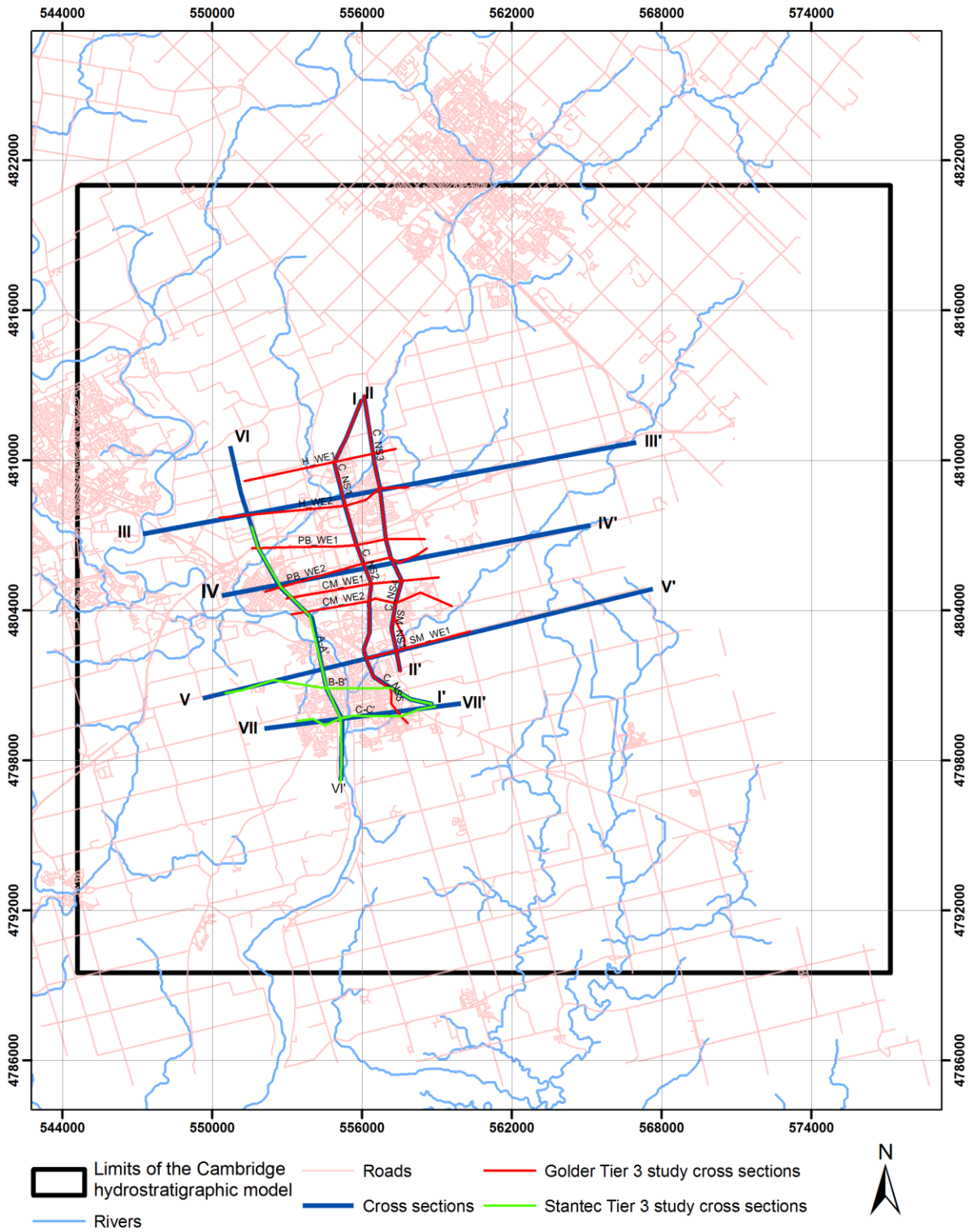
Hydrostratigraphic cross-sections

As a final check on the surfaces developed for the Cambridge hydrostratigraphic model, cross-sections through the final model were prepared. These sections were aligned with the interpretations developed for the Region of Waterloo Tier 3 study (Golder, 2011a; Stantec 2011a). The locations of the cross-sections are indicated in **Figure 13-1**, and the correspondences between the cross-sections are listed on **Table 13-1**.

Table 13-1 Listing of comparison cross-sections

Cross-section through the present hydrostratigraphic model	Tier 3 cross-section
Figure 13-2; I-I'	Golder (2011a) C_NS1, C_NS2
Figure 13-3; II-II'	Golder (2011a) C_NS3, C_NS4 Golder (2011a) SM_NS1
Figure 13-4; III-III'	Golder (2011a) H_WE2
Figure 13-5; IV-IV'	Golder (2011a) PB_WE2
Figure 13-6; V-V'	Stantec (2011a) B-B' Golder (2011a) SM_WE1
Figure 13-7; VI-VI'	Stantec (2011a) A-A'
Figure 13-8; VII-VII'	Stantec (2011a) C-C'

Review of **Figure 13-2** through **Figure 13-8** suggests that there is a close correspondence with the cross-sections presented in the Tier 3 reports. The sections were developed independently; therefore, this is in some sense a test of the consistency between the expert interpretations incorporated in the sections prepared for the Tier 3 study and the hydrostratigraphic model developed here.



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Figure 13-1 Locations of cross-sections

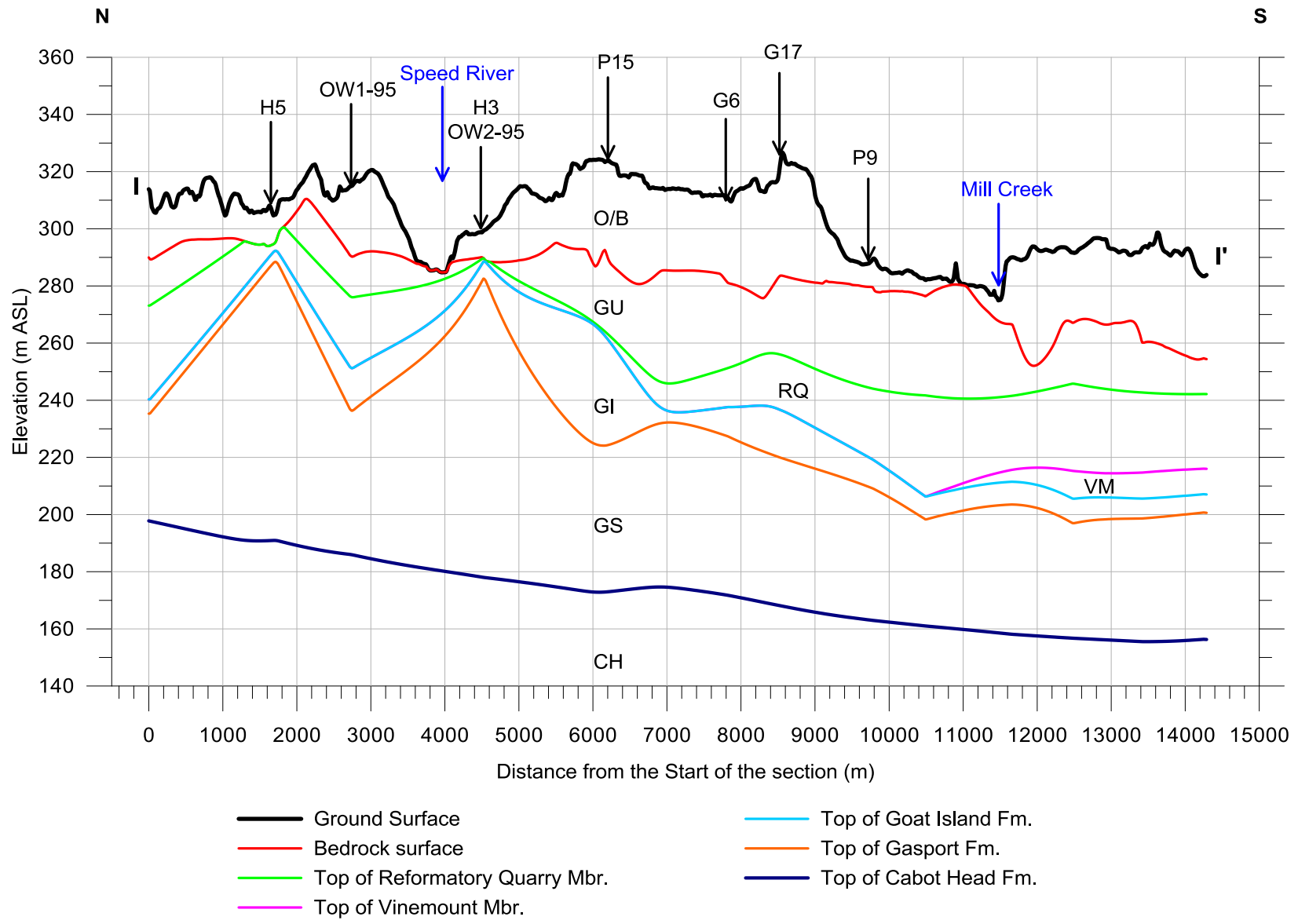


Figure 13-2 Cross-section I-I'

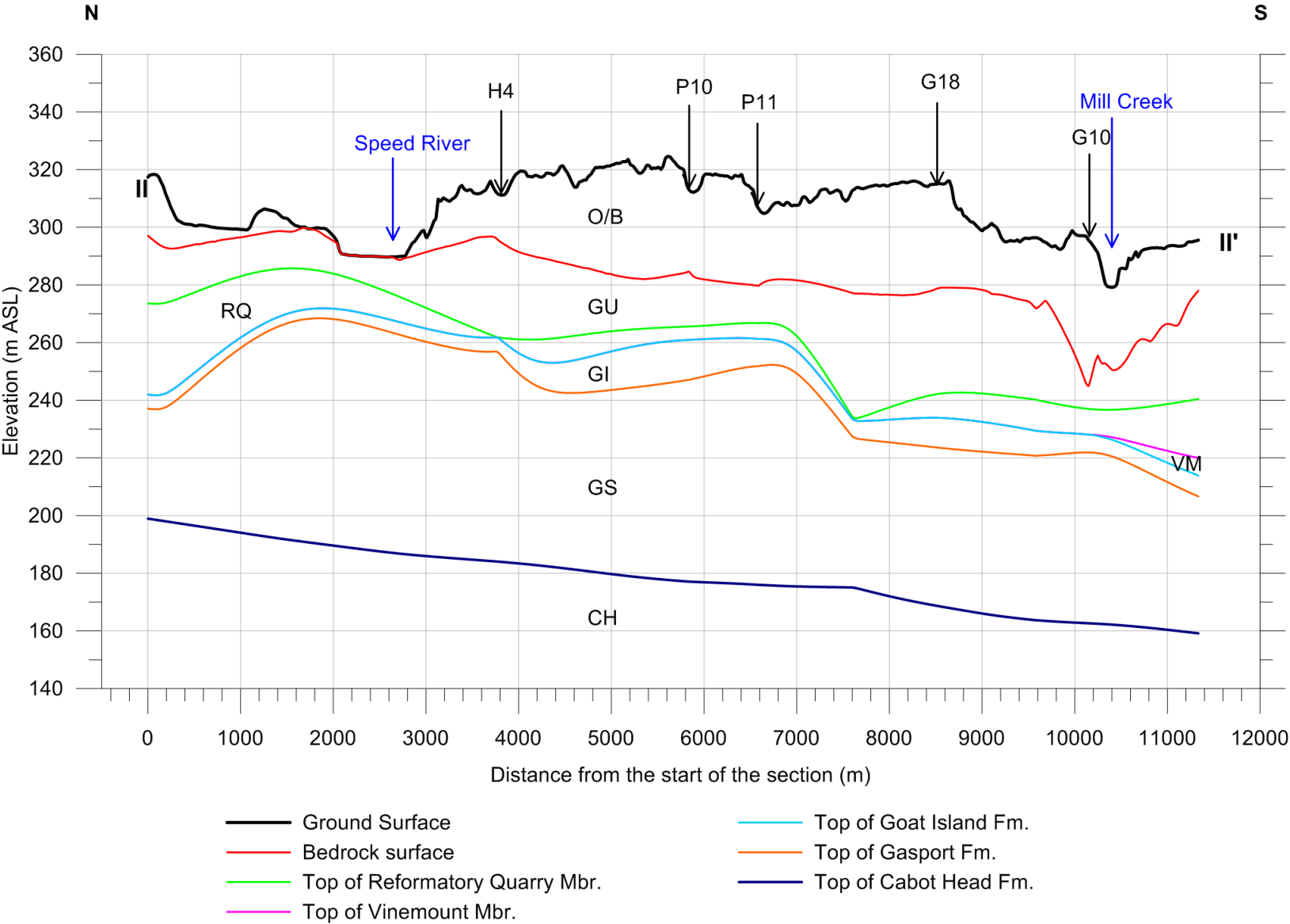


Figure 13-3 Cross-section II-II'

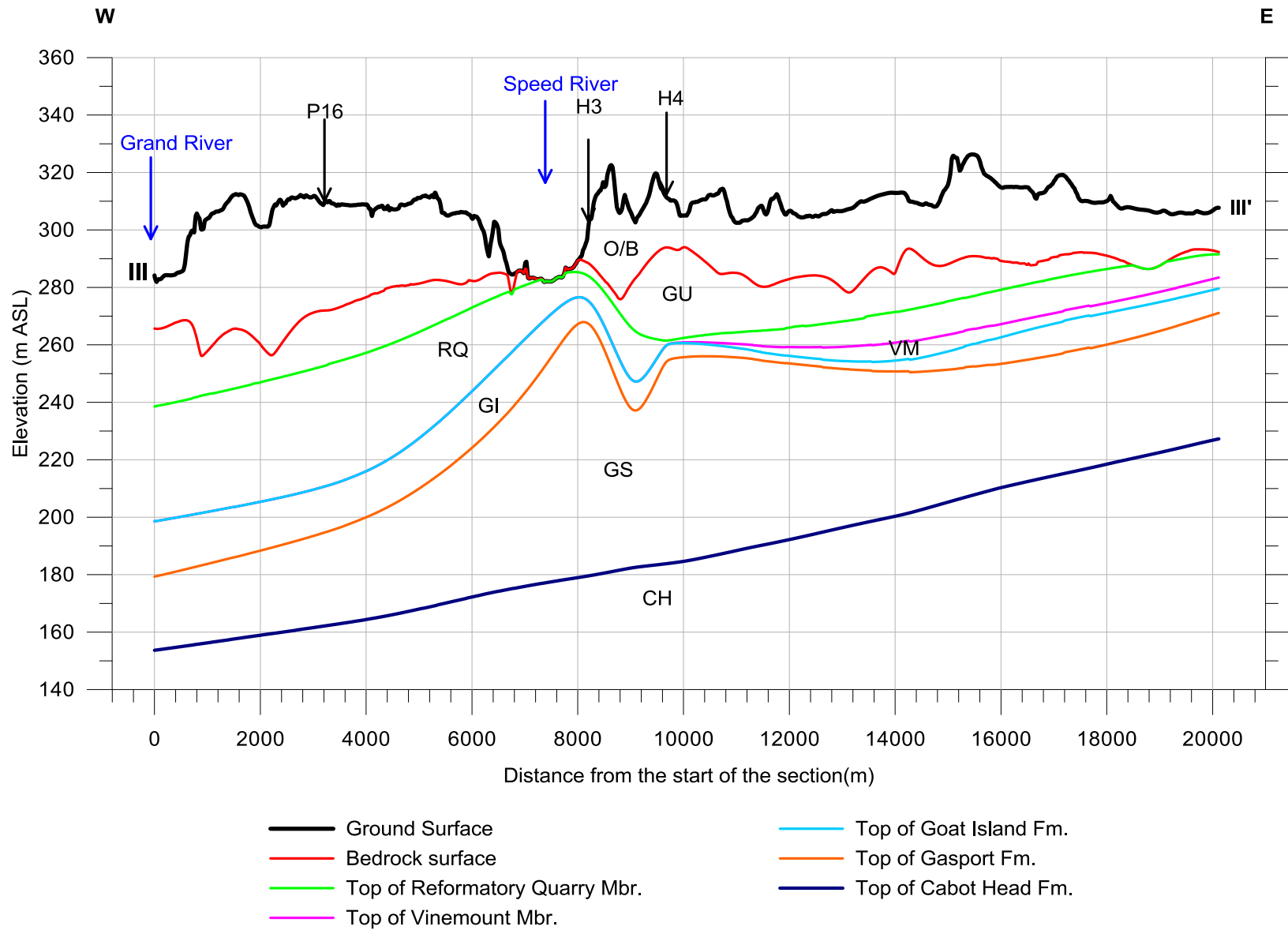


Figure 13-4 Cross-section III-III'

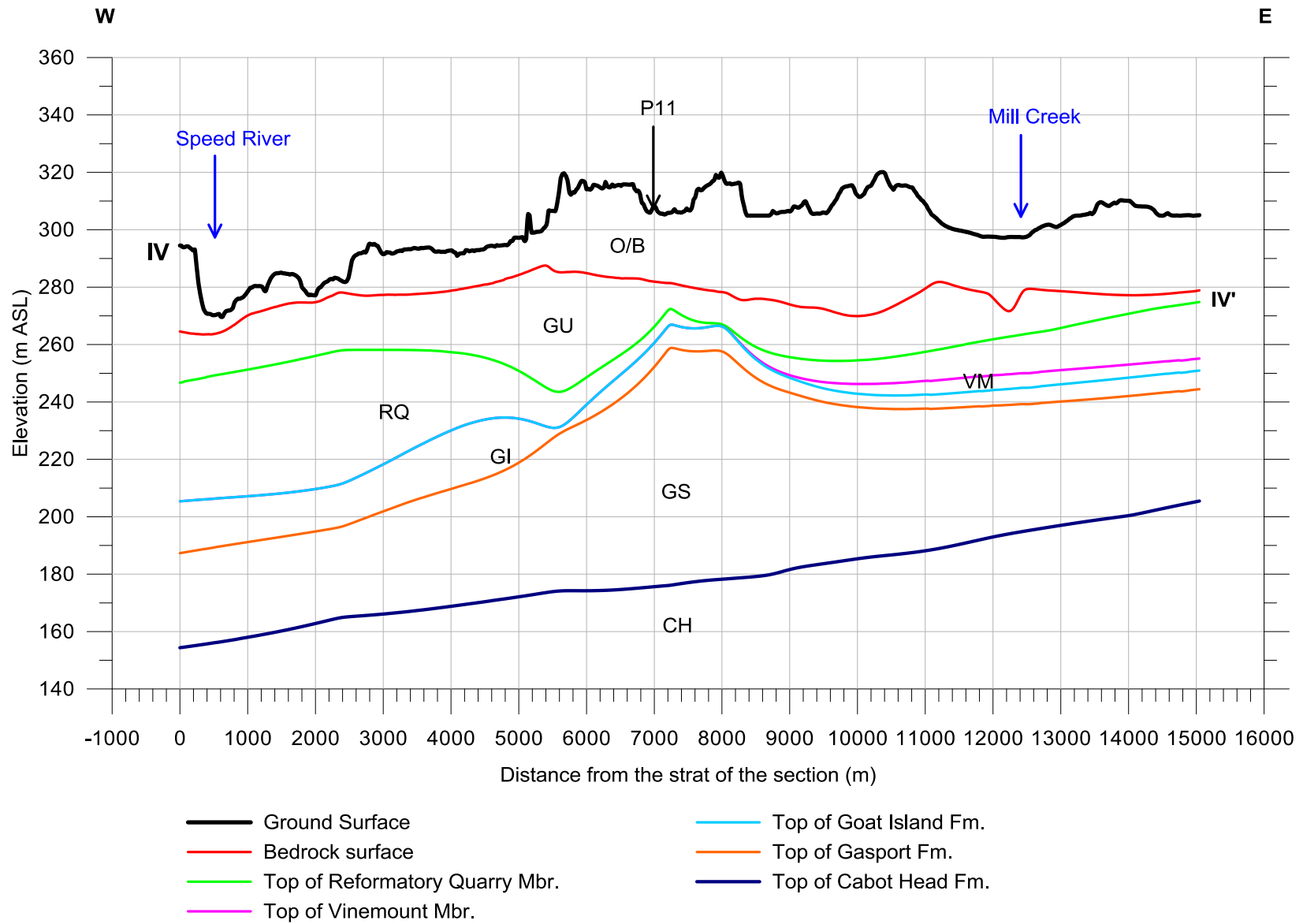


Figure 13-5 Cross-section IV-IV'

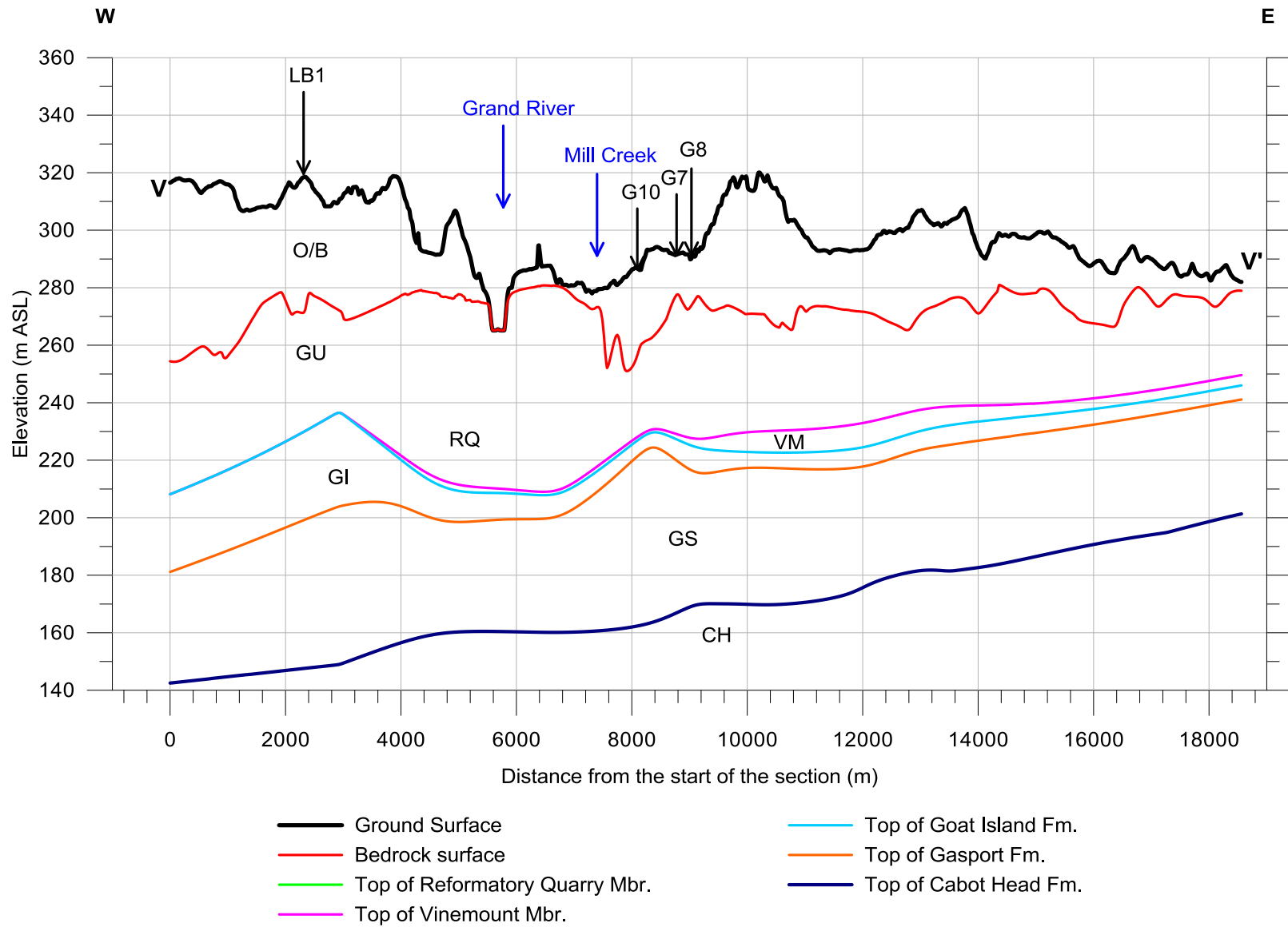


Figure 13-6 Cross-section V-V'

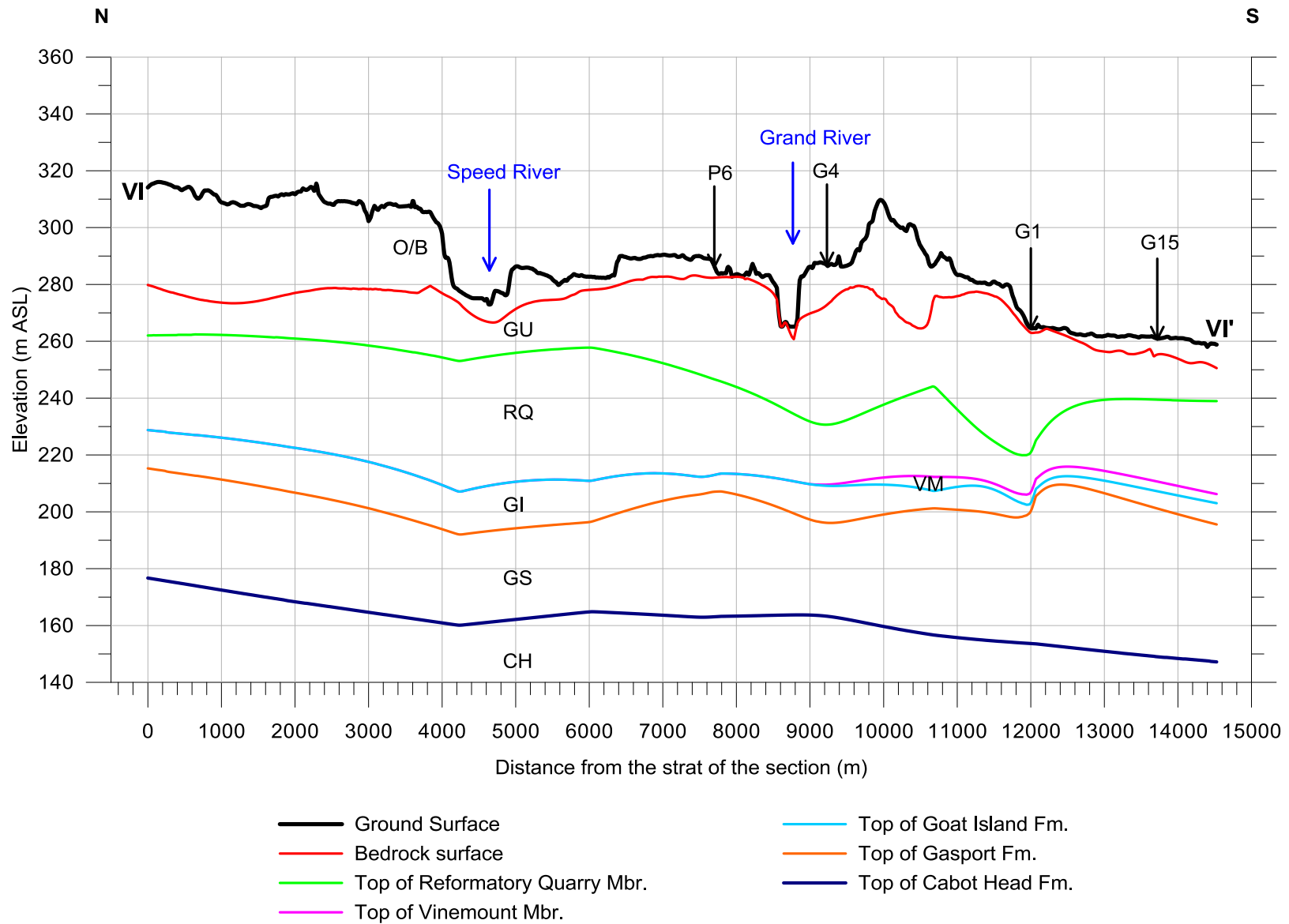


Figure 13-7 Cross-section VI-VI'

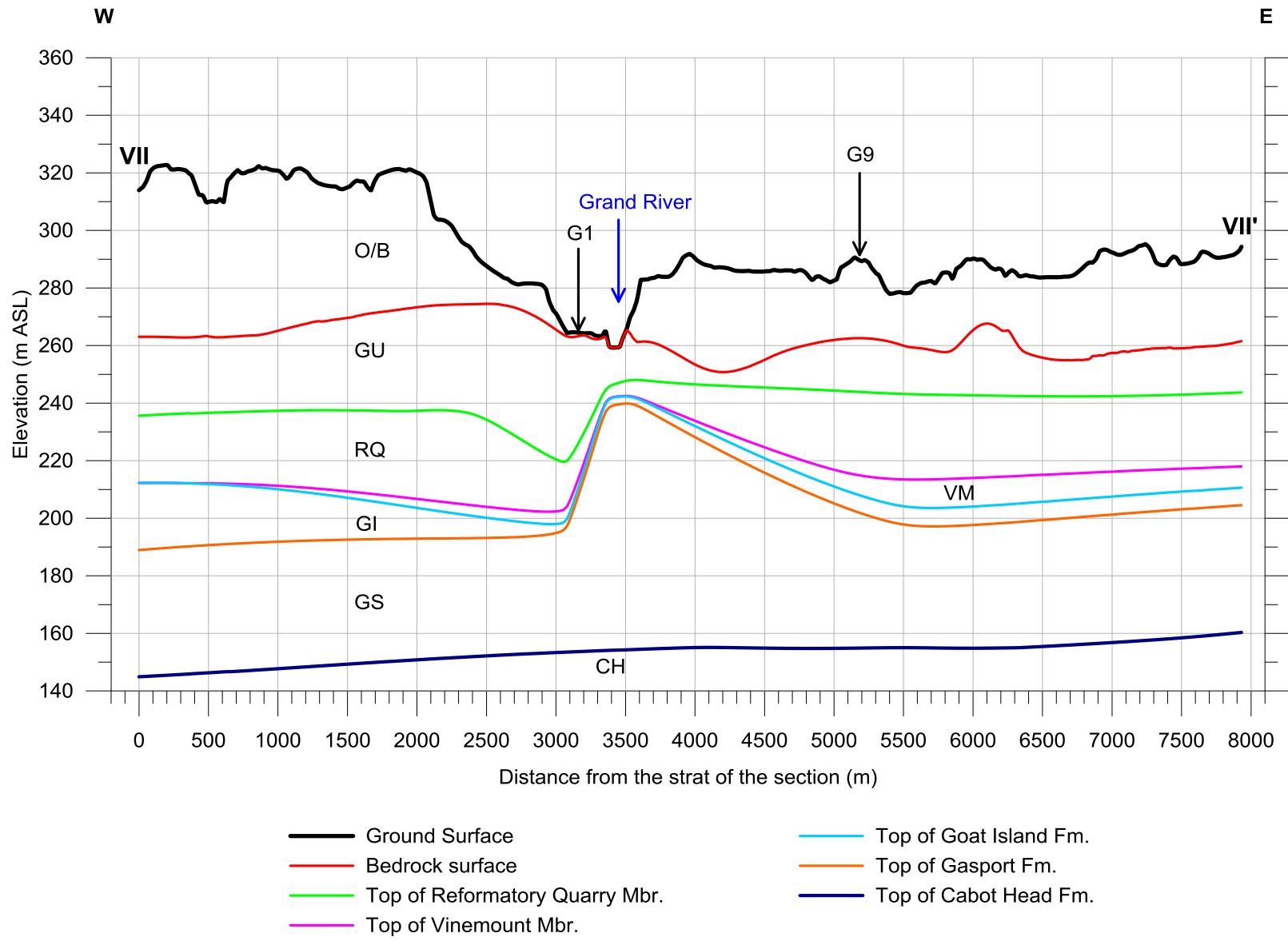


Figure 13-8 Cross-section VII-VII'

Section 14

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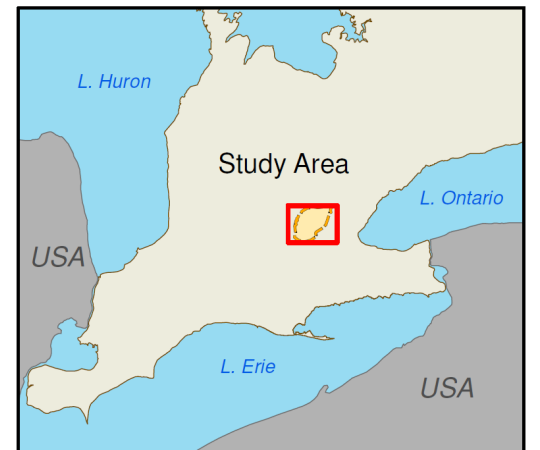
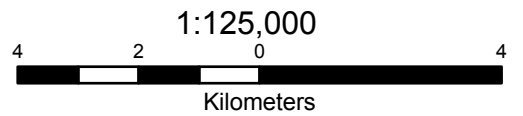
Appendix B

Cambridge East Model Calibration: Calibrated Hydraulic Conductivity

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

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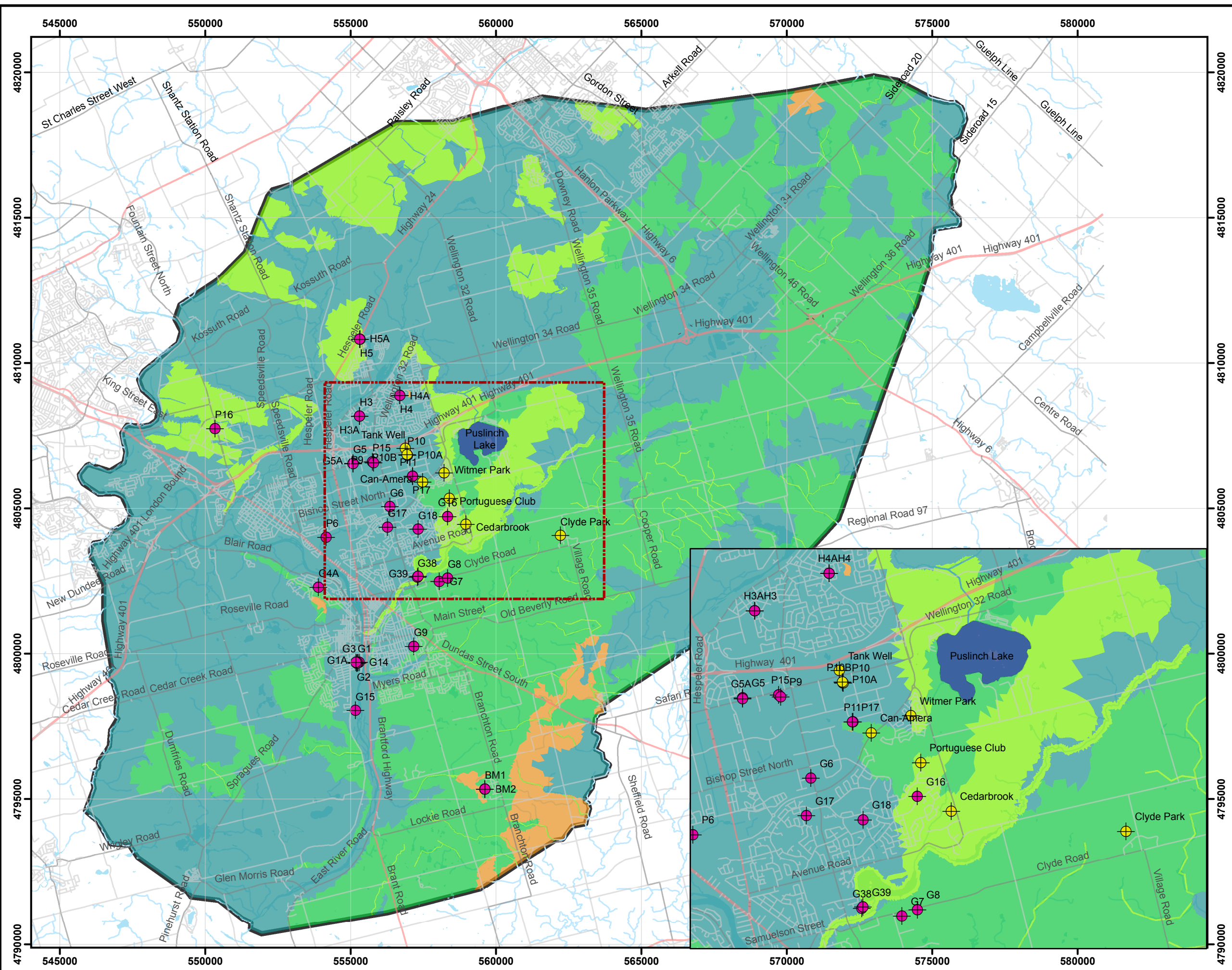
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 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Hydraulic Conductivity (m/s)
- 10^{-8}
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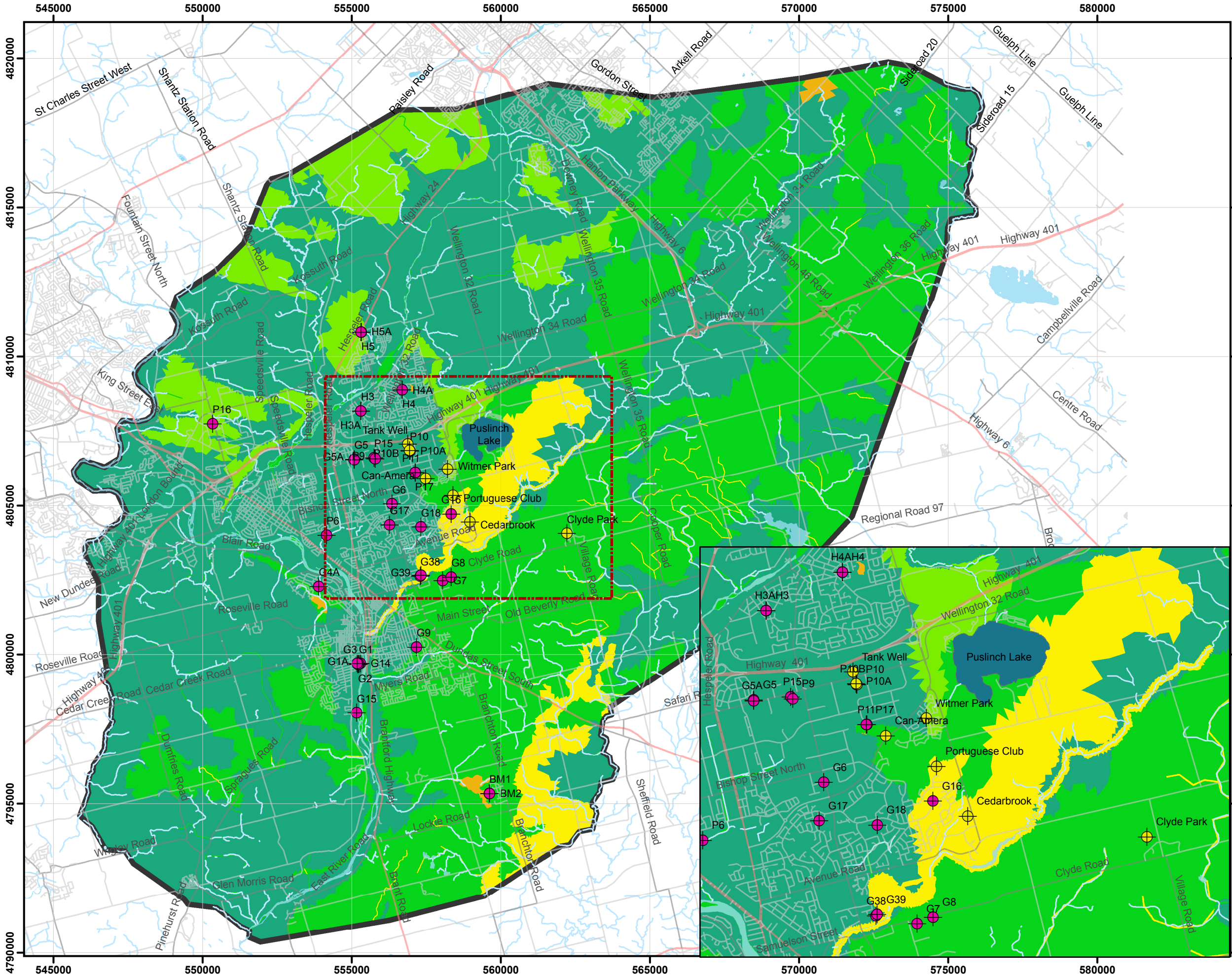
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 Map Version: 3, Map data 2015-09-14 J. Zhang



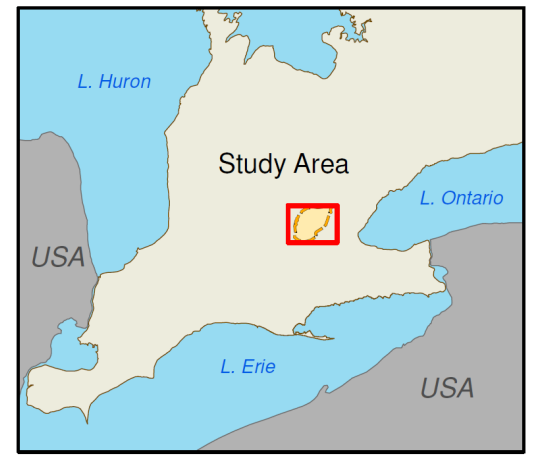
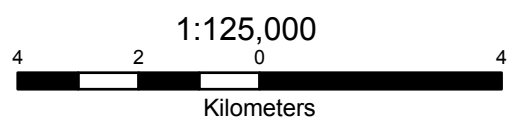
Figure 1
 Horizontal Hydraulic Conductivity:
 Model Layer 1: Overburden



Cambridge East IUS Water Supply Class EA: Groundwater Modelling



- LEGEND**
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Vertical Hydraulic Conductivity (m/s)**
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 - $10^{-10} - 10^{-9}$
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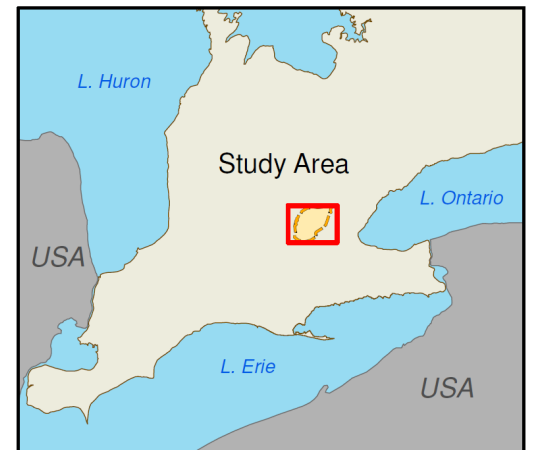
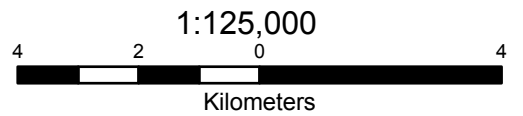


Figure 2
 Vertical Hydraulic Conductivity:
 Model Layer 1: Overburden

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

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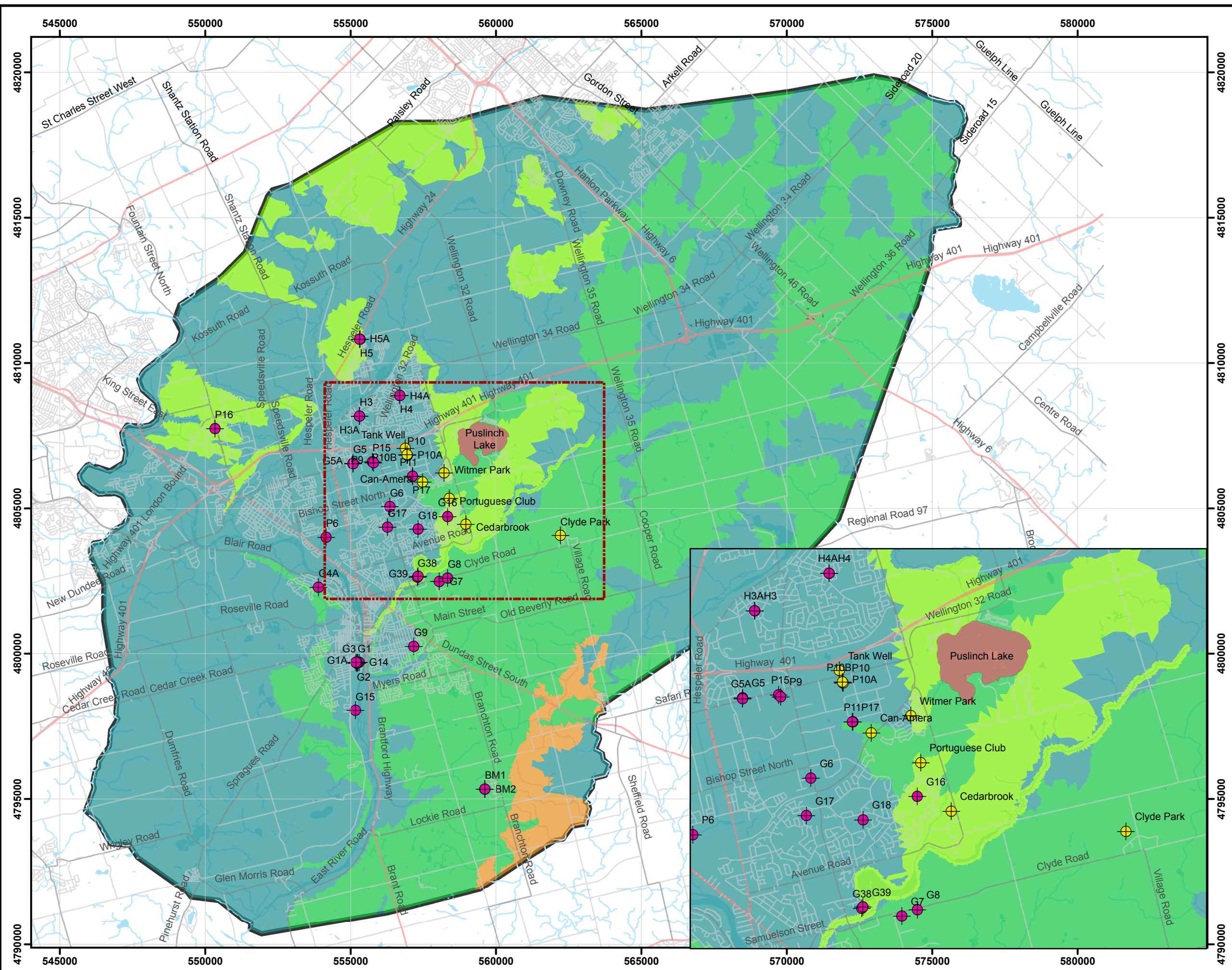
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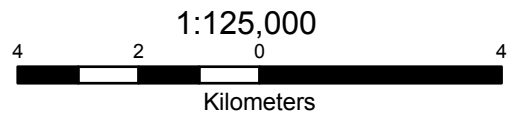
Figure 3
 Horizontal Hydraulic Conductivity:
 Model Layer 2: Overburden



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

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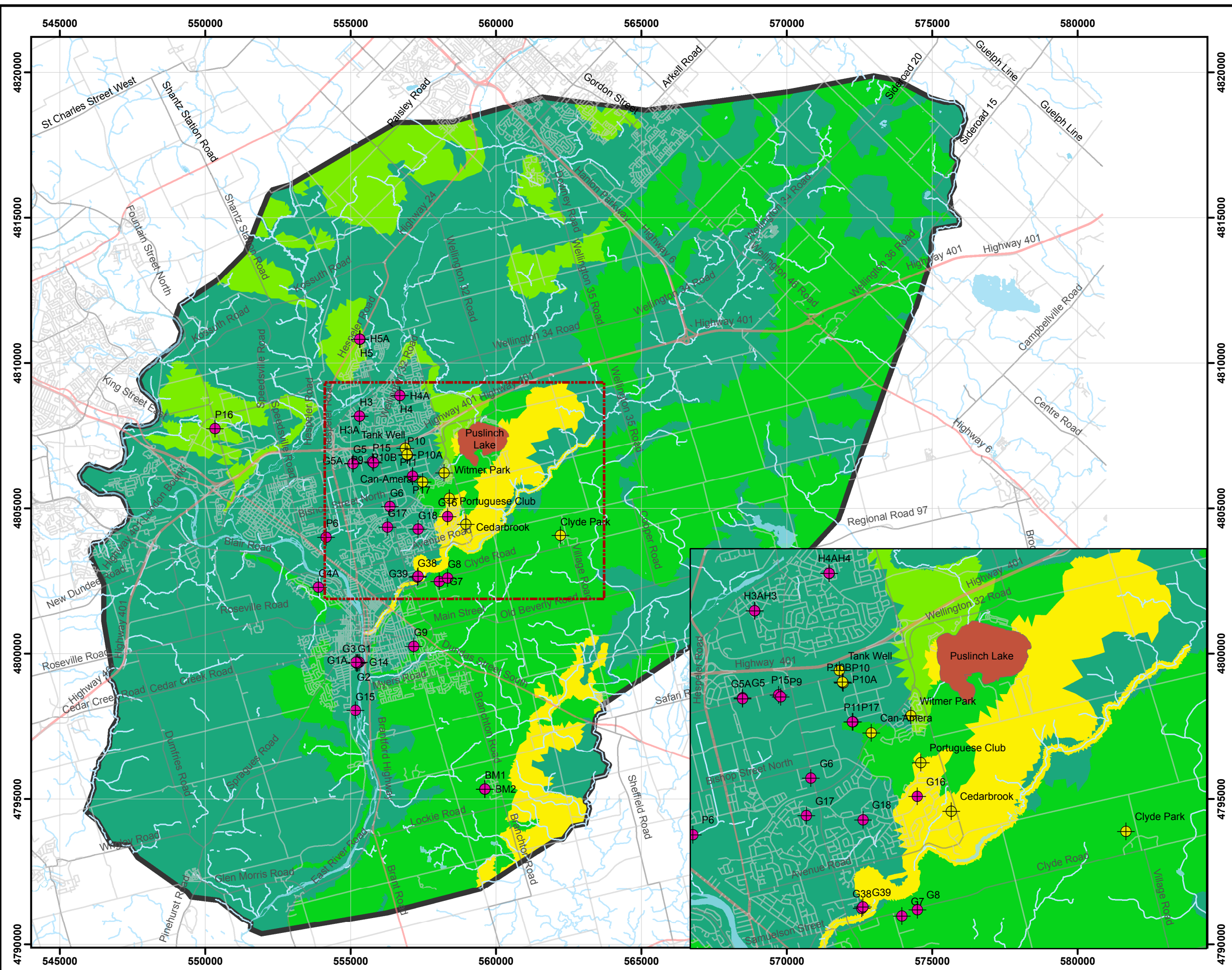
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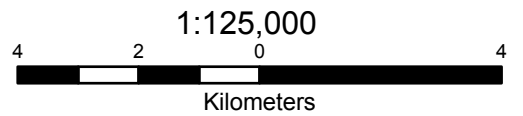
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 Vertical Hydraulic Conductivity:
 Model Layer 2: Overburden



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

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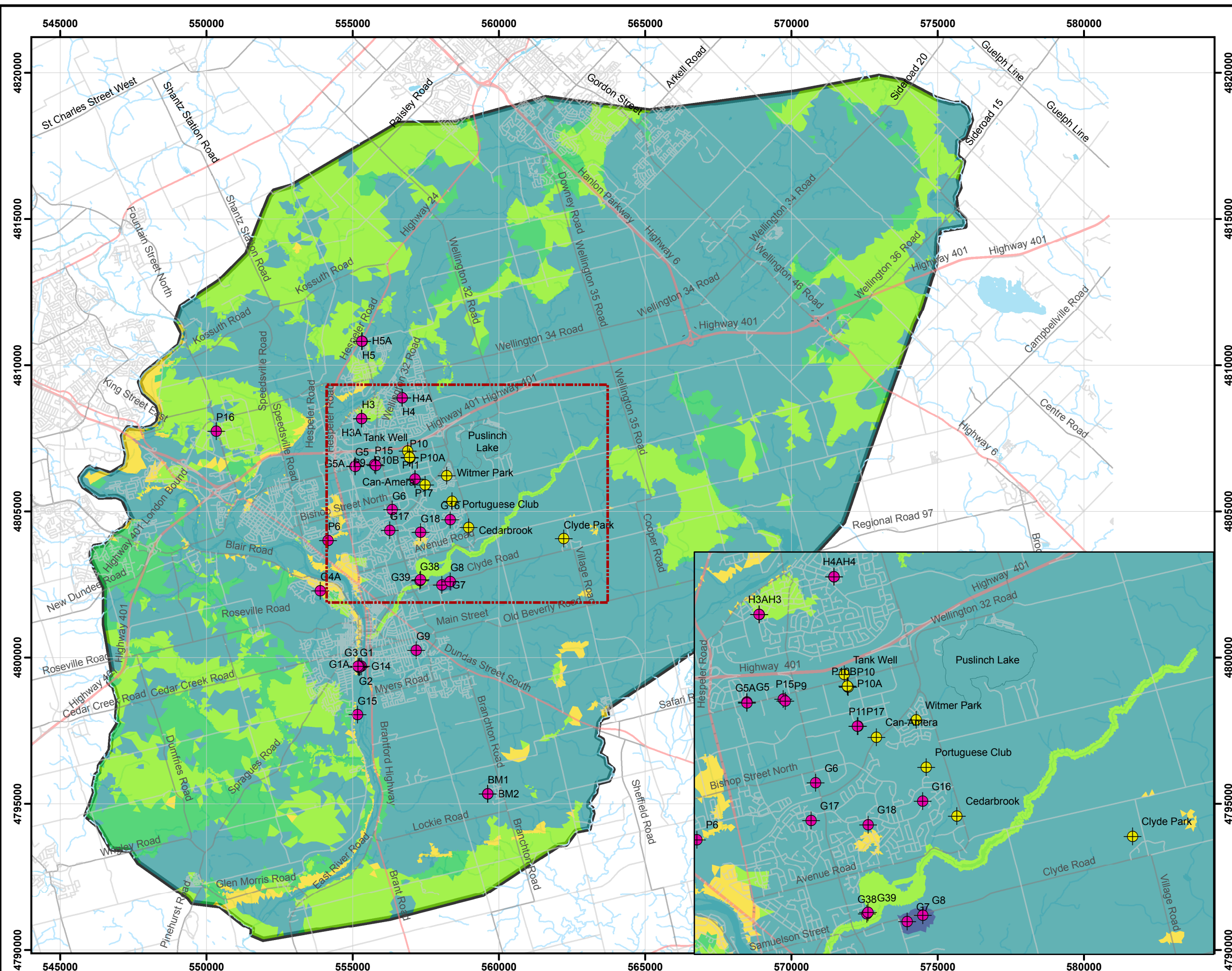
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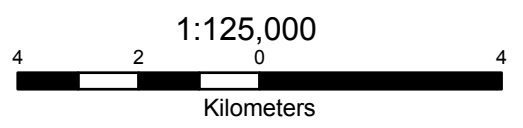
Figure 5
 Horizontal Hydraulic Conductivity:
 Model Layer 3: Overburden



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

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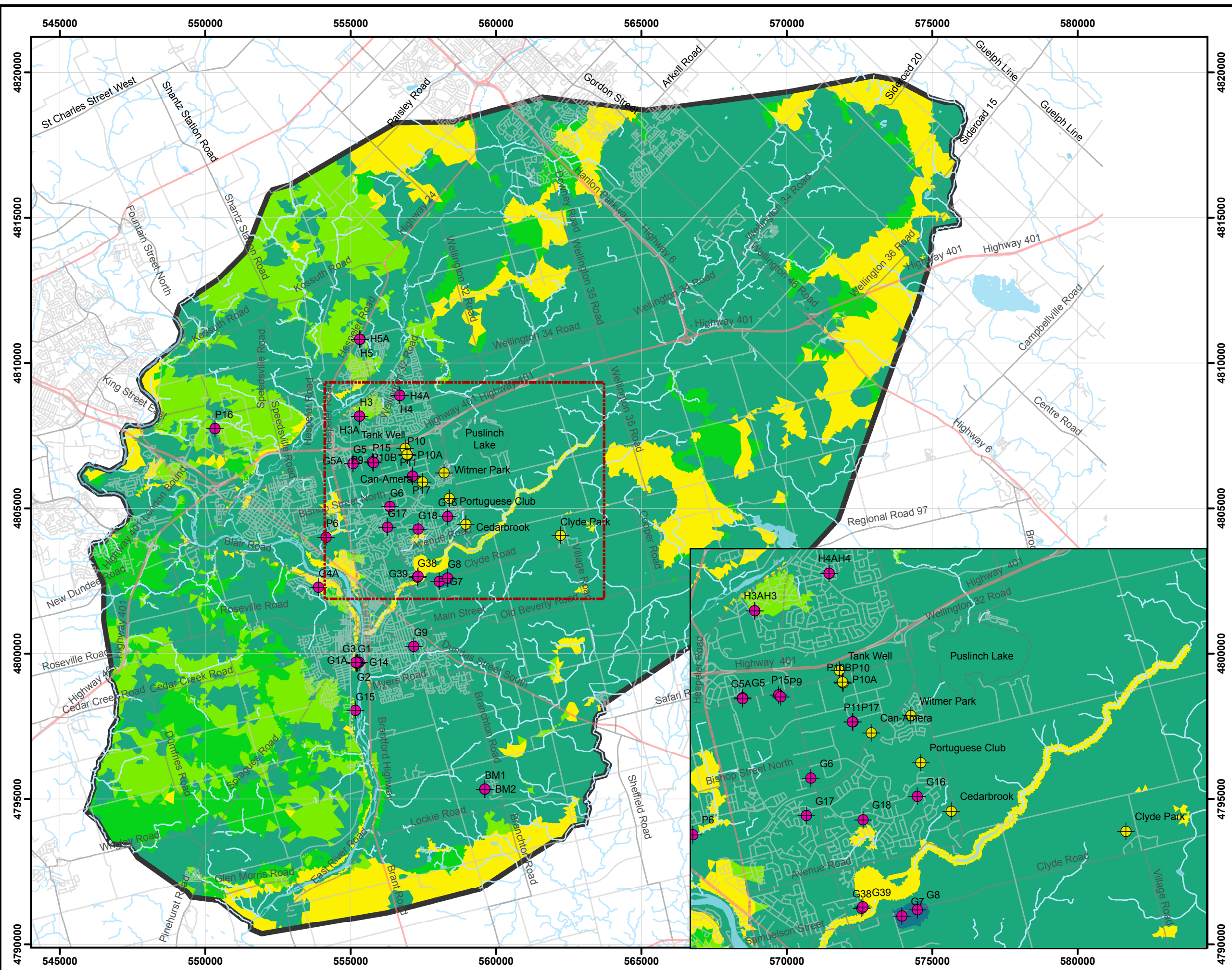
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 - Model Domain
- Vertical Hydraulic Conductivity (m/s)
- 10^{-10}
 - $10^{-10} - 10^{-9}$
 - $10^{-9} - 10^{-8}$
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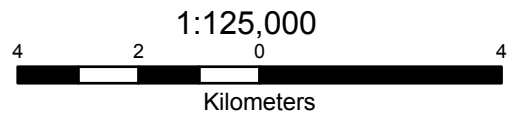
Figure 6
 Vertical Hydraulic Conductivity:
 Model Layer 3: Overburden



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

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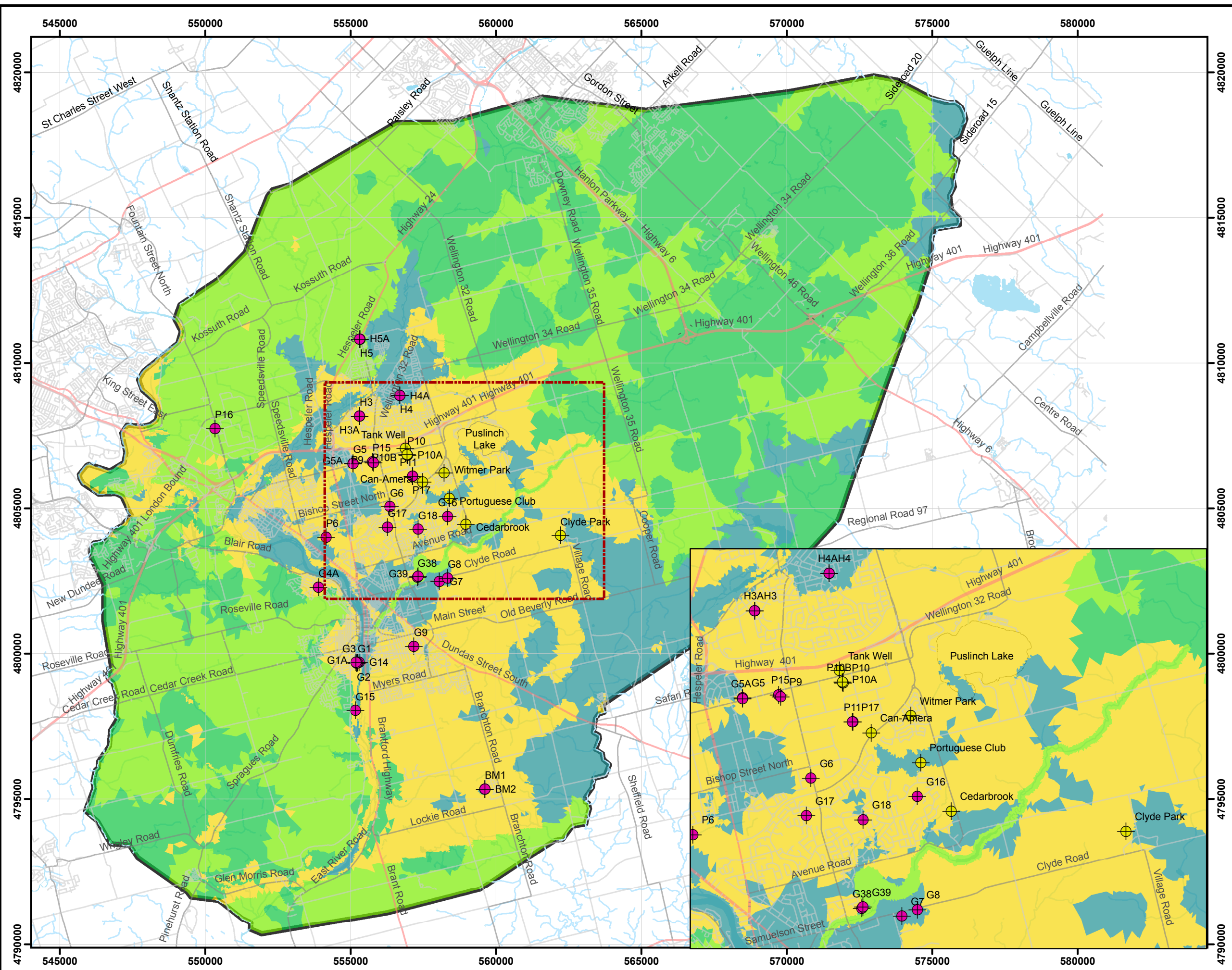
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 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Hydraulic Conductivity (m/s)
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 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
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Projection: UTM Zone 17N, NAD 83
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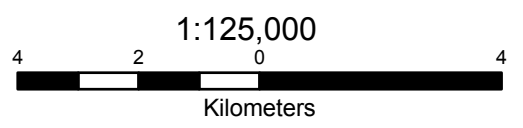
Figure 7
Horizontal Hydraulic Conductivity:
Model Layer 4: Overburden



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

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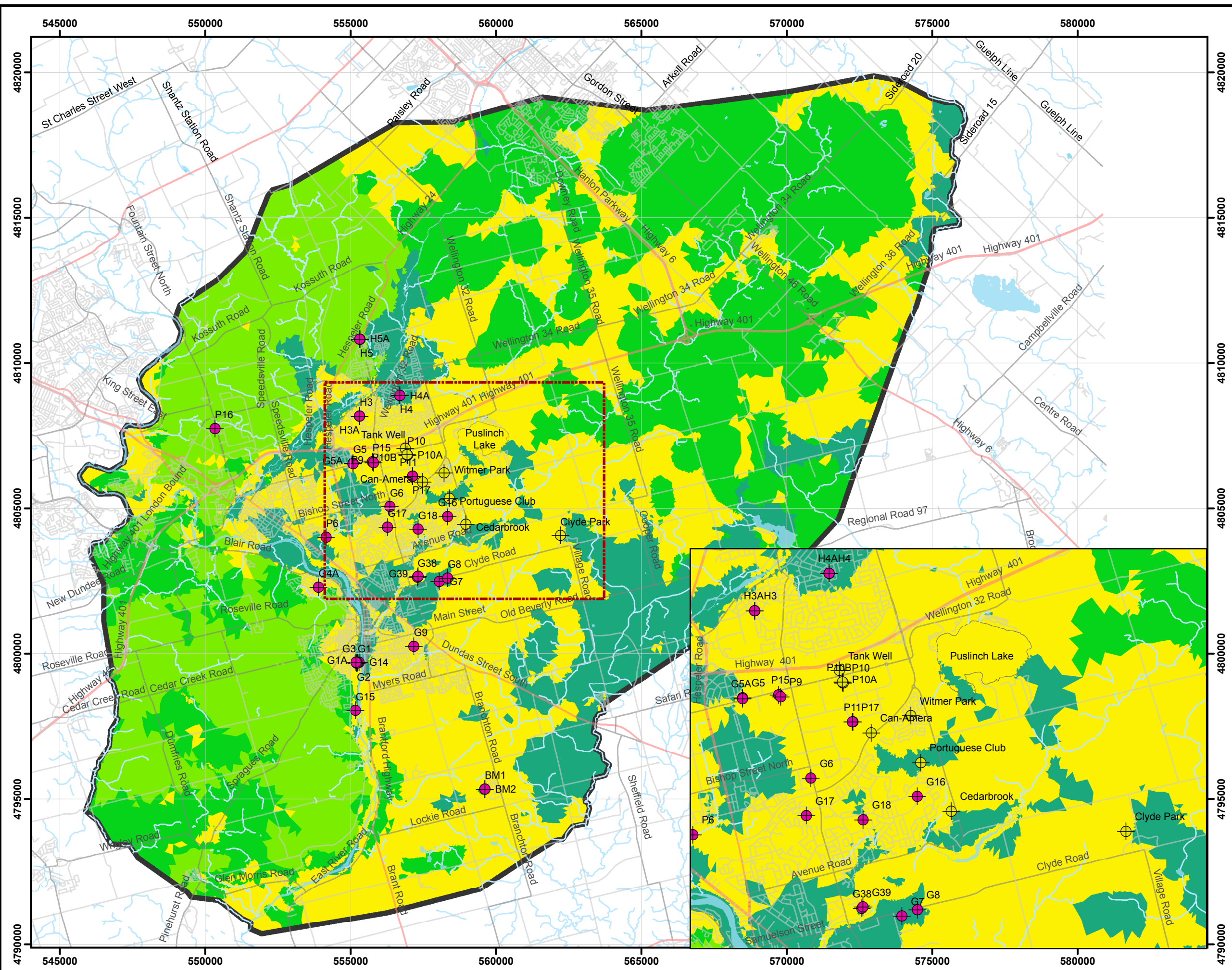
- Municipal Supply Well
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 - Model Domain
- Vertical Hydraulic Conductivity (m/s)
- 10^{-10}
 - $10^{-10} - 10^{-9}$
 - $10^{-9} - 10^{-8}$
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 2.3 \times 10^{-3}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



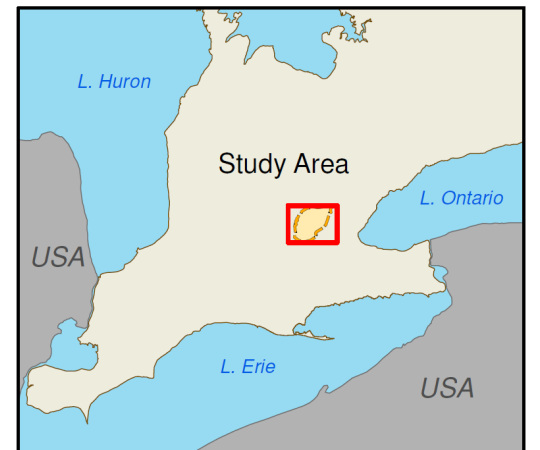
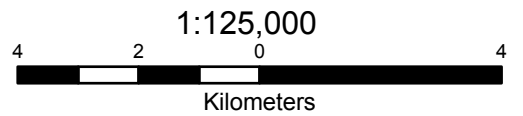
Figure 8
 Vertical Hydraulic Conductivity:
 Model Layer 4: Overburden



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

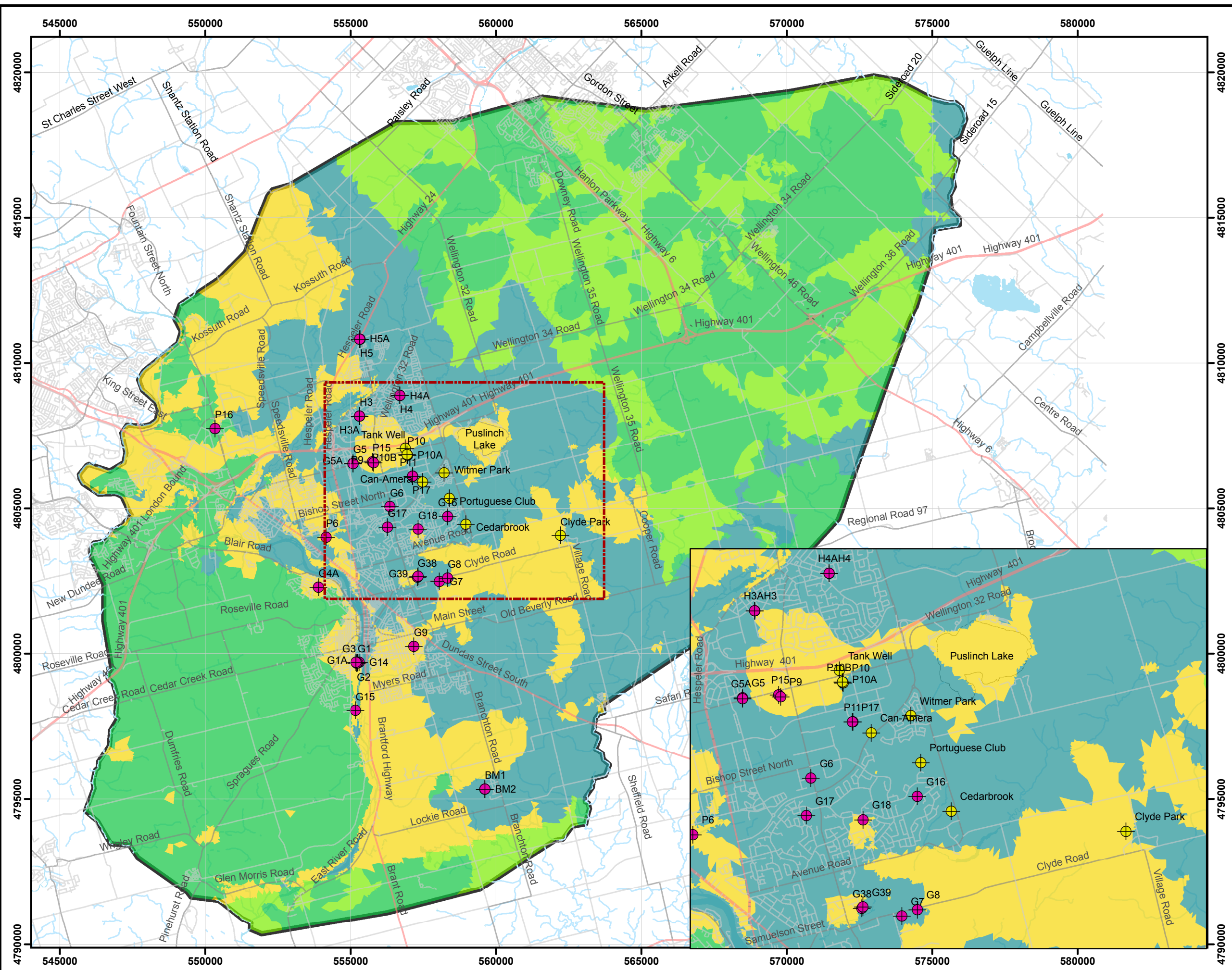
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Hydraulic Conductivity (m/s)
- 10^{-8}
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 1.5 \times 10^{-2}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang











Figure 9
 Horizontal Hydraulic Conductivity:
 Model Layer 5: Overburden












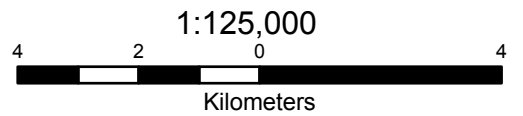
Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

-  Municipal Supply Well
-  Test Production Well
-  Expressway / Highway
-  Major Roads
-  Roads (collectors)
-  Rivers / Streams
-  Lakes and Ponds
-  Model Domain

Vertical Hydraulic Conductivity (m/s)

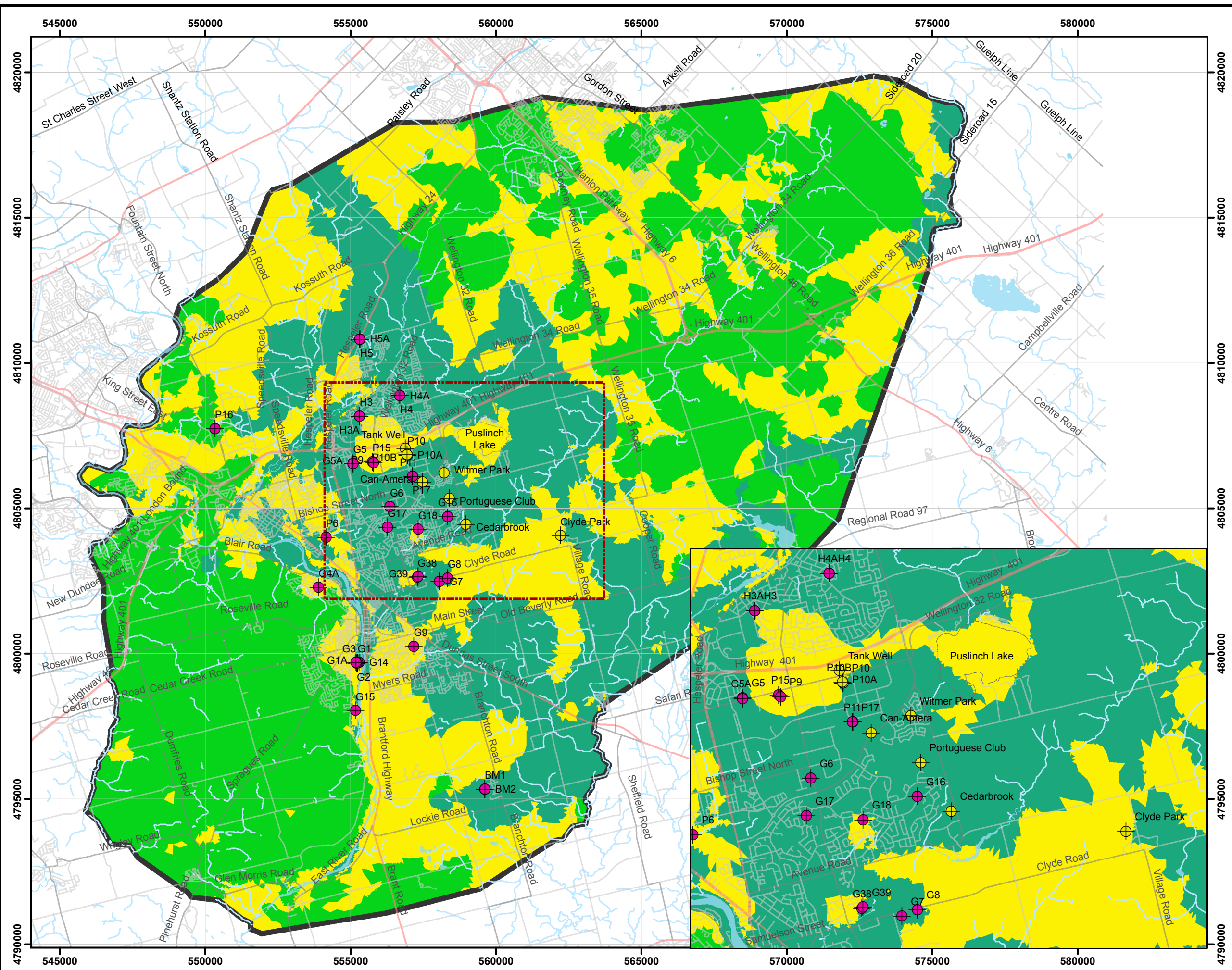
-  10^{-10}
-  $10^{-10} - 10^{-9}$
-  $10^{-9} - 10^{-8}$
-  $10^{-8} - 10^{-7}$
-  $10^{-7} - 10^{-6}$
-  $10^{-6} - 10^{-5}$
-  $10^{-5} - 10^{-4}$
-  $10^{-4} - 10^{-3}$
-  $10^{-3} - 2.3 \times 10^{-3}$



Projection: UTM Zone 17N, NAD 83
Map Version: 3, Map data 2015-09-14 J. Zhang



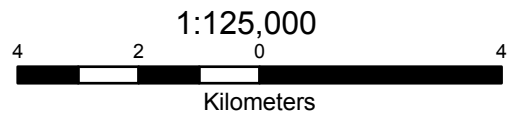
Figure 10
Vertical Hydraulic Conductivity:
Model Layer 5: Overburden



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

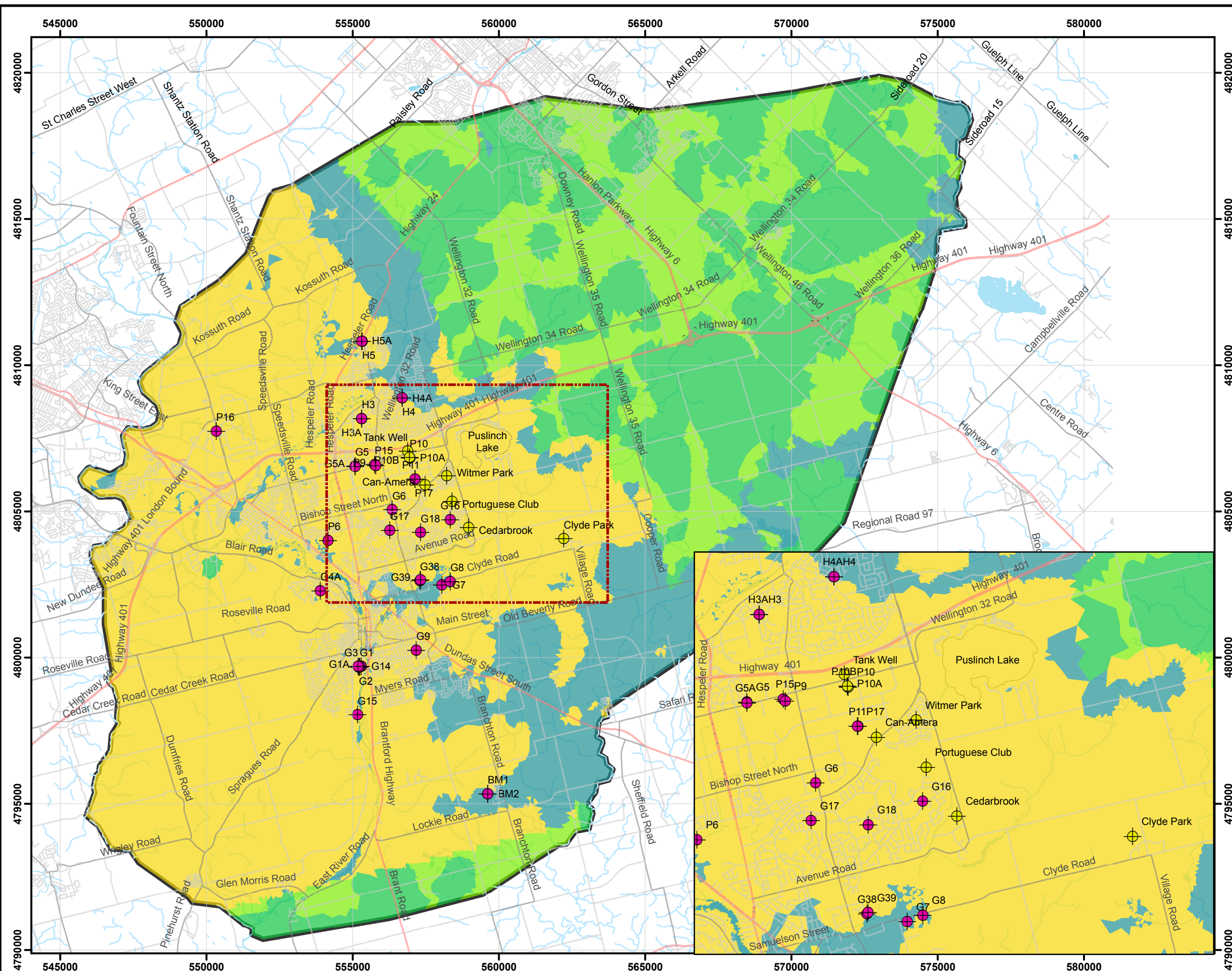
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Hydraulic Conductivity (m/s)
- 10^{-8}
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 1.5 \times 10^{-2}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



Figure 11
 Horizontal Hydraulic Conductivity:
 Model Layer 6: Overburden



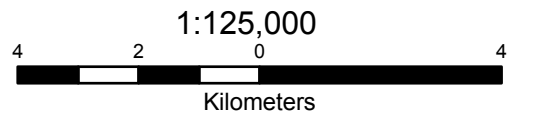
Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

- Municipal Supply Well
- Test Production Well
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds
- Model Domain

Vertical Hydraulic Conductivity (m/s)

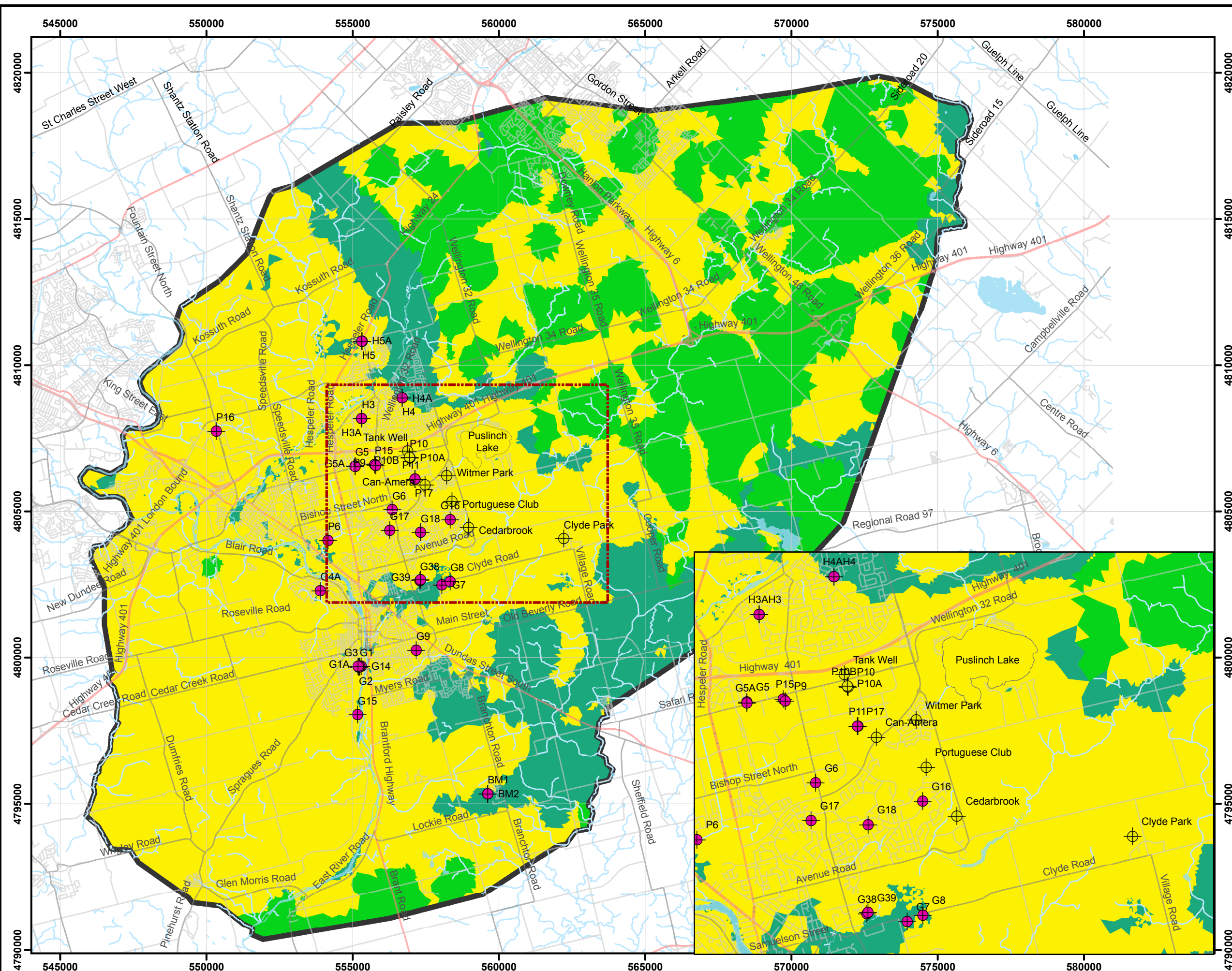
- 10^{-10}
- $10^{-10} - 10^{-9}$
- $10^{-9} - 10^{-8}$
- $10^{-8} - 10^{-7}$
- $10^{-7} - 10^{-6}$
- $10^{-6} - 10^{-5}$
- $10^{-5} - 10^{-4}$
- $10^{-4} - 10^{-3}$
- $10^{-3} - 2.3 \times 10^{-3}$



Projection: UTM Zone 17N, NAD 83
Map Version: 3, Map data 2015-09-14 J. Zhang



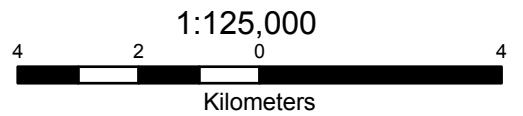
Figure 12
Vertical Hydraulic Conductivity:
Model Layer 6: Overburden



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

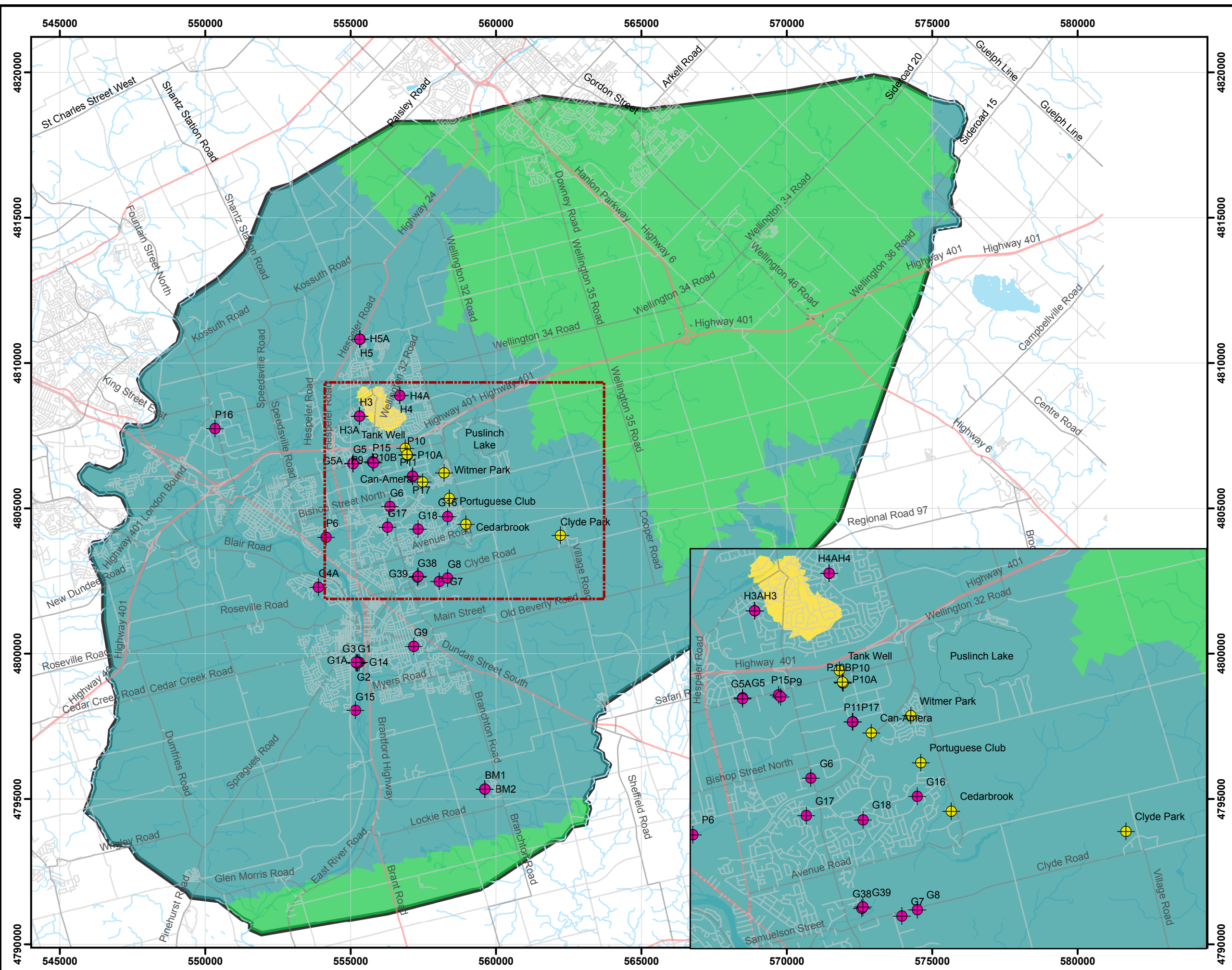
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Hydraulic Conductivity (m/s)
- 10^{-8}
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 1.5 \times 10^{-2}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



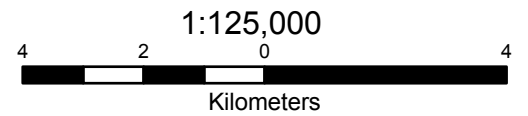
Figure 13
 Horizontal Hydraulic Conductivity:
 Model Layer 7: Overburden



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

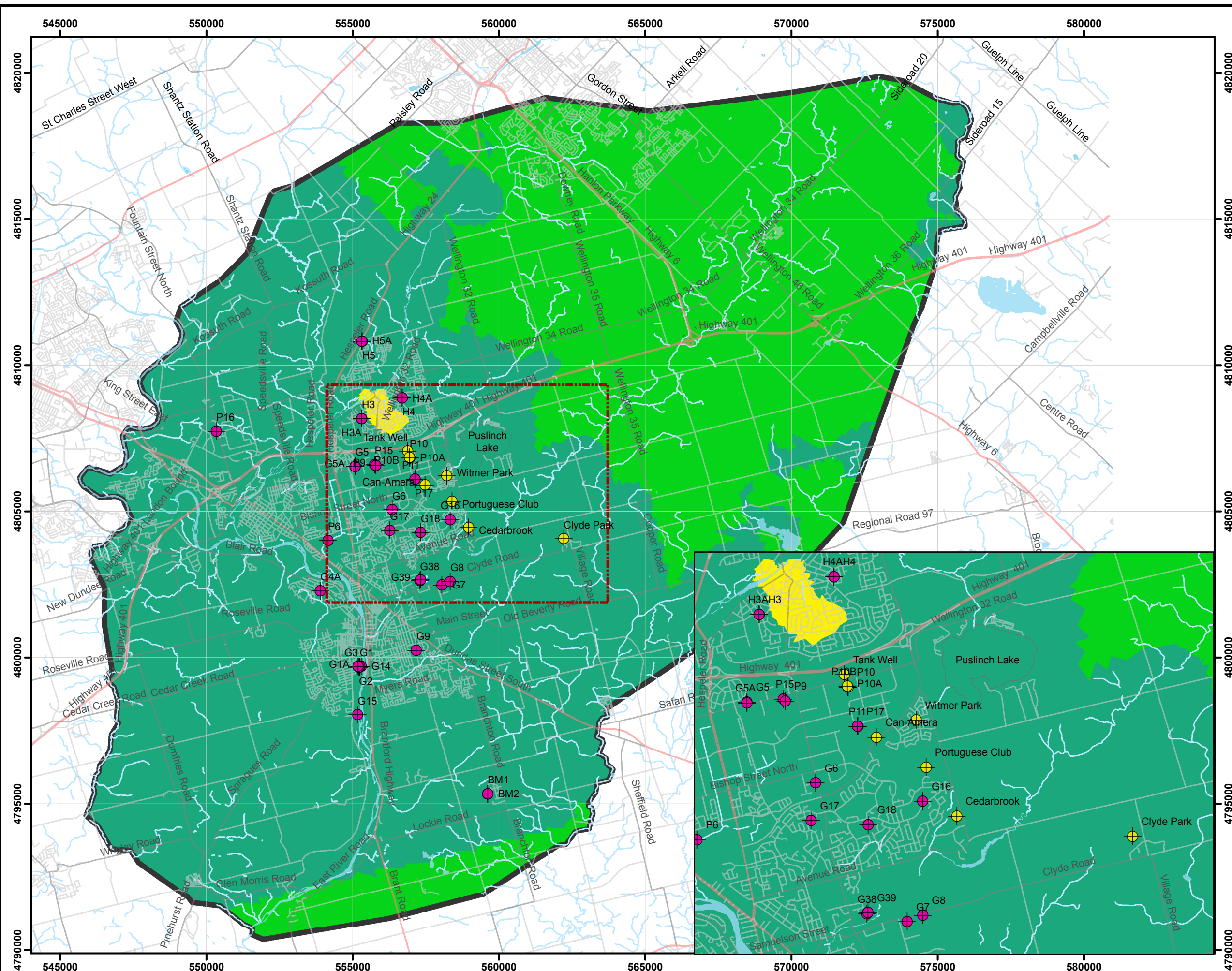
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Vertical Hydraulic Conductivity (m/s)
- 10^{-10}
 - $10^{-10} - 10^{-9}$
 - $10^{-9} - 10^{-8}$
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 2.3 \times 10^{-3}$



Projection: UTM Zone 17N, NAD 83
Map Version: 3, Map data 2015-09-14 J. Zhang



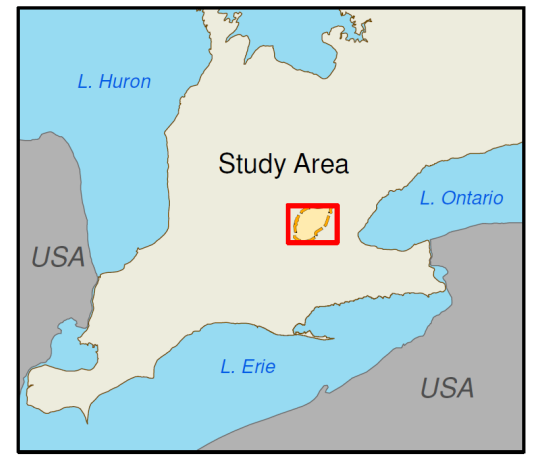
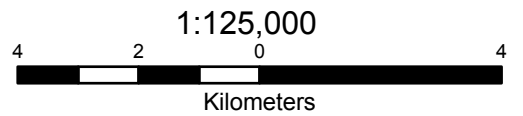
Figure 14
Vertical Hydraulic Conductivity:
Model Layer 7: Overburden



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

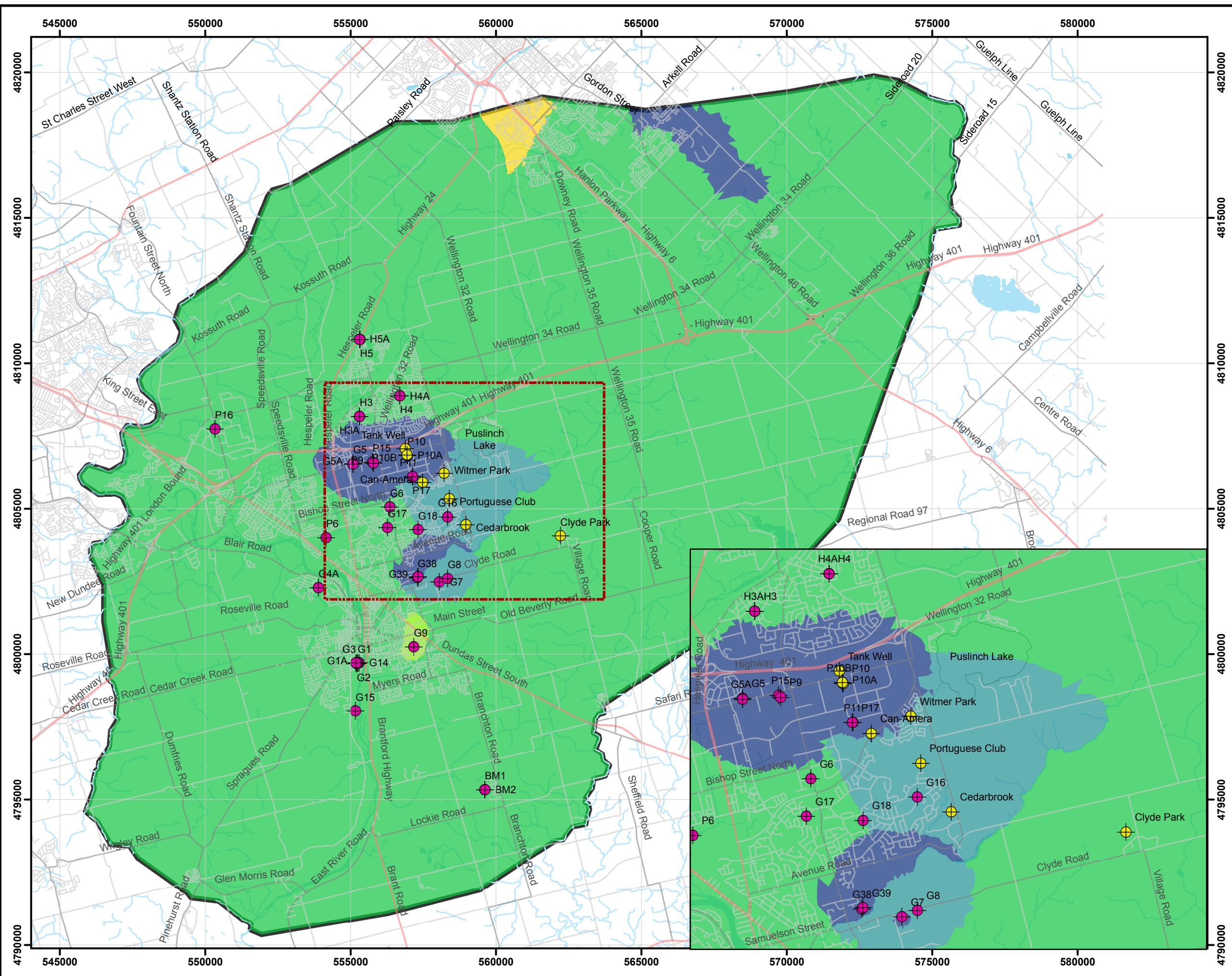
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Hydraulic Conductivity (m/s)
- 10^{-8}
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 1.5 \times 10^{-2}$



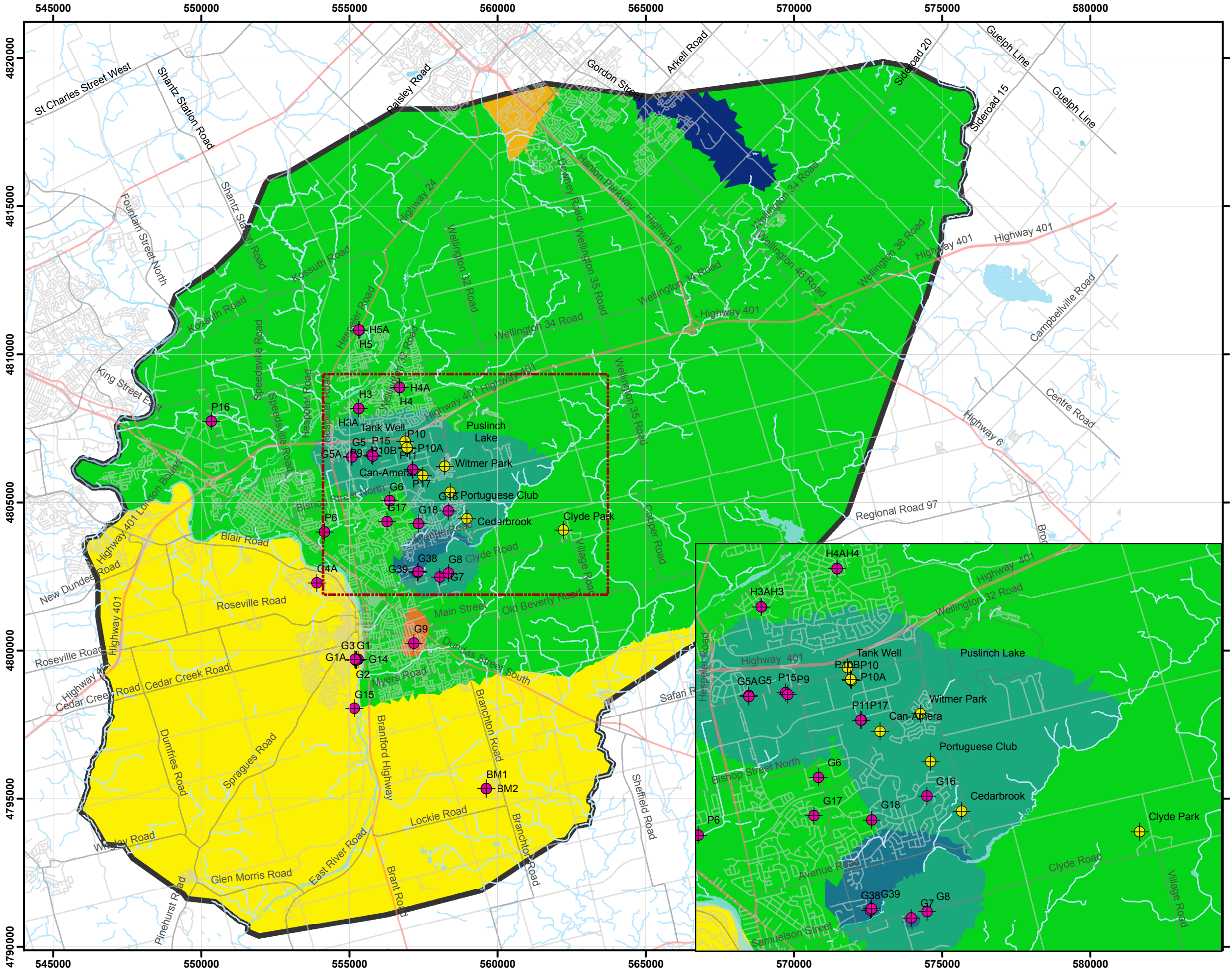
Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



Figure 15
 Horizontal Hydraulic Conductivity:
 Model Layer 8: Contact Aquifer



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

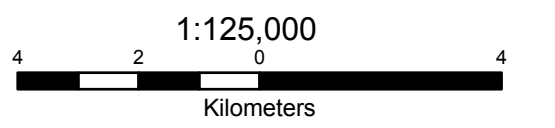


LEGEND

- Municipal Supply Well
- Test Production Well
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds
- Model Domain

Vertical Hydraulic Conductivity (m/s)

- 10^{-10}
- $10^{-10} - 10^{-9}$
- $10^{-9} - 10^{-8}$
- $10^{-8} - 10^{-7}$
- $10^{-7} - 10^{-6}$
- $10^{-6} - 10^{-5}$
- $10^{-5} - 10^{-4}$
- $10^{-4} - 10^{-3}$
- $10^{-3} - 2.3 \times 10^{-3}$

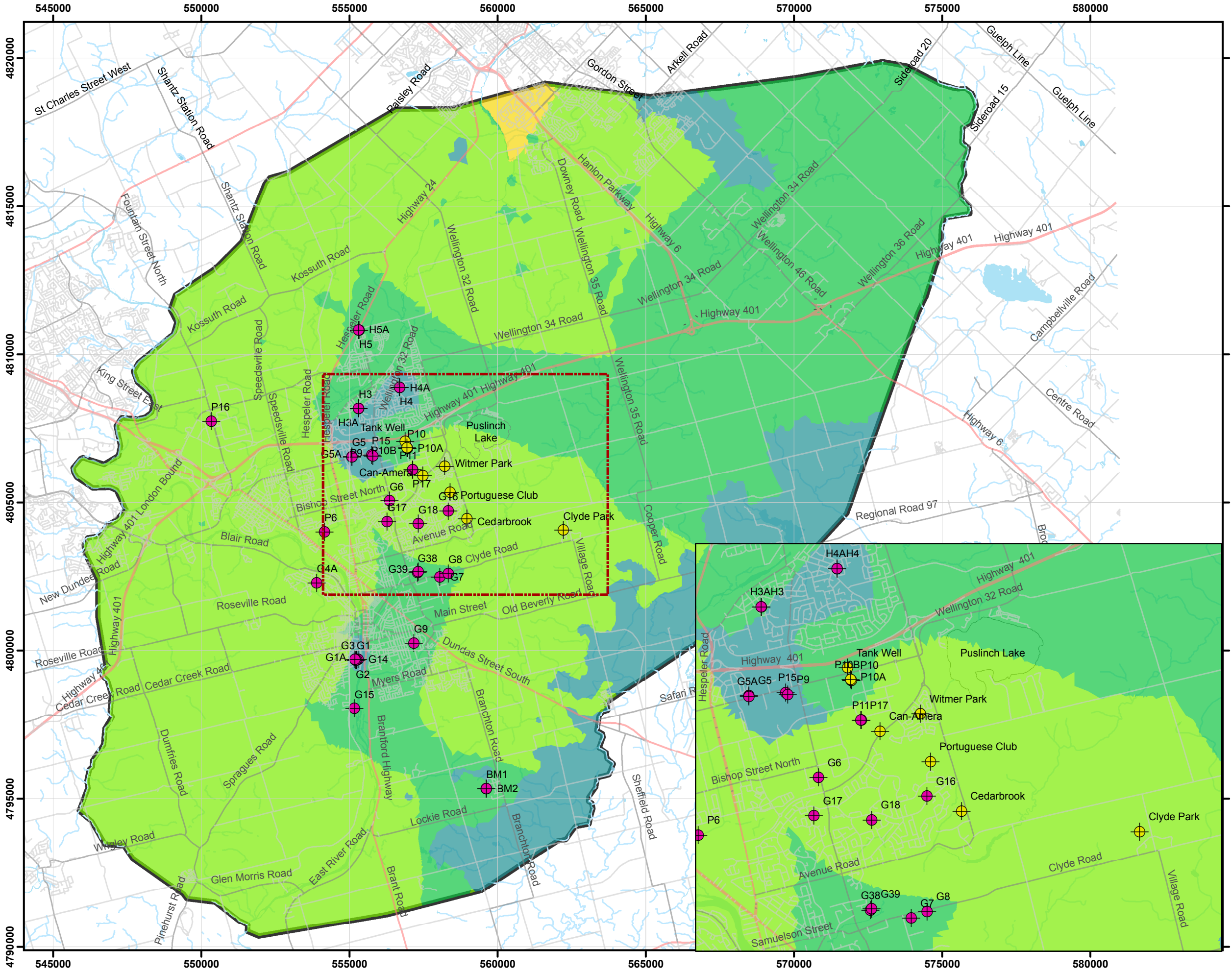


Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



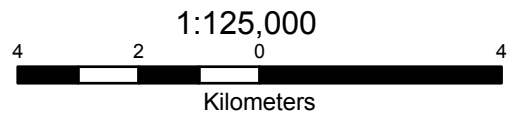
Figure 16
 Vertical Hydraulic Conductivity:
 Model Layer 8: Contact Aquifer

Cambridge East IUS Water Supply Class EA: Groundwater Modelling



LEGEND

- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Hydraulic Conductivity (m/s)
- 10^{-8}
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 1.5 \times 10^{-2}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang

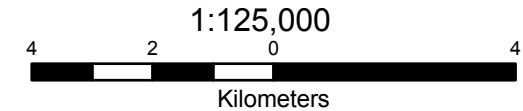


Figure 17
 Horizontal Hydraulic Conductivity:
 Model Layer 9: Guelph Formation

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

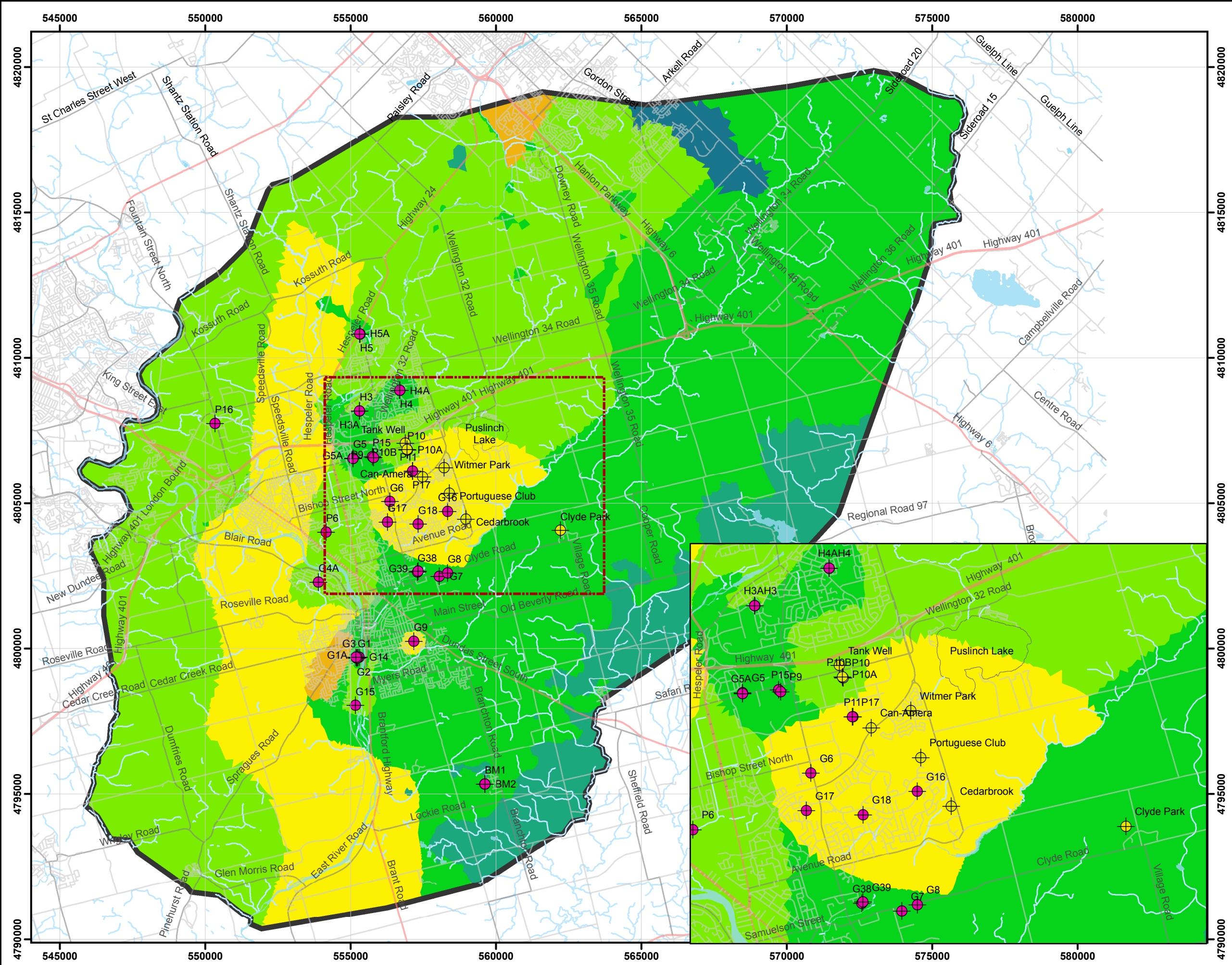
- Municipal Supply Well
- ⊙ Test Production Well
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds
- ▭ Model Domain
- Vertical Hydraulic Conductivity (m/s)
 - 10^{-10}
 - $10^{-10} - 10^{-9}$
 - $10^{-9} - 10^{-8}$
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 2.3 \times 10^{-3}$



Projection: UTM Zone 17N, NAD 83
Map Version: 3, Map data 2015-09-14 J. Zhang



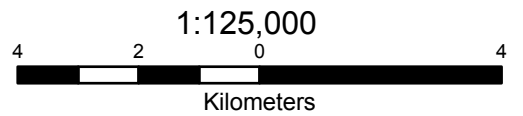
Figure 18
Vertical Hydraulic Conductivity:
Model Layer 9: Guelph Formation



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

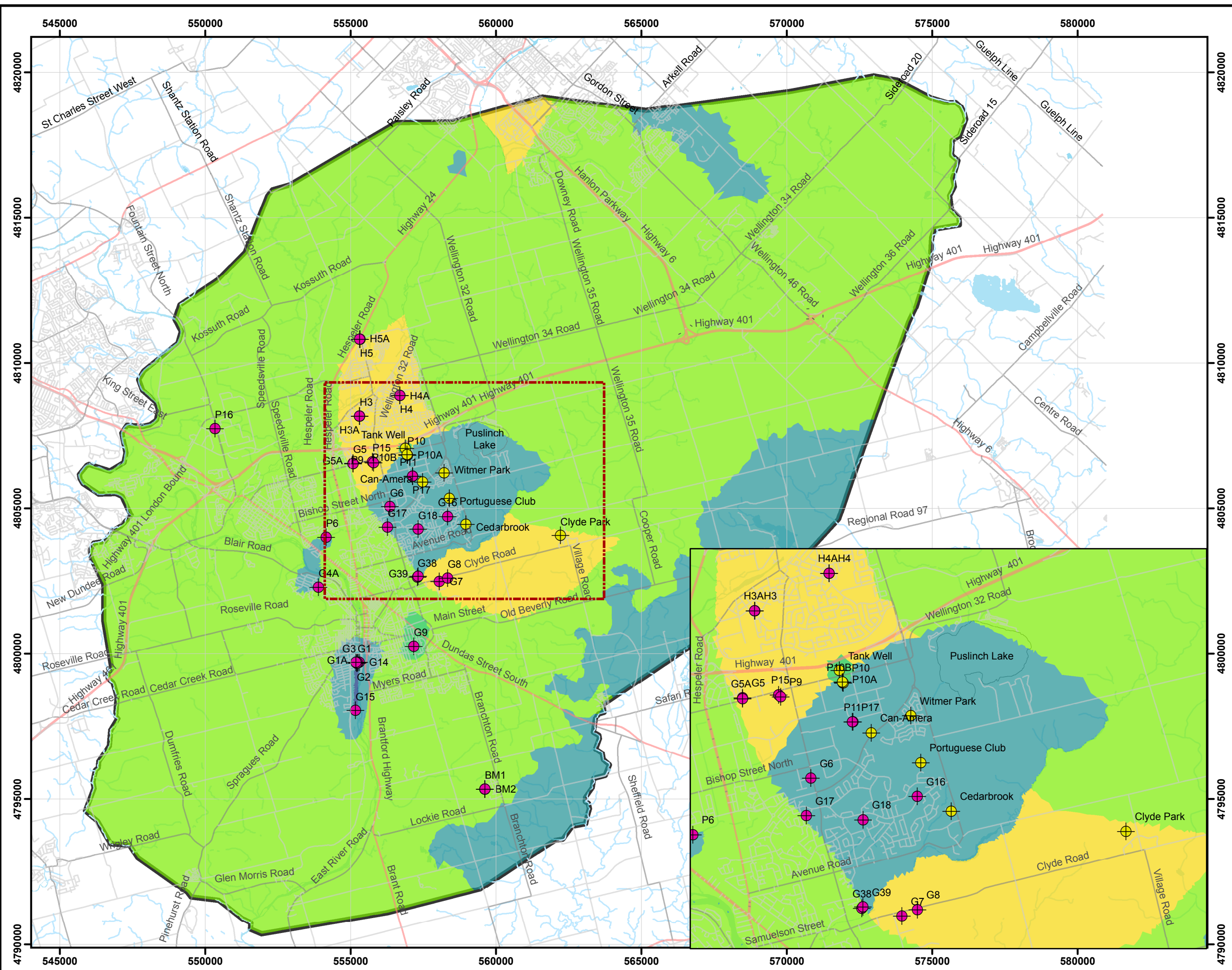
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Hydraulic Conductivity (m/s)
- 10^{-8}
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 1.5 \times 10^{-2}$



Projection: UTM Zone 17N, NAD 83
Map Version: 3, Map data 2015-09-14 J. Zhang



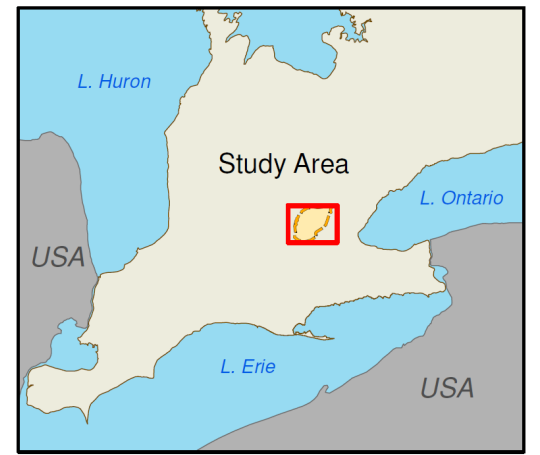
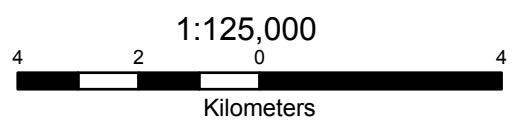
Figure 19
Horizontal Hydraulic Conductivity:
Model Layer 10:
Reformatory Quarry Member



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

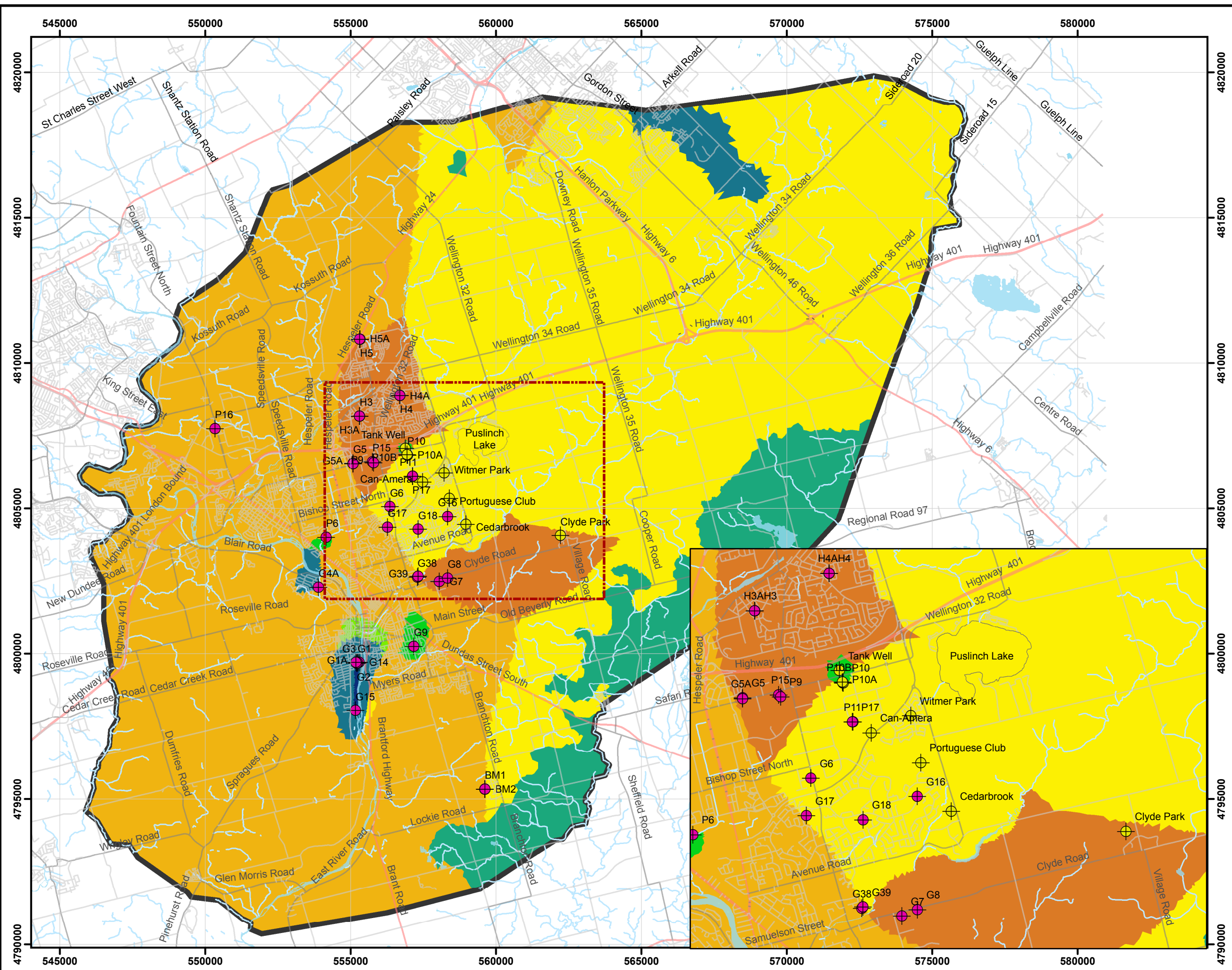
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Vertical Hydraulic Conductivity (m/s)
- 10^{-10}
 - $10^{-10} - 10^{-9}$
 - $10^{-9} - 10^{-8}$
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 2.3 \times 10^{-3}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



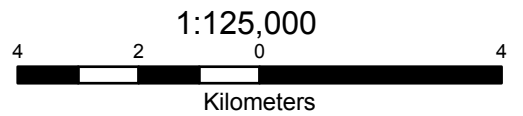
Figure 20
 Vertical Hydraulic Conductivity:
 Model Layer 10:
 Reformatory Quarry Member



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

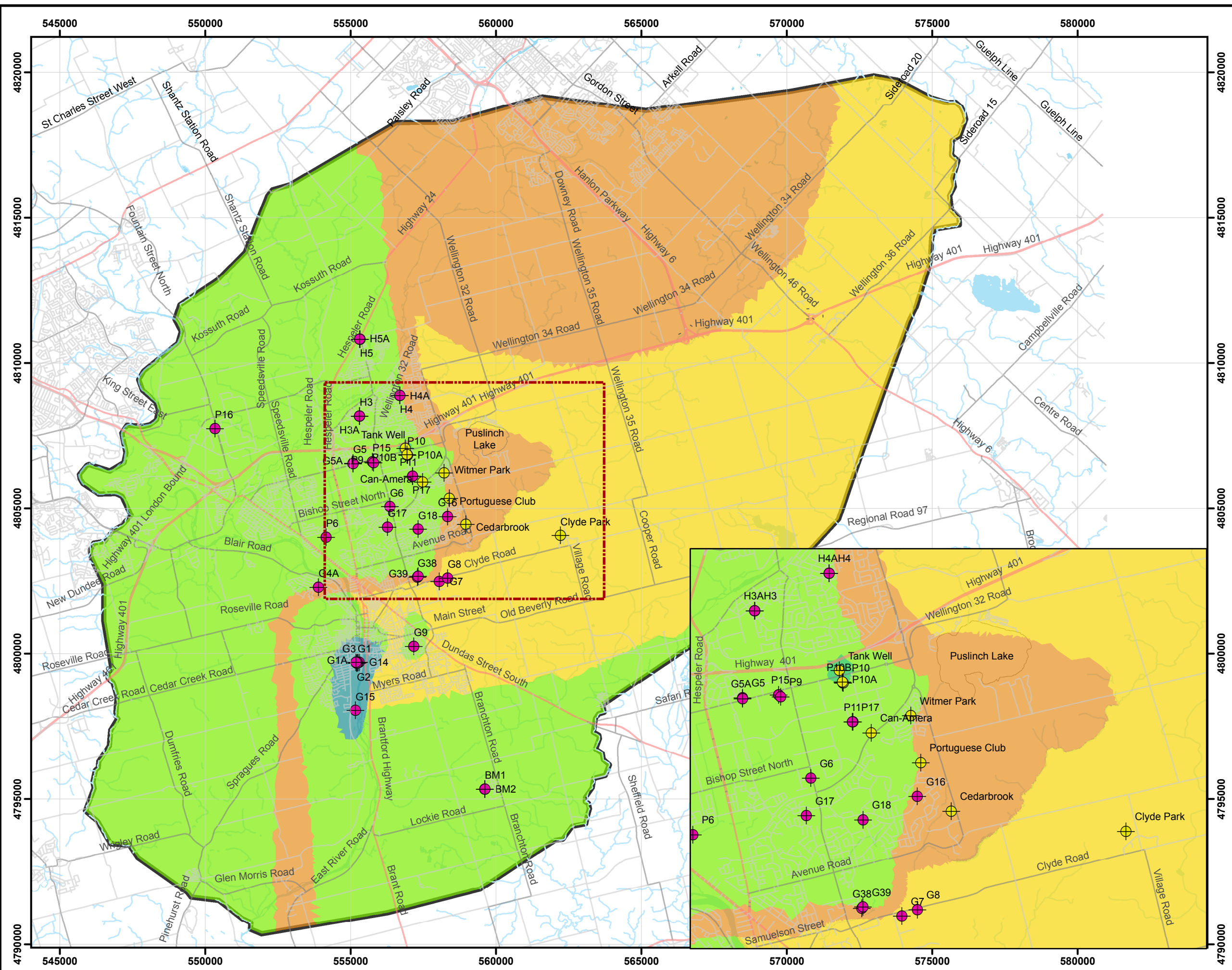
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Hydraulic Conductivity (m/s)
- 10^{-8}
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 1.5 \times 10^{-2}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



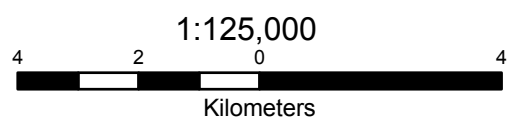
Figure 21
 Horizontal Hydraulic Conductivity:
 Model Layer 11: Vinemount Member



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

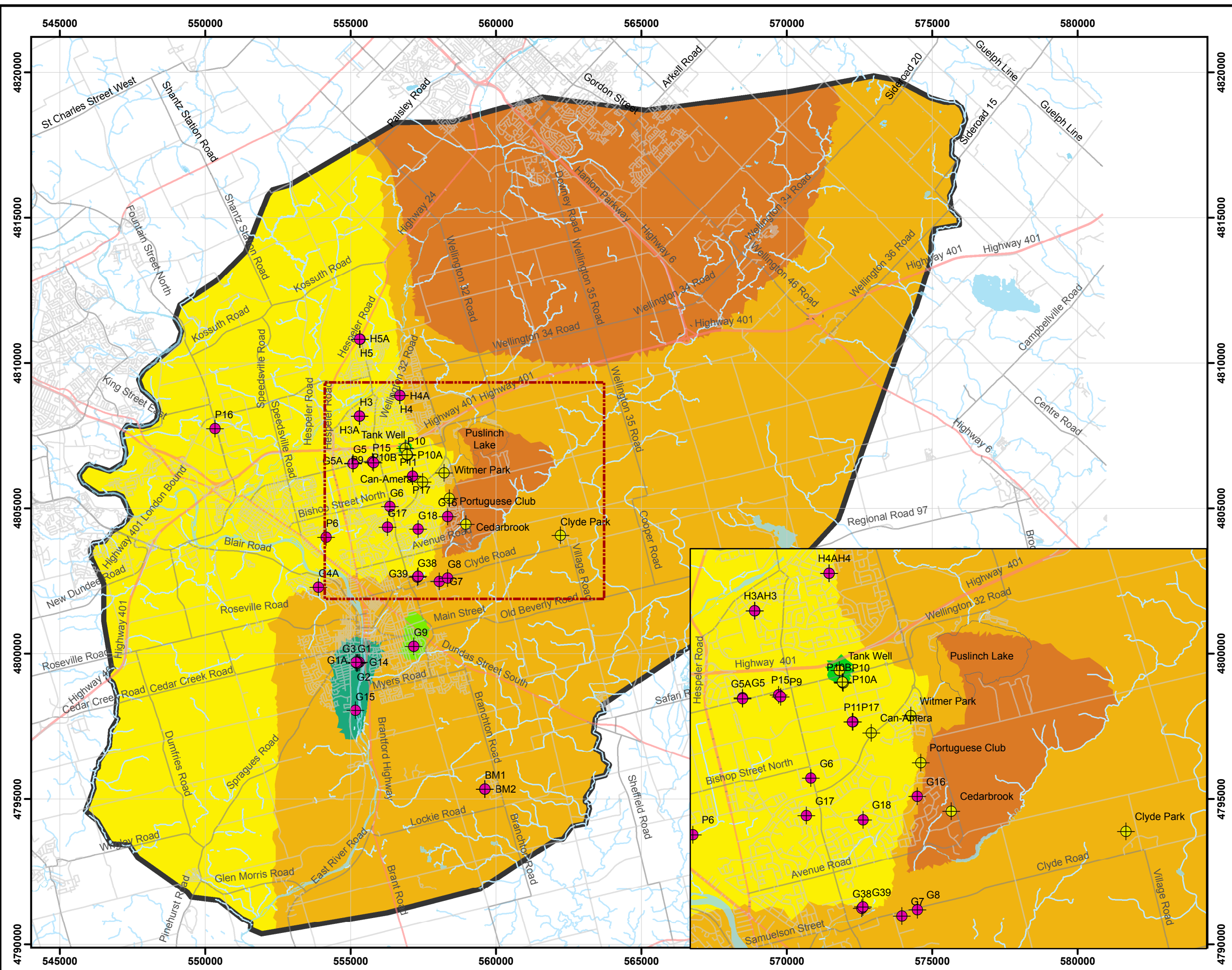
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Vertical Hydraulic Conductivity (m/s)
- 10^{-10}
 - $10^{-10} - 10^{-9}$
 - $10^{-9} - 10^{-8}$
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 2.3 \times 10^{-3}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



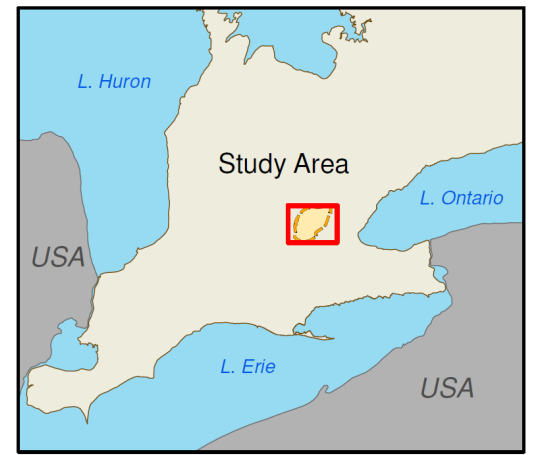
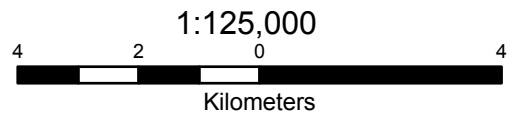
Figure 22
 Vertical Hydraulic Conductivity:
 Model Layer 11:
 Vinemount Member



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

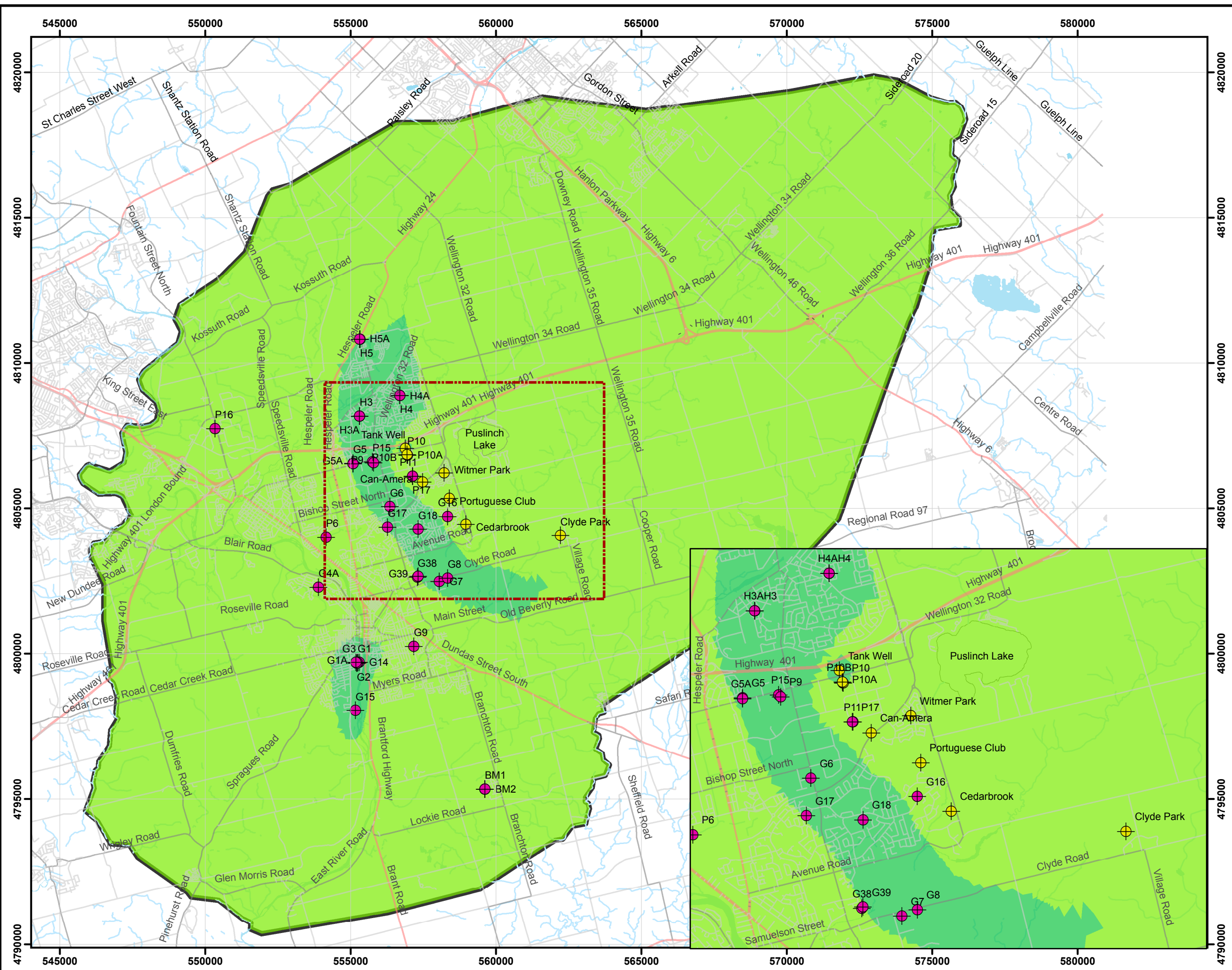
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Hydraulic Conductivity (m/s)
- 10^{-8}
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 1.5 \times 10^{-2}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



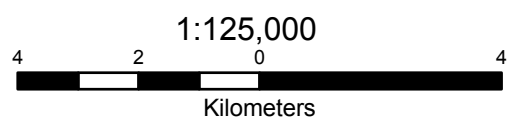
Figure 23
 Horizontal Hydraulic Conductivity:
 Model Layer 12: Goat Island Formation



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

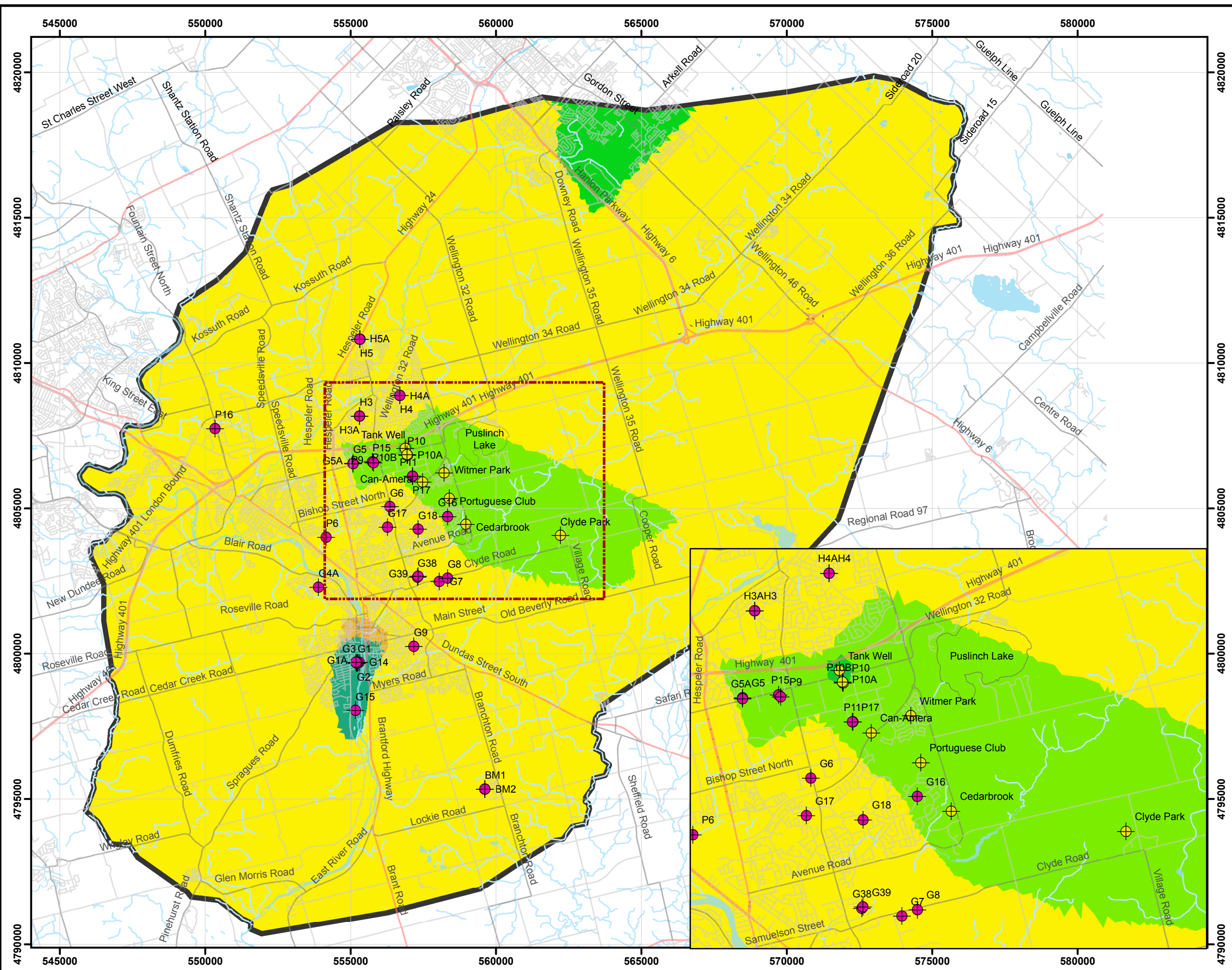
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Vertical Hydraulic Conductivity (m/s)
- 10^{-10}
 - $10^{-10} - 10^{-9}$
 - $10^{-9} - 10^{-8}$
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 2.3 \times 10^{-3}$



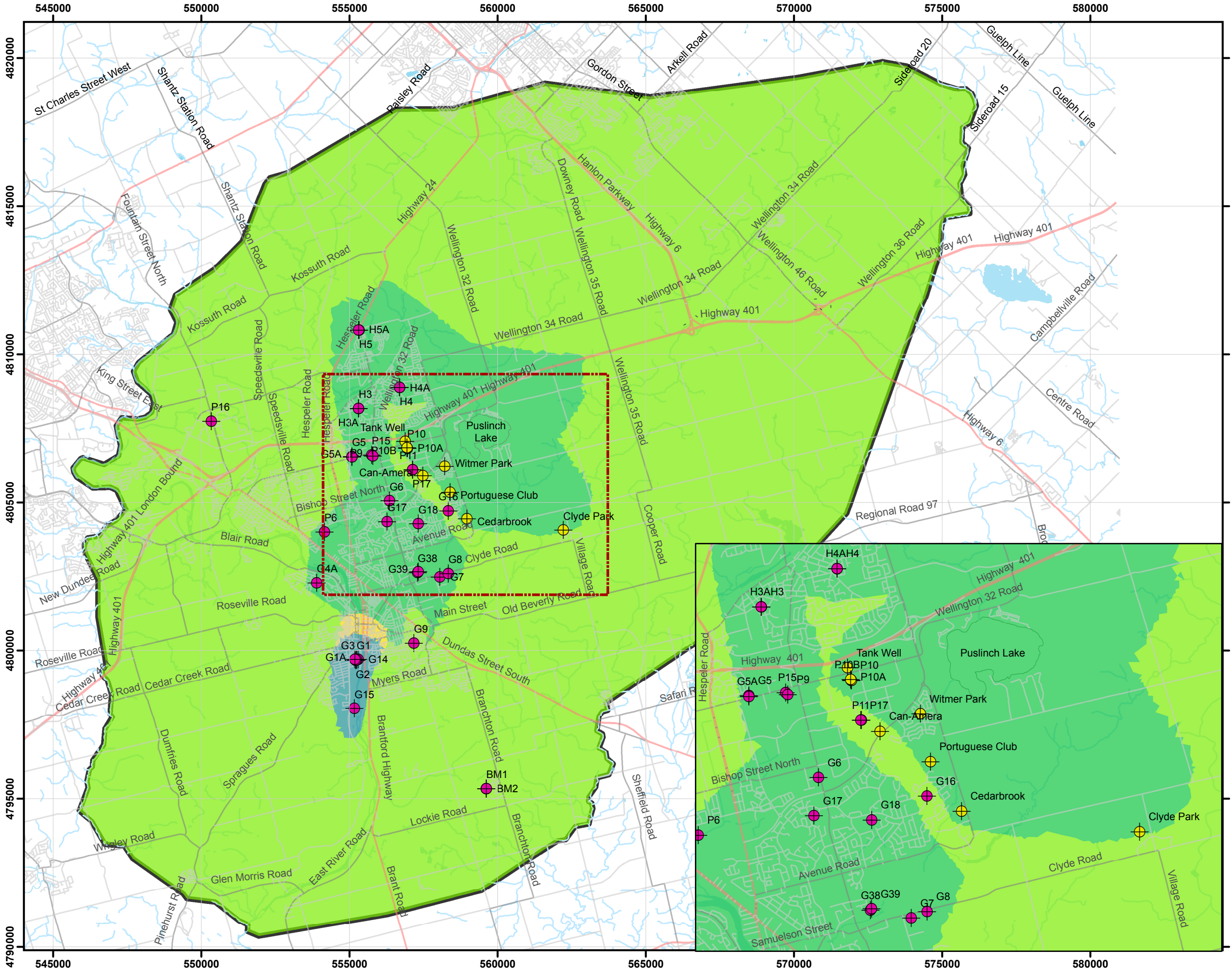
Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



Figure 24
 Vertical Hydraulic Conductivity:
 Model Layer 12:
 Goat Island Formation



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

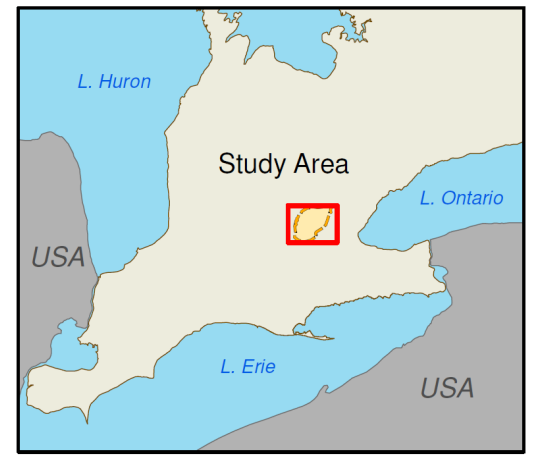
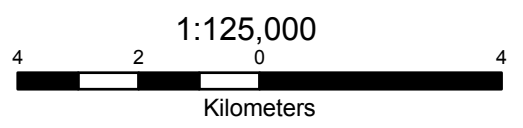


LEGEND

- Municipal Supply Well
- Test Production Well
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds
- Model Domain

Hydraulic Conductivity (m/s)

- 10^{-8}
- $10^{-8} - 10^{-7}$
- $10^{-7} - 10^{-6}$
- $10^{-6} - 10^{-5}$
- $10^{-5} - 10^{-4}$
- $10^{-4} - 10^{-3}$
- $10^{-3} - 1.5 \times 10^{-2}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang

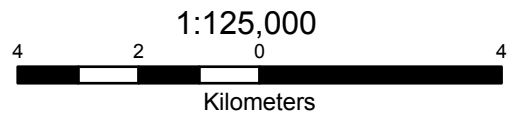


Figure 25
 Horizontal Hydraulic Conductivity:
 Model Layer 13:
 Upper Gasport Formation

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

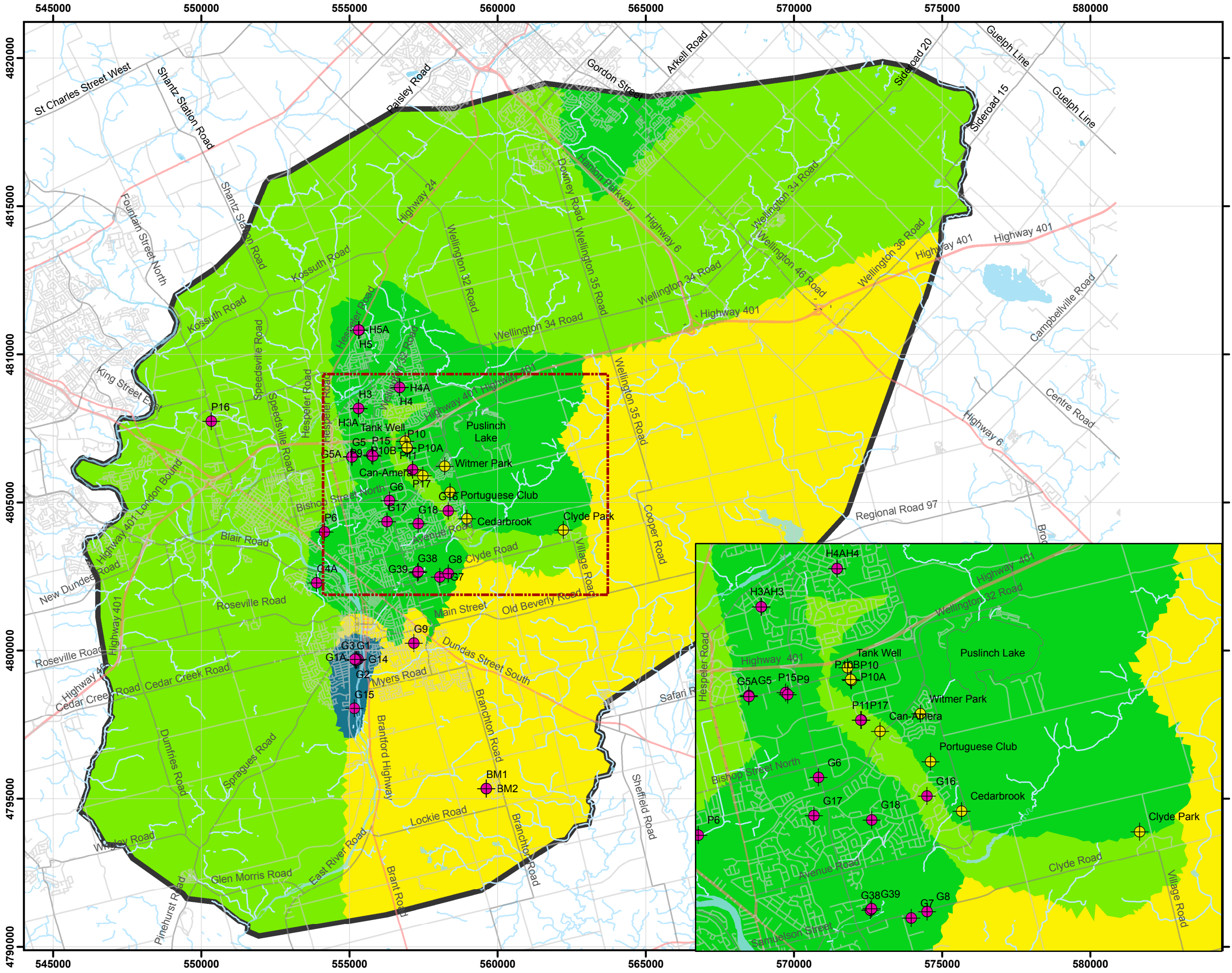
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Vertical Hydraulic Conductivity (m/s)
- 10^{-10}
 - $10^{-10} - 10^{-9}$
 - $10^{-9} - 10^{-8}$
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 2.3 \times 10^{-3}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



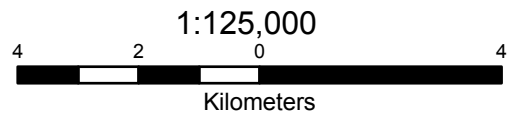
Figure 26
 Vertical Hydraulic Conductivity:
 Model Layer 13:
 Upper Gasport Formation



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

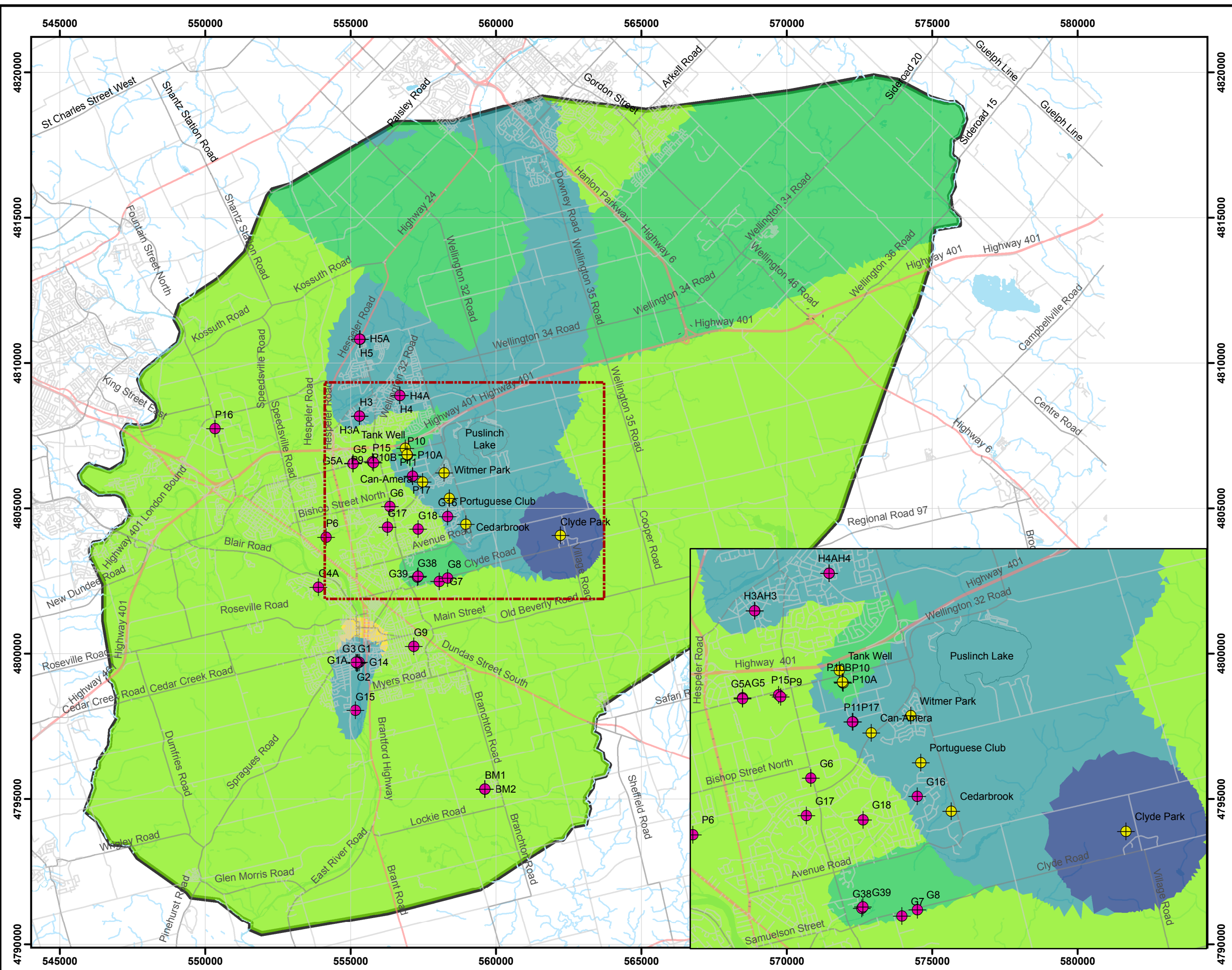
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Hydraulic Conductivity (m/s)
- 10^{-8}
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 1.5 \times 10^{-2}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



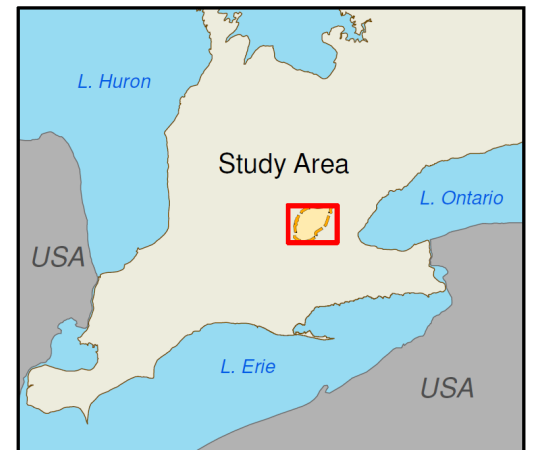
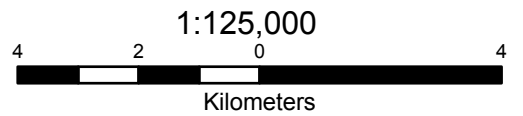
Figure 27
 Horizontal Hydraulic Conductivity:
 Model Layer 14:
 Middle Gasport Formation



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

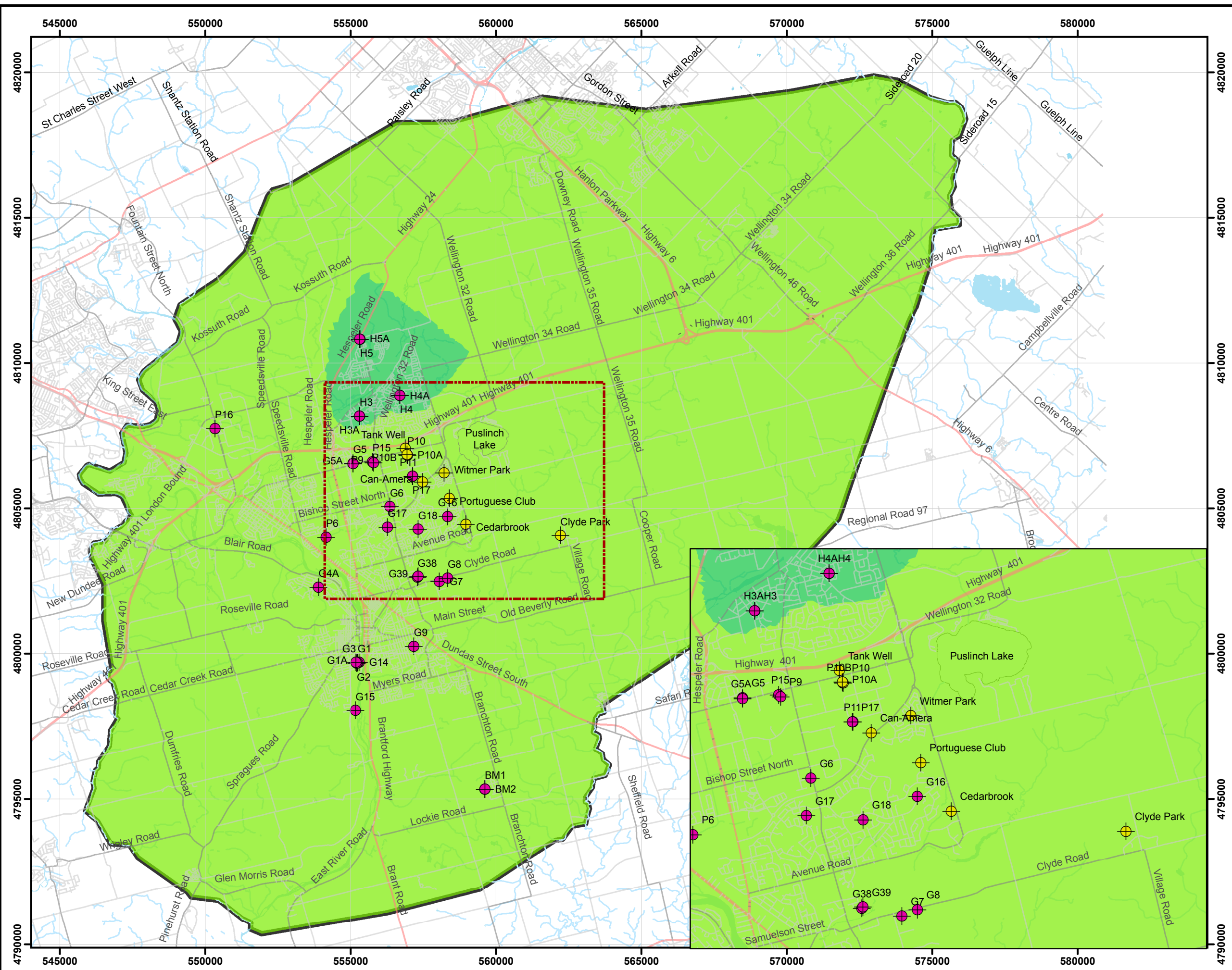
- Municipal Supply Well
 - Test Production Well
 - Expressway / Highway
 - Major Roads
 - Roads (collectors)
 - Rivers / Streams
 - Lakes and Ponds
 - Model Domain
- Hydraulic conductivity (m/s)
- 10^{-8}
 - $10^{-8} - 10^{-7}$
 - $10^{-7} - 10^{-6}$
 - $10^{-6} - 10^{-5}$
 - $10^{-5} - 10^{-4}$
 - $10^{-4} - 10^{-3}$
 - $10^{-3} - 1.5 \times 10^{-2}$



Projection: UTM Zone 17N, NAD 83
 Map Version: 3, Map data 2015-09-14 J. Zhang



Figure 29
 Horizontal hydraulic conductivity:
 Model layer 15:
 Lower Gasport Formation



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

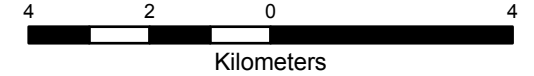
LEGEND

- Municipal Supply Well
- ⊕ Test Production Well
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds
- ▭ Model Domain

Vertical Hydraulic Conductivity (m/s)

- 10⁻¹⁰
- 10⁻¹⁰ - 10⁻⁹
- 10⁻⁹ - 10⁻⁸
- 10⁻⁸ - 10⁻⁷
- 10⁻⁷ - 10⁻⁶
- 10⁻⁶ - 10⁻⁵
- 10⁻⁵ - 10⁻⁴
- 10⁻⁴ - 10⁻³
- 10⁻³ - 2.3x10⁻³

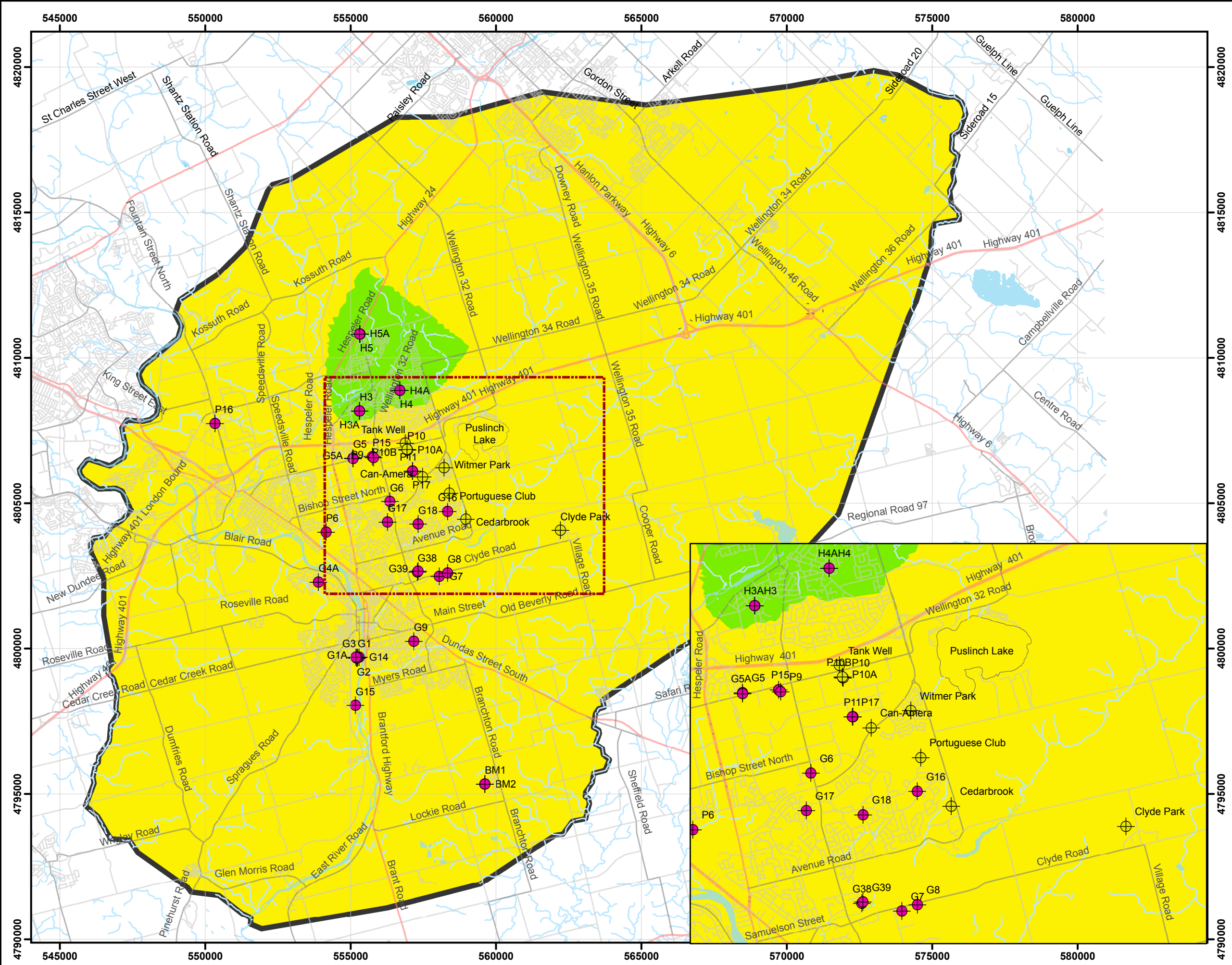
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Projection: UTM Zone 17N, NAD 83
Map Version: 3, Map data 2015-09-14 J. Zhang



Figure 30
Vertical Hydraulic Conductivity:
Model Layer 15:
Lower Gasport Formation



Appendix C

Non-Municipal Permits to Take Water

Permit No.	Easting (NAD83)	Northing (NAD83)	Specific Purpose	Formation Screened ¹	Average Annual Consumptive Demand (m ³ /d)
7710-6DTL4T	552507	4799223	Industrial	Overburden	0
03-P-2003	561056	4814413	Industrial	Overburden	0
72-P-0453	568922	4812609	Industrial	Overburden	396
6064-756QHD	551093	4800879	Industrial	Overburden	1
0717-63RNYF	548622	4797390	Industrial	Overburden	98
3234-74ER7S	568280	4810400	Industrial	Overburden	719
2448-6FUKQ5	567290	4810630	Industrial	Overburden	373
6064-756QHD	551093	4800879	Industrial	Overburden	39
02-P-2002	560055	4810875	Industrial	Overburden	0
3830-6W6JHW	569250	4811950	Industrial	Overburden	993
02-P-2002	559836	4811222	Industrial	Guelph Formation to Lower Gasport Formation	13
03-P-2047	560391	4798971	Industrial	Overburden to Lower Gasport Formation	0
6512-6WGKW8	547590	4798226	Industrial	Overburden	0
01-P-2140	556235	4792616	Commercial	Overburden	0
01-P-2140	556259	4792611	Commercial	Overburden	0
1743-6RGJA9	557585	4792061	Water Supply	Contact Aquifer	0
03-P-2099	548102	4806061	Water Supply	Overburden	3
01-P-2148	563705	4798651	Water Supply	Reformatory Quarry Member	15
8228-76XLE2	569384	4813245	Water Supply	Reformatory Quarry Member	8
8228-76XLE2	569389	4813250	Water Supply	Middle to Lower Gasport Formation	10
3331-73RKYV	569534	4814390	Water Supply	Reformatory Quarry Member to Vinemount Member	20
3331-73RKYV	569537	4814528	Water Supply	Goat Island Formation	33
3331-73RKYV	569499	4814701	Water Supply	Guelph Formation to Vinemount Member	74
3331-73RKYV	569080	4814310	Water Supply	Reformatory Quarry Member	2
1216-6SCL4W	571022	4812087	Industrial	Overburden to Reformatory Quarry Member	16
00-P-2437	557428	4798325	Commercial	Overburden	21
00-P-2791	558358	4808902	Commercial	Overburden	58
03-P-2245	548975	4805598	Commercial	Overburden	27
2540-6PLKFX	553898	4812349	Commercial	Overburden	189
2540-6PLKFX	553771	4812203	Commercial	Overburden	1
00-P-2437	557749	4797831	Commercial	Overburden	7
00-P-2791	558458	4808933	Commercial	Middle Gasport Formation	58
0334-6ARJNR	557026	4804566	Commercial	Overburden	65
03-P-2384	548934	4801969	Commercial	Contact Aquifer to Reformatory Quarry Member	204
03-P-2384	548964	4801644	Commercial	Contact Aquifer to Guelph Formation	197
03-P-2384	548940	4801662	Commercial	Contact Aquifer to Guelph Formation	191
00-P-2176	555310	4799583	Remediation	Overburden to Guelph Formation	10
00-P-2176	555345	4799590	Remediation	Overburden to Guelph Formation	7
01-P-2245	570901	4815964	Miscellaneous	Overburden	5
01-P-2245	570784	4815980	Miscellaneous	Contact Aquifer	32
3878-6Z3JUU	551100	4807800	Dewatering	Overburden	43
3878-6Z3JUU	551300	4807300	Dewatering	Overburden	14
3878-6Z3JUU	551300	4807400	Dewatering	Overburden	2
3878-6Z3JUU	550900	4807800	Dewatering	Overburden	104
3878-6Z3JUU	551400	4807300	Dewatering	Overburden	3
3878-6Z3JUU	551100	4807800	Dewatering	Overburden	20
3878-6Z3JUU	551000	4807300	Dewatering	Overburden	0
2817-6W9P32	549872	4801566	Industrial	Overburden	18
89-P-2014	569462	4812611	Industrial	Reformatory Quarry Member	5
5201-6B7HDA	567598	4812203	Industrial	Middle Gasport Formation	2
5201-6B7HDA	567476	4812030	Industrial	Goat Island Formation	12
5201-6B7HDA	567608	4811999	Industrial	Goat Island Formation	16

¹ The screened formation are inferred from the hydrostratigraphic model using the available screen information of the pumping wells

Permit No.	Easting (NAD83)	Northing (NAD83)	Specific Purpose	Formation Screened ¹	Average Annual Consumptive Demand (m ³ /d)
7724-73VQCX	551270	4799187	Industrial	Overburden	147
7724-73VQCX	551402	4799113	Industrial	Overburden	42
6661-65YPPD	558706	4801766	Remediation	Overburden	2,939
0700-6YTS5P	557183	4801047	Remediation	Overburden	46
0700-6YTS5P	557018	4801061	Remediation	Overburden	9
0700-6YTS5P	557049	4801038	Remediation	Overburden	0
0882-6FTHMA	566318	4816054	Water Supply	Overburden	9
0882-6FTHMA	566388	4816161	Water Supply	Overburden	10
0882-6FTHMA	566425	4815893	Water Supply	Guelph Formation to Upper Gasport Formation	0
2563-6VKR7T	552019	4811557	Agricultural	Overburden	3
03-P-2408	552053	4812678	Agricultural	Guelph Formation	4
00-P-2681	551316	4808472	Agricultural	Reformatory Quarry Member	232
00-P-2782	559540	4807988	Water Supply	Overburden	35
00-P-2782	559451	4808040	Water Supply	Guelph Formation	35
00-P-2782	559838	4808018	Water Supply	Contact Aquifer	35
01-P-2148	563728	4798440	Water Supply	Overburden	15
01-P-2148	563624	4798256	Water Supply	Contact Aquifer to Guelph Formation	15
02-P-2064	569356	4811814	Agricultural	Vinemount Member	39
0302-7CEL63	560648	4809366	Commercial	Reformatory Quarry Member	210
0302-7CEL63	560679	4809389	Commercial	Reformatory Quarry Member	100
0302-7CEL63	560725	4809289	Commercial	Reformatory Quarry Member	273
1065-5VFQ9K	564140	4815443	Industrial	Contact Aquifer to Lower Gasport Formation	0
1743-6RGJA9	557585	4792061	Water Supply	Contact Aquifer	0
2688-7BRKDG	557476	4801412	Remediation	Overburden to Contact Aquifer	213
2688-7BRKDG	557425	4801474	Remediation	Overburden to Contact Aquifer	115
2688-7BRKDG	557408	4801531	Remediation	Overburden	38
2768-6QXRCC	557427	4815114	Industrial	Guelph Formation to Goat Island Formation	79
3615-79ULJX	548334	4800724	Commercial	Overburden	438
4084-7AXSP4	550012	4803179	Water Supply	Overburden	1,800
4116-7CELMY	560640	4809381	Commercial	Reformatory Quarry Member	264
4116-7CELMY	560679	4809412	Commercial	Reformatory Quarry Member	196
4116-7CELMY	560725	4809296	Commercial	Reformatory Quarry Member	273
5170-6X9H33	568312	4816988	Commercial	Overburden	186
6031-7CVQ3X	555875	4809552	Dewatering	Guelph Formation to Vinemount Member	26
6066-6VXQU2	561196	4803854	Water Supply	Guelph Formation to Middle Gasport Formation	34
6066-6VXQU2	561060	4803785	Water Supply	Overburden	50
6066-6VXQU2	561189	4803847	Water Supply	Guelph Formation to Middle Gasport Formation	655
6560-6DYPGH	570188	4811581	Industrial	Overburden	250
6560-6DYPGH	569847	4811446	Industrial	Overburden	200
7316-6GAJ3X	547976	4800065	Industrial	Overburden	0
7316-6GAJ3X	547738	4799810	Industrial	Overburden	0
74-P-2065	566615	4800491	Commercial	Reformatory Quarry Member	12
7403-6HCMGT	563572	4802239		Overburden	1
7403-6HCMGT	563566	4802151	Recreational	Overburden	1
7403-6HCMGT	563569	4802197		Overburden	1
8228-76XLE2	569616	4813435	Water Supply	Contact Aquifer to Reformatory Quarry Member	0
8228-76XLE2	569536	4813137	Water Supply	Contact Aquifer to Middle Gasport Formation	0
88-P-2029	566601	4800523	Water Supply	Middle Gasport Formation	0
88-P-2069	558681	4816893	Industrial	Vinemount Member	655
88-P-2070	559242	4792452	Commercial	Guelph Formation to Reformatory Quarry Member	0
88-P-2070	559246	4792542	Commercial	Contact Aquifer	0
98-P-2064	569203	4814403	Water Supply	Goat Island Formation	164

Permit No.	Easting (NAD83)	Northing (NAD83)	Specific Purpose	Formation Screened ¹	Average Annual Consumptive Demand (m ³ /d)
99-P-2132	569100	4806163	Commercial	Contact Aquifer to Vinemount Member	1,637
7043-74BL3K	561798	4817015	Water Supply	Guelph Formation to Lower Gasport Formation	3,940
7710-6DTL4T	568935	4812721	Commercial	Contact Aquifer to Upper Gasport Formation	2,396

Appendix D

Simulated vs. Measured Groundwater Elevations for Calibrated Model (Steady-State)

Table D-1 Groundwater level targets derived from MOE water well records

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
1000488	DW-BULLOCK	A	559895	4798023	281.64	277.47	3
1000491	DW-BUDIMLIC	A	555417	4795658	266.50	272.84	7
1000507	DW-DETLON	A	553996	4809745	300.75	299.82	10
1000512	OW19-94	A	549815	4811371	304.12	307.54	9
1000523	DW-PANDEIRADA	A	562374	4803799	298.30	294.89	3
6500454	DW	A	559163	4799061	286.52	281.33	3
6500462	DW	A	559063	4799221	286.52	281.29	4
6500488		A	558798	4798811	287.15	280.60	4
6500576		A	557653	4800651	278.28	276.85	6
6500603		A	558323	4802581	283.86	282.51	6
6500604		A	558223	4802581	283.76	282.30	6
6500607	DW	A	552133	4801081	307.45	307.48	6
6500609	DW	A	551913	4800931	309.42	308.85	6
6500610	DW	A	551583	4800991	305.42	307.40	5
6500642	AVE-2A	A	559166	4803651	292.92	293.18	9
6500648	AVE-5	A	558827	4803822	275.96	292.73	9
6500653		A	557538	4802471	278.59	280.50	3
6500740	DW	A	555613	4797096	253.29	263.89	6
6500892	TW38-65	A	546138	4806421	281.64	280.15	6
1000544	DW-REINHARDT	A	549842	4808548	285.56	299.17	6
6500955	DW	A	547373	4804301	280.73	286.88	7
6500978	DW	A	549123	4805946	287.55	278.71	6
6500980	DW	A	549233	4805786	268.63	276.55	8
6500981	DW	A	549553	4805991	286.82	279.30	7
6500985	DW	A	549293	4805901	286.21	278.09	7
1000546	DW-CHARLEBOIS	A	549892	4808623	286.01	299.49	7
6500990	DW	A	549213	4805941	283.46	278.39	7
6500991	DW	A	548953	4806001	284.99	279.79	7
6501002	DW	A	548753	4805981	285.60	280.62	6
6501005	DW	A	548508	4806231	283.17	283.10	8
6501021	DW	A	548233	4806741	286.52	285.71	7
6501022	DW	A	548018	4806981	282.86	286.12	7
6501023	DW	A	548153	4806871	278.59	286.01	7
6501024	DW	A	548038	4806846	286.31	285.78	7
1000547	DW-BECHTEL	A	549867	4808619	287.76	299.48	6
6501028	DW	A	547853	4806831	280.70	285.45	7
6501030	DW	A	548133	4806771	278.24	285.68	7
6501036	DW	A	548173	4806811	281.34	285.85	7
6501037	DW	A	548113	4806781	280.42	285.68	7
6501043	OW17-65	A	546113	4806371	280.42	280.15	7
6501044	OW18-65	A	546138	4805666	276.80	278.71	8
6501045	OW19-65	A	545813	4805921	276.99	279.40	8
6501048	TW32-65	A	546233	4806271	279.81	280.25	9
6501049	TW33-65	A	546543	4805891	284.73	280.17	7

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6501050	TW34-65	A	546513	4806246	282.30	280.46	7
1000548	DW77-95	A	550970	4809772	293.95	306.84	6
6501051	TW35-65	A	546678	4806221	285.49	280.53	7
6501052	TW36-65	A	546063	4805721	277.68	278.67	7
6501053	TW37-65	A	546413	4806321	282.55	280.44	7
6501080	DW	A	548543	4808001	287.12	290.70	7
6501083	DW	A	548573	4807881	284.38	290.24	7
6501088	DW	A	548553	4807861	284.38	290.12	7
6501116	DW	A	549613	4805781	281.95	277.31	8
6501229	DW	A	548088	4804366	282.38	284.05	7
6501233	DW	A	548063	4804471	276.46	283.61	7
6501235	DW	A	547568	4804621	277.13	284.26	7
6501237	DW	A	548118	4804621	278.77	282.71	8
1000552	DW-MILLER	A	560574	4804012	296.02	297.46	6
6501655	DW	A	548663	4810186	287.72	295.23	8
1000556	DW75-95	A	564226	4800651	265.28	264.97	10
1000557	DW-MATHEWS	A	555514	4796060	257.71	265.59	9
1000560	DW-ADAIR	A	559749	4795713	270.71	263.72	10
1000563	DW-CORVAGLIA	A	554913	4793874	287.77	265.34	7
1000565	DW-SOARES	A	548188	4793518	293.59	297.55	5
6502927	DW	A	560143	4795211	269.15	258.09	8
6502934	DW	A	554893	4796061	288.35	283.22	5
6502935	DW	A	556533	4795521	259.69	266.28	8
6502937	DW	A	558213	4800921	278.90	280.92	6
6502938	DW	A	560053	4795451	269.14	259.95	8
6502939	DW	A	557873	4800321	278.90	278.64	8
6502991	DW	A	555043	4796091	284.38	280.37	6
6502994	DW	A	557363	4799051	274.62	276.49	4
6503036	DW	A	555093	4796161	275.84	274.44	8
6503039	DW	A	559923	4801341	287.43	287.29	5
6503041	DW	A	555993	4793761	256.25	262.61	7
6503057		A	549573	4810001	294.13	303.73	9
6503059	AVE-30	A	557463	4803391	281.24	285.91	9
6503062	DW	A	554953	4796171	284.69	281.62	5
6503064	DW	A	555353	4794831	260.00	272.31	10
6503065		A	556963	4810971	292.92	293.56	10
6503091	DW	A	555723	4797531	258.17	263.60	9
6503092	DW	A	557943	4799351	280.42	278.97	5
6503095	AVE-31	A	557413	4803401	281.68	285.76	9
6503096	AVE-35	A	557213	4803321	281.49	284.78	9
6503097	AVE-33	A	557363	4803371	280.35	285.46	9
1000581	DW-91312 ONTARIO INC	A	547494	4802773	298.78	296.06	6
6503110	DW	A	555713	4796661	255.64	264.54	9
6503123	DW	A	547813	4807421	288.65	287.63	6

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6503124	DW	A	553863	4809571	296.90	299.49	10
6503129	DW	A	558583	4799411	284.99	280.47	9
6503130	DW	A	549663	4801761	303.41	300.81	6
1000583	DW-KRAEMER	A	549791	4805402	279.89	274.65	7
6503147	DW	A	557963	4799321	282.25	279.01	9
6503149	DW	A	552493	4793951	298.04	296.83	6
6503150	TLR-28	A	559124	4804147	299.83	297.30	5
6503154	TW5-69	A	551173	4800811	306.33	310.01	6
6503167	TW5-69 (McLAUGHLIN)	A	551766	4803878	270.25	277.01	10
6503178		A	549093	4808181	284.99	294.95	9
6503189	DW	A	551153	4799331	307.85	318.40	6
6503200	DW	A	555903	4811821	300.07	302.07	10
6503205	DW	A	557423	4808591	300.80	298.89	9
6503217		A	552733	4808471	296.88	298.08	10
6503226	DW	A	548413	4809581	282.97	295.26	9
6503234	DW	A	551513	4813471	311.21	317.58	6
6503255	DW	A	555613	4797525	257.56	261.93	9
6503257	DW	A	561313	4795881	254.81	249.80	10
6503278	DW	A	554253	4811921	303.25	306.66	4
6503282	DW	A	557598	4798536	281.27	277.37	4
6503301	DW	A	559833	4796941	275.65	271.18	10
6503302	DW	A	558501	4795611	266.70	267.07	10
6503315	DW	A	560313	4794811	264.07	251.69	8
6503316	DW	A	559023	4794719	268.23	262.08	4
6503322	DW	A	558103	4800881	280.12	280.97	6
6503327	DW	A	557613	4798426	277.16	276.99	4
6503355	DW	A	549773	4805871	284.46	278.36	7
6503366	DW	A	557263	4809821	304.55	300.22	13
6503380	DW	A	560053	4795531	268.53	260.35	8
6503382	DW	A	550313	4794111	297.66	300.45	6
6503405	TW1-71	A	557876	4803200	286.90	286.99	3
6503415	DW	A	553743	4807821	280.72	284.35	10
6503417	DW	A	548383	4809521	282.91	295.17	8
6503444	DW	A	557948	4797601	281.03	276.08	9
6503460	TW2-71	A	557653	4803280	286.45	286.48	6
6503462	TW4-71	A	557863	4802821	284.08	282.71	3
6503463	TW5-71	A	558148	4801581	282.77	281.64	7
6503470	DW	A	559713	4795711	270.66	264.04	10
6503471	DW	A	557943	4799381	280.73	278.99	9
6503472	DW	A	560033	4796621	275.24	268.19	10
6503473		A	553538	4807501	278.59	284.96	10
6503475	DW	A	559683	4797231	274.02	274.27	9
6503486	DW	A	560113	4795251	270.06	258.61	5
6503487	DW	A	560103	4795391	268.23	260.74	4

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6503488	DW	A	557533	4798791	278.59	277.27	9
6503496	DW	A	548263	4807701	282.13	289.08	6
6503497	DW	A	554053	4815071	317.74	320.71	9
6503502	DW	A	548663	4806071	281.33	281.47	7
6503540	DW	A	559963	4795296	269.75	259.88	5
6503541	DW	A	547533	4793506	293.22	296.17	6
6503563	DW	A	551738	4796471	300.23	317.53	5
6503571	AVE-17	A	558888	4803821	287.73	296.45	5
6503636	DW	A	559963	4795091	265.39	257.77	9
6503637	DW	A	560063	4795121	267.93	257.56	9
6503642	DW	A	555063	4796171	275.08	274.71	9
6503644	DW	A	560013	4795121	268.73	257.90	8
6503681		A	555993	4807561	302.05	293.99	9
6503684	DW	A	559913	4795171	267.40	259.23	8
6503685	DW	A	559913	4795196	268.01	259.49	5
6503686	DW	A	559913	4795221	269.75	260.92	4
6503687	DW	A	550393	4800721	305.11	312.86	5
6503692	DW	A	554013	4794971	298.45	301.38	5
6503700	DW	A	553663	4809421	299.01	300.46	10
6503706	TLR-11	A	558553	4805630	305.06	299.03	6
6503708		A	560038	4799996	283.66	284.49	7
6503716	DW	A	553463	4797871	309.21	301.22	6
6503717	DW	A	548413	4793671	295.44	297.92	6
6503740	DW	A	559963	4795201	270.36	259.23	5
6503741	DW	A	559943	4795241	267.92	259.44	9
6503742	DW	A	559933	4795261	269.45	259.64	9
6503745	AVE-25	A	557863	4803631	285.49	288.15	9
6503747	DW	A	558653	4801001	284.71	282.82	6
6503806	DW	A	560903	4798841	274.63	278.55	9
6503807	DW	A	560953	4799141	270.37	279.79	9
6503808	DW	A	560963	4798681	276.15	277.43	9
6503809	DW	A	561293	4798181	264.57	269.49	9
6503814	DW	A	561313	4798511	269.76	273.85	9
6503819	DW	A	562033	4798721	258.78	266.26	10
6503828	DW	A	563583	4799241	263.66	261.88	10
6503837	DW	A	561033	4799231	277.07	280.23	9
6503838	DW	A	560813	4799931	283.17	285.12	9
6503839	DW	A	561023	4799401	276.76	281.82	9
6503840		A	560893	4799511	279.20	282.99	9
6503848	DW	A	560913	4800271	286.51	286.52	9
6503849	DW	A	560903	4801201	288.04	288.47	8
6503855	DW	A	563343	4801781	280.42	277.64	10
6503857	DW	A	563993	4800581	262.08	265.01	10
6503865	DW	A	564583	4800521	267.62	264.35	10
6503868	DW	A	559863	4802921	298.70	291.86	3

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6503873	DW	A	561053	4801311	291.70	289.33	3
6503874	DW	A	561393	4801481	291.14	289.41	9
6503881	DW	A	562073	4803301	297.19	293.67	9
6503882	DW	A	562393	4803331	295.05	292.71	9
6503887		A	563553	4803861	292.68	288.15	9
6503888	DW	A	563993	4802041	273.71	270.57	3
6503889	DW	A	559329	4803798	295.73	296.32	6
6503891	DW	A	560433	4804281	296.28	298.18	2
6503893	DW	A	561063	4803771	292.61	297.13	6
6503896	DW	A	561953	4803631	297.19	294.79	8
6503904	DW	A	562443	4803831	295.66	294.42	9
6503907	DW	A	562493	4803841	301.20	294.61	3
6503908	DW	A	562513	4803771	298.40	293.97	8
6503909	DW	A	562493	4803941	299.62	295.03	3
6503910		A	562913	4803904	300.23	292.69	6
6503914	DW	A	564403	4804221	296.29	290.43	9
6503915	DW	A	564498	4804291	293.72	290.89	8
6503916	DW	A	562333	4799481	263.66	268.97	10
6503917	DW	A	562413	4799571	265.48	269.66	10
6503918	DW	A	562453	4799561	264.27	269.21	10
6503919	DW	A	563933	4800601	267.32	265.16	10
6503921	DW	A	562553	4803641	289.57	293.30	9
6503944	DW	A	564253	4799921	260.00	263.01	10
6504020	DW	A	561293	4798291	266.09	271.06	9
6504022	DW	A	561333	4798376	271.88	272.00	2
6504027	DW	A	564908	4799571	260.44	262.02	10
6504030	DW	A	563333	4800701	262.41	269.64	10
6504031	DW	A	563748	4800601	265.79	265.56	10
6504032	DW	A	564633	4800441	262.74	264.12	10
6504033	DW	A	560193	4803041	294.10	292.59	9
6504034	DW	A	562473	4803721	298.70	294.20	3
6504035	DW	A	563853	4802001	277.98	271.27	7
6504036		A	564573	4803161	274.02	277.22	10
6504037	DW	A	561293	4804501	295.16	299.39	8
6504038	DW	A	562493	4803901	295.05	294.70	9
6504106	DW	A	558873	4795021	268.49	263.85	3
6504115	DW	A	563063	4801671	276.31	281.07	8
6504125	DW	A	557930	4800849	280.11	279.43	9
6504147	DW -COUROUX	A	552153	4810111	300.84	307.50	10
6504172	DW	A	561991	4803577	292.92	294.57	9
6504202	DW	A	562058	4803535	293.53	294.33	9
6504232	DW	A	554552	4793567	306.33	278.42	6
6504240	DW	A	560213	4794921	265.18	253.99	5
6504245	DW	A	550113	4795681	313.69	312.99	6
6504272	DW	A	562693	4804761	301.75	298.84	9

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6504336	DW	A	564233	4804221	297.49	290.52	9
6504344	DW	A	563153	4801271	272.43	276.98	10
6504345	DW	A	552785	4796119	308.46	311.96	6
6504350	DW	A	547967	4807000	280.27	286.07	7
6504360	DW	A	557196	4794389	263.37	264.85	9
1000668	DW-STAGER	A	553915	4809553	299.35	299.17	10
6504383	DW	A	554113	4811721	297.79	305.26	10
6504414	AVE-32	A	557473	4803461	285.60	286.22	7
6504417	DW	A	553512	4793353	293.33	285.38	6
6504428	DW	A	561633	4804601	300.16	299.49	9
6504464	DW	A	563243	4800901	267.52	272.75	10
6504475	DW	A	553673	4809241	301.55	299.93	10
6504498	AVE-18	A	557983	4803641	292.66	290.12	6
6504499	DW	A	551953	4811491	313.04	312.27	6
6504535	DW	A	549653	4805691	279.50	276.45	9
6504548	DW	A	559913	4799341	285.90	281.67	9
6504560	DW	A	552953	4807991	282.86	294.33	9
6504561	DW	A	552973	4808041	300.54	295.45	6
6504570	DW	A	560732	4804369	294.14	298.65	9
6504571	DW	A	561583	4803547	295.05	295.47	9
6504572	DW	A	561887	4803612	295.84	294.88	8
6504573	DW	A	563212	4804922	298.71	298.85	6
6504581	DW	A	549293	4811421	299.32	304.87	7
6504653	DW	A	551733	4809861	308.46	309.12	4
6504658	DW	A	550913	4806721	283.54	284.21	8
6504717	DW	A	560693	4798901	279.51	279.54	3
6504724		A	553123	4814221	326.16	319.08	6
6504753	DW	A	556333	4795461	261.53	264.47	8
1000688	DW28-96	A	549496	4810491	289.67	303.57	7
6504822	DW-BRITAIN	A	562140	4804561	295.66	299.18	6
6504850		A	556153	4806501	294.13	292.41	12
6504854	DW	A	559913	4799141	278.89	280.83	8
6504859	DW	A	558573	4803881	289.57	291.99	9
6504897		A	549593	4800581	307.85	313.09	6
6504942	TW1-79	A	555913	4797161	265.11	266.85	10
6504958	DW	A	550993	4806721	286.97	283.94	8
6504964	DW	A	550573	4812101	311.51	314.51	4
6504971	DW	A	553193	4814241	324.62	317.92	9
6504990	DW	A	558093	4803661	286.04	289.13	9
6504991	DW	A	551953	4806541	264.69	278.68	10
6504995	TW15-79	A	555963	4812421	295.64	302.97	13
6505012	DW	A	559833	4795561	275.07	263.35	3
6505015	DW	A	560693	4798381	267.62	277.04	8
6505016	DW	A	560153	4795481	270.05	259.50	8
6505017	DW	A	559673	4797961	288.04	277.67	3

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6505019	DW	A	560133	4795521	270.66	259.80	5
6505020	DW	A	550353	4801721	301.09	302.69	6
6505022	DW	A	559933	4795401	266.70	260.20	10
6505024	DW	A	553473	4811901	302.64	308.06	10
6505030	DW-WITMER	A	557205	4807071	304.94	295.56	9
6505040	DW	A	559113	4798761	286.52	279.95	4
6505041	DW	A	559093	4798821	286.52	280.33	4
6505053	DW	A	559209	4803781	287.75	294.54	9
6505059	DW-BAKER	A	548425	4809519	289.87	295.41	6
6505060	DW	A	553213	4816201	329.18	318.39	9
6505063		A	551753	4810261	310.29	308.57	8
6505074	DW	A	548213	4805121	275.85	280.75	7
6505078	DW	A	549893	4811541	307.85	309.43	6
6505083	DW	A	555733	4796561	252.38	264.30	10
6505102		A	548013	4798661	306.19	316.47	5
6505122	DW	A	559653	4795721	270.66	264.49	10
6505127	DW	A	551813	4793261	291.16	290.62	6
6505128	DW	A	553273	4793181	293.13	282.81	6
6505131	DW	A	555353	4794561	266.09	269.24	6
6505133	DW	A	555653	4798061	265.18	262.51	9
6505136	DW	A	551693	4798001	308.90	320.51	5
6505138	DW	A	552213	4807141	276.33	287.53	9
1000724	DW-MOYER	A	546747	4794696	295.79	300.61	6
6505178	DW-OROSZ	A	564513	4800554	264.87	264.50	10
6505182	DW-PULINSKI	A	564897	4799796	261.83	262.50	10
6505183	DW-LASKOVAR	A	560125	4803016	293.52	292.20	8
6505185	DW-MAGNUSSEN	A	548415	4808017	286.51	290.38	7
6505195	DW	A	563133	4801401	273.29	278.63	9
1000728	DW57-96	A	556308	4795449	259.11	264.18	9
6505212	DW-BEN-TEL	A	551822	4806911	280.72	286.40	9
6505213	DW-BENTEL	A	553512	4808948	289.10	300.82	9
6505214	DW-PIONEER SPORTSMAN	A	548271	4805144	274.46	280.53	8
6505223	DW	A	555353	4796481	258.93	266.59	9
6505224	DW	A	555353	4796561	252.99	266.51	7
6505225	DW-FILIPOVIC	A	555486	4796111	258.17	265.94	9
6505226	DW-ELLSMERE	A	558808	4803815	284.69	292.70	9
6505228	DW-JOHNSON	A	555708	4797658	260.91	263.16	10
1000736	DW71-96	A	560234	4802994	292.09	292.49	9
6505360	DW-HIPEL	A	552271	4811183	312.73	310.26	3
6505366	DW-MUELLER	A	548005	4807751	282.43	287.38	9
6505374	DW-MEDEIROS	A	559994	4799396	287.13	282.67	4
6505387	DW-SPORTSWORLD	A	548999	4806419	283.16	283.11	9
6505392	DW-CORDEIRO	A	551728	4806861	281.73	285.94	9

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6505394	DW-LEWIS	A	559392	4802573	300.52	288.84	8
6505399	DW-SMITH	A	553904	4808301	283.43	290.38	10
6505414	DW-MCLAUGHLIN	A	552048	4807037	281.03	286.95	9
6505421	DW-HALL	A	562405	4799508	263.04	268.88	10
6505429	DW-TOEBES	A	562444	4799515	266.71	268.70	10
6505457	DW-MORGENWEG	A	559939	4801277	288.04	287.23	5
6505460	DW-DONKERS	A	560782	4794964	255.35	251.00	4
6505464	DW-BARKER	A	555621	4797353	261.22	262.59	10
6505467	DW-DYKSTRA	A	560943	4798248	271.90	273.48	9
6505468	DW-INCITTI	A	559821	4799429	292.31	282.02	7
6505473	DW-PRONGE PUMP	A	548984	4806475	284.38	283.73	9
6505474	DW-SOOTHREN	A	548525	4805923	278.28	281.27	7
6505479	DW-GRESBRECHT	A	552238	4815856	321.27	318.60	8
6505495	DW-CEKAN	A	563855	4799933	265.48	263.30	10
6505507	DW-CITY OF CAMBRIDGE	A	551974	4806730	275.44	283.91	10
6505542	DW-PISCIVNERI	A	555141	4803348	284.38	282.22	9
1000780	DW-COLLINS	A	559991	4801224	285.51	287.24	5
6505564	DW-UNIVERSITY OF GUE	A	552007	4803379	268.84	281.75	10
6505567	DW-BAKER	A	556553	4809801	291.39	294.71	9
6505569	DW-HESSLER	A	560237	4794877	263.77	253.48	6
6505570	DW-INKSETTER	A	563603	4802113	276.15	273.60	2
6505571	DW-RHOEKSTRA	A	558372	4799902	284.99	280.93	9
6505572	DW-LAMONTAGNE	A	559688	4801296	288.04	286.88	3
6505575	DW-WEIGL	A	555273	4796122	264.57	271.69	6
6505576	DW-EDWORTHY	A	550926	4797529	308.76	320.39	6
6505577	DW-CAMBRIDGE SHRINE	A	557949	4797929	277.98	276.87	9
6505578	DW-LAMONTAGNE	A	559664	4801288	288.34	286.81	3
6505579	DW-RHOEKSTRA	A	558400	4799882	285.29	281.38	4
6505580	DW-SHINGLER	A	555041	4798421	263.35	259.59	9
6505581	DW-CITY AUTO WRECKER	A	558371	4801339	282.25	282.64	3
6505583	DW-CAMBRIDGE AUTO WR	A	560015	4800825	289.23	286.59	7
6505661	DW-GIGNAC	A	560722	4798976	277.37	279.75	8
6505663	DW-LANKA & SOLKIE	A	559858	4801184	284.38	286.97	8
6505667	DW-FREY	A	552956	4808430	283.77	299.18	10
6505668	DW-HILBORN	A	552454	4813722	304.19	316.58	9
6505669	DW-OTTMAN	A	552140	4811841	311.81	313.19	8
6505681	AVE-8	A	558755	4803846	290.96	292.61	9
6505682	DW-TUCKER	A	551937	4810462	301.14	308.47	10
6505690	DW-LOEWEN	A	553017	4808136	294.44	295.34	10
6505705	DW-EKKEBUS	A	560821	4798906	274.75	279.09	9

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6505706	DW-CLARKSON	A	564009	4800641	265.18	265.15	10
1000792	DW-DASILVA	A	559984	4803036	294.02	291.90	9
6505718	DW-SMITH JAMES CONST	A	551597	4806843	279.91	285.78	9
1000793	DW-FIORETTI	A	564201	4804058	294.83	289.34	9
6505724	DW-JESUS	A	555719	4797544	263.35	263.50	9
6505725	DW-TERESI	A	562677	4803985	298.71	294.52	3
6505726	DW-MACKIE	A	559903	4802982	295.97	291.32	8
6505727	DW-NICKLIN	A	555709	4797616	261.83	263.21	9
6505728	DW-ROPOTRYN	A	559859	4802984	296.57	291.26	8
6505729	DW-RUNIONS	A	562306	4803763	299.93	294.96	3
6505730	DW-MOREIRA	A	560053	4803043	292.31	292.61	6
1000794	DW-DAVIES	A	560472	4796455	274.07	264.38	9
6505733	DW-HERBERT	A	561276	4798445	266.70	273.11	10
6505735	DW-STRYKER	A	561216	4799210	266.50	279.53	9
6505736	DW-CAPORORTO	A	555728	4797261	264.57	267.92	6
6505755	DW-SANDERS	A	555021	4798290	262.74	259.90	8
6505756	DW-BEAVER	A	552102	4810546	300.23	308.59	10
6505761	DW-KRUCKEBERG	A	562637	4803908	298.41	294.32	3
6505762	DW-RUNIONS	A	562179	4803825	299.32	295.61	3
6505764	DW-SHAVER POULTRY	A	560818	4800880	290.79	287.86	8
6505766	DW-ROSS	A	562606	4803799	297.80	293.94	3
6505767	DW-MUNCH	A	561174	4799281	272.50	280.32	9
1000798	DW-CHRISVIEW HOMES	A	562432	4803657	299.58	294.03	3
6505768	DW-BENDER HOMES	A	562575	4804006	297.79	295.02	3
6505779	DW-BROOKFIELD GOLF	A	554297	4812426	307.54	308.47	10
1000799	OW-AB BRADLEY	A	557018	4801061	279.58	275.77	3
6505780	DW-REGIONAL READY MI	A	560613	4801786	289.87	289.05	9
1000800	DW-HESSLER	A	560246	4794878	264.98	253.45	5
6505802	DW-BROWN	A	558765	4799337	283.77	280.79	9
6505805	DW-REIDCO	A	558634	4803868	291.20	292.10	8
6505808	OW2-86	A	554983	4802729	264.34	273.37	9
6505810	OW1-86	A	555071	4802645	264.45	272.96	9
6505811	OW4-86	A	554930	4802750	264.44	272.80	9
6505812	TW16-86	A	552645	4796504	308.77	314.48	6
1000801	DW-BROUWER	A	559920	4795240	270.22	259.79	5
6505822	DW-GRAMBLICKA	A	549473	4803508	285.30	280.23	6
6505860	DW-PRONG	A	553566	4814715	316.08	320.55	9
6505861	DW-KOOLE	A	561149	4799262	271.88	280.20	9
6505862	DW-BEEMER	A	560281	4794803	264.57	252.47	8
6505863	TW?-86	A	554943	4803186	274.97	280.15	9

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6505879	DW-CREZEL	A	562229	4800100	267.31	278.04	9
6505880	DW-PINDER	A	547083	4797320	283.77	309.46	7
6505895	DW-SANSFILIPPO	A	555032	4798219	264.56	260.06	8
6505896	DW-KOELHER	A	555019	4798293	270.05	260.09	7
6505903	DW-MILLAR	A	563872	4800604	265.79	265.25	10
6505923	DW-MATTHEWS	A	551663	4812218	312.77	314.07	5
6505931	DW-HACKNEY	A	555319	4796597	262.13	267.57	6
6505933	DW-HAASE	A	548469	4810016	287.13	294.26	8
6505942	AVE-27	A	557662	4803589	286.70	287.33	8
6505943	DW-PURDY	A	557022	4810721	295.97	294.71	9
6505944	DW-DETTWEILER	A	552961	4808328	295.66	298.58	8
6505945	DW-BANNON	A	553595	4809058	294.75	300.52	9
6505946	DW-ISLEV	A	553874	4808405	282.86	292.99	10
6505949	DW-BONKOFF	A	549638	4805725	281.94	276.76	7
6505950	DW-SPORTSWORLD	A	549090	4806150	283.47	280.58	8
6505953	DW-BROUWER	A	561274	4798481	266.40	273.63	10
6505954	DW-WOOD	A	554929	4797381	269.14	262.56	7
6505955	DW-ELLIS	A	562683	4803893	297.48	294.08	2
6505956	DW-LEADER	A	560929	4799145	277.06	279.90	8
6505957	DW-NICHOLAS	A	554952	4797339	268.53	263.54	6
6505958	DW-PEREIRA	A	553549	4798548	306.49	300.01	6
6505961	DW-LEDUC	A	560861	4799109	275.85	279.94	9
6505965	DW-OSTOPOVICH	A	562772	4804055	296.88	294.51	3
6505966	DW-BENDER HOMES	A	562722	4804014	297.49	294.50	3
6505967	DW-RUNIONS CONSTRUCT	A	562155	4803806	298.10	295.59	3
6505968	DW-FRACEL CUSTOM	A	562549	4803922	297.79	294.73	3
6505969	DW-DRY	A	562482	4803965	300.14	295.18	3
6505973	DW-FRACEL CUSTOM	A	562610	4803917	300.85	294.52	2
6505974	DW-MEDEIROS	A	561306	4799230	267.61	279.49	9
1000816	DW-LANG	A	552328	4807990	292.98	296.68	7
6505991	DW-HUBERCHECH	A	549098	4806188	282.25	281.03	6
6505998	DW-CULLTON	A	560053	4795585	264.88	260.48	10
6506002	DW-JONAS	A	563674	4801749	275.85	272.71	10
6506003	DW-ZANDER	A	561081	4799293	277.68	280.67	8
1000817	DW-WEINHARDT	A	551780	4806832	278.66	284.92	9
6506011	DW-BELAND	A	554881	4797434	267.01	262.62	7
6506012	DW-MACHADO	A	558976	4799430	286.21	281.48	9
6506017	DW-DENOUDEN	A	551164	4797655	307.54	319.48	6
6506025	DW-ADAMCHYCK	A	555637	4797720	259.39	262.10	10
6506026	DW-NEWMAN	A	555639	4797684	259.39	262.15	10
6506044	DW-FITZPATRICK	A	554886	4797409	271.88	262.46	8

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6506050	DW-BOISVERT	A	548376	4793672	297.18	297.95	6
6506051	DW-HAMMER	A	564083	4800662	265.18	265.13	10
6506053	DW-CIRCLE SQUARE	A	546576	4806229	281.95	280.48	7
6506079	DW-SODTRE	A	553967	4814082	314.56	317.83	9
6506086	DW-TIBBITTS	A	556754	4810327	288.65	300.28	12
6506088	DW-NEVEN CASEY	A	562667	4803813	299.93	293.54	9
6506102	DW-MARTIC	A	551478	4811129	309.37	311.65	9
6506103	DW-WATERLOO CB ED	A	547991	4798688	307.85	316.32	5
6506112	DW-HELM	A	549661	4803293	285.91	282.03	6
6506113	DW-REIS	A	549481	4803313	286.21	282.88	6
6506130	DW-SGS PLASTIC	A	555814	4809512	283.46	298.18	10
6506135	DW-VANDERPOOL	A	551695	4806846	290.31	285.82	9
6506136	DW-ROBERTSON	A	552450	4807433	291.70	289.86	7
6506137	DW-GRIER	A	553895	4808312	282.55	290.92	10
6506138	DW-HARNACK	A	553906	4808266	281.34	290.68	10
6506145	TW2-87	A	557173	4803261	285.92	284.39	9
6506149	DW-TULLIS ESTATE	A	552362	4799642	308.76	313.84	6
6506152	DW-BARTHOLOMEW	A	553498	4798140	306.64	298.40	8
6506154	DW-WING	A	555565	4795753	260.91	267.77	7
6506159	DW-PARRISH	A	549673	4803436	286.82	279.82	6
1000852	DW-SILVA	A	553182	4814381	312.76	318.51	9
6506178	DW-FALKINER	A	558135	4800901	281.34	281.17	6
6506179	DW-GLOADE	A	553657	4809086	293.83	299.74	10
6506181	DW-DOHERTY	A	554013	4808051	280.42	287.46	10
6506187	DW-RUMPH	A	560743	4798929	275.85	279.44	9
6506189	DW-FUNG	A	562202	4803850	299.01	295.66	3
6506229	AVE-4	A	558844	4803825	296.70	296.37	3
6506231	DW-LLOYD	A	553863	4807943	280.72	286.42	10
6506232	DW-COOK	A	553887	4808364	284.27	292.37	10
6506233	DW-SMITH	A	558647	4799831	286.21	281.93	4
6506235	DW-PERRIN	A	554272	4793849	284.38	278.66	9
6506237	DW-MORGAN	A	555357	4796394	257.56	266.88	8
6506240	DW-HOEKSTRA CONST	A	554610	4793666	293.83	275.14	6
6506244	DW-RUNIONS CONSTRUCT	A	562240	4803884	299.32	295.71	3
6506245	DW-TINKER	A	554901	4797482	270.97	263.55	6
6506248	DW-SPELIC	A	562199	4799533	264.27	270.97	9
6506249	DW-WOOD	A	561070	4799331	275.23	281.06	9
6506250	DW-?ASTAG	A	555185	4796318	258.28	271.29	10
6506251	DW-RENDLE	A	561253	4799230	268.54	279.66	9
6506252	DW-MILLER	A	560045	4795425	269.45	262.02	3
6506265	DW-NEWMMASTER	A	548798	4793748	298.70	299.80	6

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6506279	DW-CRESSMAN	A	549613	4803378	285.91	281.02	5
6506280	DW-COLES	A	556966	4810616	293.83	300.53	12
6506281	DW-SOLDERA	A	562284	4803819	297.48	294.96	9
6506293	DW-BEECRAFT	A	553655	4816296	321.87	319.81	10
6506295	DW-HARTNELL	A	549576	4803250	286.21	283.23	6
6506296	DW-POLADIAN	A	549673	4803310	284.99	281.88	6
6506297	DW-NATURELLA FARMS	A	559529	4796984	277.97	272.91	8
6506298	DW-JOANISSE	A	554536	4793585	294.44	273.03	6
6506299	DW-SHELL	A	554682	4793684	294.75	272.33	6
6506300	DW-DEIGNAN	A	561058	4799367	271.88	281.43	9
6506301	DW-VLASOV	A	555872	4793913	258.47	260.37	9
6506302	DW-VLASOV	A	555875	4793879	258.47	260.61	7
6506309	DW-BENNETT	A	550880	4803394	278.86	285.32	9
6506312	DW-MCLEAN	A	561397	4801440	289.50	289.33	9
6506339	DW-ANEJA	A	551241	4806984	292.01	287.66	8
6506341	DW-HIGGINS	A	557051	4810641	296.26	300.54	13
6506344	DW-TONCIC	A	551345	4806870	287.12	285.92	8
6506346	DW-BRADBURRY	A	563307	4800594	263.04	268.51	10
6506347	DW-JOHNSON	A	552346	4794456	304.20	302.78	6
6506348	DW-PITTENS	A	561794	4801801	289.56	289.61	9
6506357	DW-COLES	A	556997	4810619	285.91	296.22	10
6506358	DW-BLAIR	A	553633	4809745	301.45	301.00	9
6506359	DW-BARTELS	A	553693	4809356	294.75	300.22	9
6506362	DW-SOOKRAM	A	551135	4806939	289.26	287.25	7
6506363	DW-HUNTER	A	557025	4810632	295.35	297.63	10
6506389	OW?-88	A	557128	4801043	281.64	276.05	3
6506390	DW-MACLEOD	A	554801	4798533	284.69	259.92	9
6506399	DW-PATTERSON	A	552973	4808607	297.80	300.30	10
6506400	DW-DUPOUIS	A	551964	4807015	282.56	287.08	9
6506401	DW-FILIPOVIC	A	551634	4806842	282.26	285.82	9
6506402	DW-SOLOMON	A	557117	4810682	292.61	297.96	10
1000874	DW-HUSSON	A	557734	4798085	277.09	276.78	4
6506417	DW-MUNSON	A	561639	4799414	265.79	278.80	9
6506420	DW-KLEINSTEUBER	A	562374	4803983	297.49	295.69	3
6506426	DW-ANEJA	A	551282	4806989	286.65	287.70	9
6506428	DW-SOLDERA	A	562292	4803817	299.01	295.24	3
6506435	DW-SCHWARR	A	551780	4812027	287.13	313.60	10
6506441	DW-BRANCHTON CAMP	A	558377	4795051	262.89	264.37	10
6506460	DW-LARSON	A	555484	4796220	254.74	265.35	9
6506462	DW-KRUCKEBERG	A	552988	4808546	300.84	300.06	10
6506467	DW-PATTERSON	A	551872	4801335	309.07	305.32	6
6506490	DW-WHETMAN	A	553513	4813006	309.07	311.13	6
6506492	DW-FINK	A	556251	4812717	307.55	302.03	9

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6506493	DW-DUNSTALL	A	555001	4797809	266.40	261.06	6
6506502	DW-WELLS	A	564644	4800392	259.99	263.99	10
6506509	TW1-91	A	555445	4798707	258.47	259.14	8
6506513	DW-HESSLER	A	560088	4794828	266.40	253.72	8
6506515	DW-LOTIMER	A	551894	4805426	271.62	274.51	9
6506519	PW3-89	A	549781	4802863	289.89	288.01	6
6506520	DW-DRAWWEHN	A	553755	4809425	284.99	299.67	10
6506538	DW-CLEMENT	A	563834	4800625	265.18	265.36	10
6506544	DW-CEKAN	A	563910	4799953	264.87	263.30	10
6506559	DW-MONGEON	A	556908	4810592	290.78	300.48	11
6506560	DW-CITY OF KITCHENER	A	549560	4805756	290.48	276.95	9
6506586	DW-DEFREITAS	A	548925	4803515	282.25	284.55	6
6506595	DW-MCCREADY	A	551196	4806970	289.25	287.53	8
6506602	DW-MACMILLAN	A	555518	4795961	259.39	266.64	9
6506603	DW-STIELER	A	554590	4793635	292.01	272.17	6
6506604	DW-AZEVEDO	A	559869	4795393	270.67	261.00	9
6506607	DW-MAPLE MANOR EST	A	557551	4795978	267.53	270.32	3
6506609	DW-GOODMAN	A	564429	4800637	264.88	264.84	10
6506610	DW-WHITTIER	A	564270	4800156	264.26	263.54	10
6506611	DW-TIMELESS HOMES	A	551151	4806957	288.04	287.45	7
6506614	DW-COTE	A	555424	4796169	258.45	267.11	8
6506631	DW-B&M CONSTRUCTION	A	553624	4809068	292.92	300.33	8
6506636	DW-PETRO CANADA	A	548566	4806553	276.76	284.94	8
1000905	DW-ELLIOT	A	553009	4796349	307.40	311.63	6
6506652	DW-CUNNING	A	555803	4794127	256.34	259.70	6
1000908	DW-ED WRIGHT CONSTRU	A	551345	4798621	307.33	320.63	5
6506679	DW-RUNIONS	A	560220	4803026	290.79	292.61	9
6506697	DW-RUNIONS CONSTRUCT	A	560433	4803096	291.70	293.55	9
6506698	DW-DEGRAAF	A	564714	4800126	261.82	263.34	10
6506699	DW-AREND	A	564881	4799731	261.83	262.37	10
6506703	DW-KOWALESKI	A	555193	4794156	266.70	262.07	9
6506706	DW-ENGELHARDT	A	555001	4797739	265.29	260.63	8
6506707	DW-GAUCH	A	555433	4794899	266.70	271.64	10
6506710	DW-LEE WAYNE CONST	A	555933	4793943	252.99	261.19	9
6506713	DW-ROCHA	A	555154	4796298	271.15	272.03	9
6506714	DW-OLIVER FOWLIE SUB	A	551501	4798720	309.68	320.32	5
6506719	DW-MARKLE	A	553662	4809542	295.66	300.56	10
6506723	DW-PANDEIRADA	A	562363	4803789	296.58	294.89	3

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6506725	DW-HERNER	A	553149	4815476	315.16	317.86	6
6506726	DW-SCHWEBRO	A	552206	4812590	314.25	315.77	9
6506747	DW-WALDEN	A	553997	4811433	300.53	304.59	10
6506748	PS2	A	557014	4808775	299.02	299.31	11
6506749	PS1	A	557013	4808780	299.32	298.54	9
6506771	DW-ULTRAMAR	A	558668	4799780	277.84	281.66	4
6506774	DW-OLDFIELD	A	555635	4799249	264.26	260.42	9
6506775	DW-PINCOTT	A	547595	4805494	279.20	281.71	7
6506776	DW-EASTON	A	547511	4805435	275.24	281.54	7
6506840	DW-LE BRUN	A	555244	4794280	278.90	263.45	8
6506846	DW-GALT COUNTRY CLUB	A	553775	4803290	272.62	267.89	9
6506847	DW-FORD	A	560085	4795250	269.48	258.72	8
6506848	DW-LUNGHI	A	562381	4803919	301.94	295.37	3
6506892	DW-COUTO	A	561153	4799195	268.51	279.58	9
6506893	DW-FRANK ROCHA CONST	A	562407	4804002	299.68	295.65	3
6506905	DW-MEYER	A	552023	4806994	284.63	286.57	9
6506912	DW-MUIR	A	559712	4795903	264.88	265.15	10
6506913	DW-CASATTA	A	550537	4809754	303.29	305.70	7
6506915	DW-GREEN	A	559999	4799711	287.12	283.41	9
6506922	DW-GOODHAND	A	562179	4798591	258.47	264.82	10
6506941	DW-BAUER	A	548043	4807779	281.95	287.57	9
6506945	PS3?	A	557123	4808632	302.01	299.36	13
6506966	DW-STALKER	A	564288	4801622	267.31	268.14	10
6506969	DW-KINGSWOOD	A	555122	4795195	276.45	277.78	6
6506972	DW-FURTNEY	A	554971	4793914	287.63	264.81	6
6507016	DW-IGEL	A	562363	4803872	299.62	295.24	3
1000957	DW-E.D. WRIGHT CO-97	A	551356	4798670	305.89	320.56	5
6507019	DW-SCHIEDEL CONS	A	547260	4799503	302.97	306.05	5
6507025	DW-SHIRY	A	549336	4810313	288.65	302.28	6
1000958	DW-KOHMAN	A	564272	4800659	264.17	264.97	10
6507030	DW-TYSDALE	A	564737	4800234	260.91	263.58	10
6507031	DW-WATERLOO CONCRETE	A	547938	4800119	302.98	310.57	6
6507047	DW-VANDEL CONTRACTIN	A	554024	4809760	302.97	299.89	10
6507048	DW-VANDEL CONTRACTIN	A	554001	4809757	302.36	299.87	10
6507049	DW-GONDER	A	552478	4807399	278.59	289.43	9
6507053	DW-MOORE	A	548508	4796876	302.98	318.23	5
6507081	DW-DUOBAITIS	A	550128	4808686	287.96	299.96	7
6507102	DW-PARAGON	A	563312	4800581	257.38	268.29	10
6507146	DW-BADDER	A	555008	4797461	265.79	260.79	9
6507154	DW-ROSS	A	556761	4810173	293.83	295.19	9

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6507155	DW-BUSHEN	A	556810	4810340	292.00	294.57	10
6507160	DW-MORTON	A	564690	4800326	255.73	263.81	10
6507168	CL43-91	A	558716	4802050	290.97	284.36	9
6507169	DW-BARTOLO	A	548365	4806060	279.73	282.69	7
6507192	DW-SPRATT TM LTD	A	561291	4795919	255.12	250.38	10
6507193	DW-GINGRICH	A	559051	4799479	285.90	281.68	5
6507198	DW-CREZEL	A	562317	4799530	260.61	269.84	10
6507214	DW-NATURALL FARMS	A	560505	4797751	276.76	273.46	9
6507245	DW-GRIFFIN GROUNDWAT	A	553933	4809191	299.98	296.00	6
6507246	DW-GRIFFIN GROUNDWAT	A	553948	4809281	301.05	295.96	6
6507248	DW-GRIFFIN GROUNDWAT	A	554003	4809261	303.58	295.96	3
6507258	DW-WEIR	A	551592	4813576	312.73	317.58	6
6507267	DW-WEIGL	A	555307	4796185	261.22	269.00	9
6507294	DW-CABRAL	A	559915	4801164	288.65	287.03	5
6507296	DW-CAMBRIDGE LANDSCA	A	560992	4799245	272.80	280.47	9
6507320	DW-ORIENTAL SPORTSCL	A	560901	4799475	269.84	282.71	9
6507328	CL-PW2-92	A	558797	4801787	287.74	284.49	3
6507329	DW-PAYNE	A	560138	4795167	270.06	259.74	4
6507335	DW-LAUGALYS	A	553440	4809033	296.88	300.45	10
6507419	DW-TAYLOR	A	555458	4793693	248.57	254.53	9
6507455	DW-JAMES	A	560041	4795598	271.72	260.79	4
6507456	DW-PFEIFFER	A	552175	4807266	276.86	288.98	9
6507457	DW-DOYLE	A	553730	4808523	290.25	295.34	13
1001017		A	550251	4795646	307.78	312.77	5
6507470	DW-KNUTSON	A	553967	4808021	269.30	291.79	12
6507471	DW	A	561333	4798421	265.87	272.65	9
6507472	DW	A	561333	4798501	260.26	273.57	10
6507496	DW-GOUTEM	A	553716	4808219	282.28	290.72	9
6507497	DW-MURCOR	A	560801	4798866	279.77	279.09	7
6507498	DW-SOUTHEND	A	560118	4795433	270.05	259.51	5
6507519	DW-DETTWEILER	A	552980	4808444	300.74	299.65	6
6507520	DW-DOBBIE	A	552965	4808404	298.99	299.18	9
6507532	DW-FACH	A	554664	4813231	313.94	312.78	6
6507533	DW-SITTLER	A	563596	4800625	267.16	266.13	10
6507534	DW-GALVAO	A	561318	4799265	271.07	279.83	9
1001030		A	556195	4795463	258.37	264.66	10
6507548	DW-DAMM	A	549262	4801876	301.39	298.19	6
6507553	DW-SOLOMON	A	548120	4806138	278.39	283.38	8
1001035		A	547028	4799723	302.04	303.31	5

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6507570	DW-AUTOMATIC TOOLING	A	551996	4807021	295.73	287.02	9
6507584	DW-HAFEMANN	A	552866	4814103	310.88	316.57	10
6507590	DW-YOANIDIS	A	553899	4808353	283.30	293.79	12
6507603	DW-BUTLER	A	552938	4806062	280.02	282.60	6
1001039		A	548509	4800206	303.17	313.76	5
6507862		A	561448	4799412	271.88	280.45	9
6507863		A	560102	4803069	292.92	292.46	9
6507865		A	562473	4803708	297.48	294.14	3
6507866		A	562129	4803742	297.18	295.35	3
6507867		A	562192	4803741	298.40	295.17	3
6507868		A	562248	4803811	300.23	295.34	3
6507869		A	562555	4804326	299.93	296.90	3
6702272		A	559852	4806897	297.79	302.64	7
6702305		A	559859	4808314	303.19	303.68	3
6703187		A	564704	4814103	324.00	316.18	11
6703262		A	564614	4811483	308.76	310.24	12
6703315		A	563114	4811203	315.63	316.76	7
6703347		A	561994	4813073	323.09	306.77	12
6703348		A	564454	4813643	328.58	313.19	12
6703354		A	556994	4814483	304.80	306.48	10
6703370		A	563134	4812083	316.99	307.36	12
6703371		A	555214	4817123	333.18	322.17	9
6703373		A	568374	4811563	314.25	312.45	4
6703385		A	565574	4811203	310.89	310.93	11
6703404		A	557364	4816573	329.57	317.34	9
6703426		A	564254	4811423	314.25	317.44	9
6703438		A	564874	4811673	315.17	310.98	12
6703451		A	556224	4817733	324.78	316.35	13
6703460		A	565524	4805583	300.54	298.19	9
6703467		A	557614	4815123	301.77	306.56	11
6703484		A	557414	4815163	305.24	308.40	11
6703492		A	559594	4808223	303.89	303.40	8
6703496		A	569944	4812433	309.99	319.55	9
6703535		A	567884	4810143	299.92	310.94	13
6703552		A	567794	4811613	306.64	308.74	11
6703590		A	560804	4812643	311.14	305.95	13
6703600		A	564314	4813613	326.14	312.48	12
6703601		A	559894	4807903	302.73	303.14	8
6703607		A	561394	4810963	309.98	311.49	9
6703614		A	570804	4810123	308.15	318.66	7
6703640		A	568574	4811553	306.63	311.63	13
6703648		A	565134	4812483	315.46	321.45	7
6703666		A	554224	4815923	313.48	320.02	10
6703671		A	564224	4814623	324.92	321.66	11

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6703673		A	560414	4813653	318.52	316.40	9
6703719		A	554254	4815923	314.55	318.28	12
6703721		A	560184	4807793	303.89	303.82	6
6703722		A	558764	4807773	302.36	301.23	9
6703732		A	563514	4813103	321.56	308.45	12
6703780		A	556264	4813803	303.67	306.85	10
6703784		A	558854	4807773	304.19	301.51	9
6703804		A	563834	4811293	320.04	317.44	7
6703819		A	558537	4806345	306.87	298.44	8
6703822		A	560664	4807173	300.23	304.02	7
6703823		A	565504	4811483	311.81	313.50	9
6703849		A	566374	4807563	305.10	306.71	9
6703852		A	567734	4811693	306.81	308.10	5
6703855		A	570534	4809973	305.71	318.02	10
6703856		A	564384	4811443	312.11	317.30	7
6703865		A	565314	4814173	322.42	327.87	10
6703867		A	564054	4813543	324.31	311.09	12
6703869		A	565474	4805543	299.92	297.88	9
6703894		A	559924	4808973	305.10	305.47	3
6703900		A	565184	4812413	312.42	312.70	12
6703909		A	565174	4812063	314.25	318.42	10
6703920		A	557689	4815148	304.24	306.94	12
6704020		A	557664	4815173	300.89	307.09	12
6704029		A	557114	4816648	326.44	319.00	9
6704031		A	562454	4810963	313.64	314.66	7
6704038		A	569614	4809823	305.41	316.70	6
6704040		A	558984	4812263	308.26	310.32	7
6704041		A	567724	4811683	306.63	308.27	8
6704042		A	569774	4809483	303.28	316.12	7
6704044		A	560732	4812793	310.90	315.14	10
6704046		A	570604	4810053	310.59	318.35	9
6704048		A	559904	4810423	306.32	307.91	6
6704049		A	560139	4806923	301.47	302.72	7
6704050		A	560089	4806898	303.51	302.72	7
1001077		A	547606	4803253	297.28	293.06	6
6704072		A	561134	4812783	317.02	316.19	9
6704098		A	562894	4811083	312.42	316.00	7
6704109		A	557944	4812003	301.75	301.40	10
6704137		A	557364	4811323	298.10	298.01	9
6704141		A	563854	4805173	293.74	296.30	9
6704142		A	566864	4805893	292.60	296.09	9
6704213		A	560289	4807073	292.61	302.85	10
6704217		A	561904	4809513	303.73	309.34	5
6704229		A	559714	4806073	304.80	301.20	9
6704249		A	564514	4814283	322.47	314.50	12

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6704286		A	555364	4815798	322.68	320.27	9
6704296		A	562334	4811013	313.94	314.38	9
6704302		A	558964	4814398	309.66	307.55	9
6704314		A	558439	4807723	311.20	300.82	6
6704325		A	567154	4812093	312.42	309.76	9
6704326		A	569014	4807323	290.30	293.06	10
6704331		A	565554	4811323	309.06	311.96	9
6704332		A	560439	4807923	299.31	304.43	8
6704342		A	558264	4816173	309.68	310.79	10
6704352		A	569574	4812903	317.60	313.23	13
6704356		A	561574	4804633	298.70	299.63	6
6704389		A	569634	4809783	307.24	316.64	6
6704390		A	563974	4813563	326.14	326.27	7
6704391		A	558939	4805923	303.98	300.66	5
6704393		A	557164	4811273	298.77	296.91	8
6704398		A	560164	4806973	294.51	302.73	8
6704401		A	569454	4812703	313.94	316.05	10
6704421		A	563154	4814143	306.33	307.09	13
6704479		A	558739	4807798	290.78	301.21	8
6704486		A	566574	4808383	298.40	307.11	6
6704487		A	558714	4807823	300.23	301.26	6
6704500		A	561604	4804643	292.61	299.62	7
6704511		A	563594	4813983	326.14	326.27	10
6704518		A	560694	4806213	302.21	302.72	5
6704520		A	559024	4811573	303.27	308.17	8
6704531		A	565164	4812333	316.99	320.10	10
6704550		A	558849	4807833	303.58	301.28	9
6704558		A	560225	4807814	304.50	304.26	3
6704559		A	557707	4815081	301.92	304.83	10
6704562		A	557329	4814830	299.77	306.52	10
6704568		A	570344	4808403	296.88	309.38	8
6704576		A	559257	4810938	306.93	306.83	6
1001102		A	547203	4801170	301.75	304.74	6
6704628		A	558177	4814661	296.57	299.01	10
6704629		A	560956	4812703	313.07	314.95	10
6704635		A	558953	4815561	306.33	304.53	11
6704636		A	558504	4815789	308.26	307.52	10
6704656		A	563554	4811188	323.70	317.21	7
6704657		A	559191	4814515	312.72	308.20	9
1001113		A	549456	4795052	307.25	311.04	6
6704673		A	567673	4813039	314.86	314.50	7
6704686		A	562751	4811041	315.73	315.59	7
6704687		A	565259	4811693	315.77	315.88	8
6704692		A	566810	4807606	300.22	307.30	10
1001114		A	548625	4800224	303.70	313.90	6

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6704693		A	568214	4809488	306.32	312.34	7
6704694		A	557337	4811265	298.71	297.74	8
6704695		A	557366	4811315	299.62	297.95	7
6704698		A	560158	4806955	299.93	303.50	6
6704719		A	568191	4809783	306.02	311.74	12
6704728		A	556273	4817794	326.18	322.81	10
6704741		A	557616	4811321	299.31	298.94	9
6704742		A	558259	4812099	306.63	304.40	9
1001122		A	550811	4797192	307.79	320.47	6
6704769		A	566493	4808085	296.29	307.14	10
6704785		A	557054	4816543	325.52	319.08	9
1001130		A	552440	4796195	307.57	314.91	5
6704826		A	568974	4807263	288.64	292.72	10
6704841		A	561336	4806508	303.31	303.36	5
6704843		A	566121	4813361	316.99	323.21	7
6704858		A	563180	4811781	313.39	319.43	9
6704959		A	561275	4806484	298.40	303.37	8
6704969		A	567832	4813260	297.18	314.78	13
6704990		A	560145	4810345	308.46	308.38	8
6705008		A	567593	4812085	307.85	309.23	10
6705023		A	562305	4804821	297.18	299.63	8
6705029		A	567317	4812307	309.86	312.87	12
6705048		A	564413	4811435	318.52	317.20	7
6705089		A	562683	4811141	317.37	315.72	9
6705091		A	565540	4811635	308.25	314.38	10
6705094		A	567155	4807759	301.75	307.36	10
6705097		A	568464	4808858	301.76	310.07	13
6705112		A	556254	4814403	315.16	310.88	9
6705113		A	563783	4805247	298.09	297.20	6
6705144		A	559514	4808073	302.37	302.91	9
6705162		A	557014	4816503	327.66	318.02	10
6705189		A	562512	4810948	309.37	314.78	9
6705193		A	566537	4808404	306.94	306.97	4
6705194		A	566417	4808339	308.76	306.66	6
6705218		A	562349	4810897	302.49	314.19	9
6705239		A	557614	4815223	301.75	307.35	12
6705284		A	564555	4814369	330.52	328.68	7
6705330		A	568095	4809837	304.80	312.22	10
6705335		A	558942	4811976	306.53	307.60	11
6705378		A	569116	4808402	296.57	307.52	10
6705383		A	559114	4811423	301.76	307.75	8
6705385		A	568509	4808913	311.91	311.79	7
6705392		A	557110	4811836	295.05	299.26	10
6705488		A	567634	4811560	302.36	311.76	12
6705510		A	570975	4810584	325.05	320.39	7

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6705521		A	558164	4807373	310.04	299.16	6
6705523		A	557397	4811887	280.95	298.99	10
6705550		A	563505	4813454	318.82	324.47	10
6705555		A	557274	4816803	325.53	318.72	9
6705566		A	570354	4812050	303.53	318.04	13
6705567		A	558755	4807743	300.84	300.92	9
6705611		A	557734	4815123	300.44	306.78	13
6705625		A	568929	4807315	277.97	294.37	11
6705682		A	557514	4815223	304.70	308.46	10
6705737		A	570823	4806883	286.51	287.68	10
6705773		A	569305	4811654	309.38	315.63	10
6705805		A	557394	4814923	300.54	306.76	10
6705826		A	559129	4810174	300.18	305.34	10
6705851		A	564084	4809205	303.28	304.81	6
6705870		A	567138	4812055	306.79	309.70	10
6705872		A	569137	4806966	287.43	288.67	10
6705876		A	565508	4812167	313.34	318.05	10
6705877		A	569575	4809782	306.33	316.53	7
6705884		A	560287	4810943	311.81	309.43	9
6705967		A	564764	4813647	323.70	314.19	12
6705970		A	566865	4807314	311.81	306.19	5
6705971		A	561623	4813258	325.83	320.23	10
6705988		A	560231	4806895	301.75	303.51	5
6706014		A	566726	4807791	307.24	307.83	5
6706027		A	558518	4806415	302.21	297.69	9
6706096		A	559989	4808071	300.44	303.56	9
6706097		A	558608	4808944	300.93	302.66	9
6706105		A	560593	4813731	287.43	305.86	13
6706106		A	556534	4813863	302.97	305.52	10
6706156		A	565814	4813543	324.00	317.39	11
6706160		A	558614	4808903	306.94	302.91	9
6706198		A	558694	4805803	295.65	298.72	9
6706220		A	567034	4806483	290.17	300.06	10
6706259		A	568234	4808223	301.75	308.89	7
6706261		A	564574	4805343	296.67	294.91	3
6706264		A	563254	4811703	309.98	316.80	11
6706296		A	570762	4806833	284.99	287.36	10
6706317		A	566434	4808123	303.58	306.97	6
6706318		A	567774	4808083	305.41	308.53	5
6706344		A	561374	4812843	317.20	317.65	10
6706346		A	566434	4808443	306.31	306.58	9
6706365		A	556254	4815843	317.30	314.33	10
6706431		A	558154	4808943	303.58	301.47	4
6706432		A	558654	4807683	304.80	300.54	8
6706444		A	568514	4810943	308.76	311.75	4

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6706483		A	561414	4810723	312.92	311.27	8
6706503		A	561134	4809763	311.20	309.11	3
6706533		A	564614	4814023	323.59	327.72	10
6706546		A	558434	4808923	302.52	300.64	14
6706556		A	560834	4812763	315.17	309.64	11
6706563		A	556174	4814423	313.33	309.84	10
6706570		A	562774	4811023	311.51	315.55	9
6706576		A	570154	4809283	299.32	313.95	13
6706615		A	558674	4807363	305.11	299.58	8
6706619		A	567434	4811683	305.18	311.89	12
6706620		A	566474	4812183	307.22	312.95	13
6706627		A	569414	4808363	297.48	306.35	10
6706694		A	570514	4812023	311.81	318.91	14
6706724		A	566514	4808743	303.53	306.69	8
6706771		A	567214	4812523	303.99	312.12	11
6706774		A	564194	4808483	300.53	300.33	10
6706807		A	566674	4808343	308.76	307.41	6
6706808	TW 14	A	557734	4811403	302.37	301.34	12
6706821		A	557714	4816623	324.00	315.35	10
6706874		A	569474	4809943	305.41	316.56	6
6706875		A	570534	4808823	305.56	310.55	13
6706876		A	561654	4804643	289.56	299.58	9
6706881		A	565094	4809303	300.23	304.11	9
6706882		A	558814	4812243	310.90	309.45	9
6706885		A	557434	4812683	296.26	297.69	10
6706916		A	568814	4810343	311.15	314.38	10
6706947		A	561534	4814823	321.52	320.76	5
6706953		A	565014	4811643	309.68	316.41	9
6706982		A	564994	4813523	316.74	314.69	12
6707021		A	560134	4807843	302.62	304.13	3
6707041		A	559934	4805043	298.86	297.24	12
6707043		A	566614	4805963	293.56	297.83	10
6707091		A	570294	4812123	312.54	321.40	8
6707092		A	558954	4812043	306.09	309.52	6
6707200		A	566294	4807683	297.26	306.67	10
6707201		A	567574	4807883	304.13	307.75	5
6707202		A	567294	4807923	314.04	307.81	9
6707204		A	557454	4812683	297.71	297.79	10
6707208		A	569534	4808463	303.16	308.84	6
6707222		A	560014	4806243	302.06	302.42	3
6707241		A	569494	4812183	318.49	316.94	10
6707265		A	558634	4807723	300.23	300.00	9
6707266		A	558854	4807783	304.29	301.23	9
6707271		A	569314	4811663	309.38	315.65	10
6707273		A	568954	4812523	309.18	308.61	14

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6707290		A	569194	4812803	310.28	307.29	13
6707297		A	563594	4814723	324.91	327.86	9
6707298		A	570714	4812063	319.69	320.01	11
6707317	TW11/16-80	A	565094	4808763	303.58	302.10	11
6707387		A	566991	4806944	301.33	303.86	6
6707441		A	564114	4811423	309.37	317.66	8
6707454		A	570494	4806883	286.51	287.40	10
6707457		A	567794	4809923	305.41	310.88	10
6707516		A	557688	4815319	300.94	307.31	13
6707576		A	570434	4811983	320.65	321.82	7
6707580		A	558894	4808743	302.20	303.17	9
6707581		A	568374	4813103	312.11	313.15	13
6707583		A	564994	4805443	293.22	296.22	9
6707585		A	569874	4812423	321.78	319.04	10
6707714		A	556454	4816963	320.04	314.58	13
6707737		A	565454	4813843	318.33	326.81	10
6707738		A	560534	4807583	299.62	304.45	4
6707760		A	560054	4808423	303.51	304.43	6
6707762		A	568554	4810923	306.62	312.14	11
6707797		A	568974	4812563	307.41	308.18	13
6707851		A	557174	4810723	299.88	296.56	9
6707877		A	556192	4813625	311.33	306.46	10
6707912		A	557414	4811263	299.12	298.08	9
6707914		A	558714	4810423	298.26	303.93	9
6707935		A	568334	4812943	306.63	312.63	13
6707994		A	559794	4806003	300.22	301.28	7
6708041		A	556214	4815763	317.83	317.42	9
6708045		A	557594	4812143	298.70	298.69	5
6708076		A	568354	4812983	306.63	312.72	13
6708094		A	568594	4808243	302.76	308.38	6
6708095		A	562014	4810803	314.22	312.65	10
6708127		A	566934	4812403	309.99	310.96	5
6708178		A	557790	4815572	307.55	308.84	10
6708185		A	558685	4807694	305.27	300.88	6
6708206		A	559396	4812321	315.69	311.61	9
6708207		A	557202	4813261	291.05	294.15	10
6708224		A	562313	4810583	313.90	313.09	7
6708236		A	560945	4811262	313.08	311.69	9
6708267		A	560979	4811206	312.60	311.25	10
6708298		A	556979	4816412	324.12	317.24	10
6708304		A	557174	4811430	298.09	297.35	9
6708316		A	564005	4811629	315.49	318.97	7
6708318		A	558710	4807699	302.34	300.83	8
6708330		A	557169	4810613	296.81	298.00	10
6708331		A	563847	4810750	314.08	314.43	7

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6708332		A	560518	4810649	309.64	310.02	9
6708334		A	563722	4814423	325.06	327.83	7
6708335		A	558768	4807878	305.12	301.66	6
6708337		A	568654	4807926	304.94	305.05	5
6708385		A	561055	4804537	288.55	299.37	8
6708408		A	564837	4809477	303.79	305.44	10
6708492		A	558704	4807771	303.17	300.91	9
6708511		A	566614	4809841	305.26	307.89	7
6708512		A	562224	4810532	309.52	308.44	11
6708515		A	555134	4815363	321.87	319.37	9
6708537		A	566480	4808321	303.07	306.90	7
6708538		A	562117	4810801	313.90	310.27	11
6708539		A	560043	4812496	308.34	313.40	7
6708573		A	558533	4807706	303.29	299.77	8
6708574		A	570657	4808699	299.66	311.91	10
6708588		A	556675	4816537	324.53	319.92	9
6708668		A	557211	4815602	314.51	309.43	13
6708696		A	560291	4806472	301.31	302.83	5
1301369		A	552644	4792223	264.27	259.36	7
6708697		A	560251	4807792	299.65	304.30	3
6708698		A	570380	4807250	285.93	289.64	10
6708732		A	555853	4815384	318.90	317.54	9
6708737		A	564883	4806660	301.15	302.99	5
6708738		A	565623	4814068	323.46	328.24	5
6708739		A	559257	4810429	303.25	306.19	6
6708761		A	559029	4814495	310.90	306.44	11
6708795		A	564573	4811089	317.38	314.60	9
6708859		A	560922	4812700	312.99	315.01	7
6708884		A	557913	4810060	294.46	300.18	10
1301386		A	557214	4793973	265.48	265.02	3
6708978		A	564393	4813685	324.76	326.98	8
6709102		A	570456	4811904	313.14	321.64	10
6709103		A	561313	4812744	315.55	316.82	9
6709104		A	557395	4810083	300.79	298.35	5
1301407		A	560384	4794593	257.25	252.05	5
6709134		A	562652	4811099	314.33	315.56	7
1301418		A	558584	4792773	263.96	258.13	4
6709136		A	558693	4808905	302.32	302.93	9
6709165		A	558520	4807954	303.92	300.81	9
6709166		A	559976	4808071	303.42	303.56	8
6709237		A	564560	4809542	303.21	306.41	8
6709239		A	563936	4814418	329.72	328.16	8
6709266		A	563251	4811449	319.81	318.14	9
1301423		A	552504	4792793	286.82	271.65	9
6709322		A	565540	4809717	301.34	305.14	10

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6709387		A	561698	4812783	320.51	318.86	8
6709407		A	559794	4804364	295.31	298.12	5
6709411		A	562516	4813269	320.73	322.45	9
6709414		A	560033	4807897	304.87	303.50	9
6709478		A	570658	4812485	316.99	318.15	13
6709523		A	559626	4808263	303.22	303.67	3
6709524		A	558761	4807902	303.13	301.57	8
6709654		A	565776	4810979	307.90	309.19	10
1301497		A	553304	4791643	254.79	251.50	9
6709668		A	560047	4808075	290.86	303.71	6
1301529		A	549624	4791783	277.98	292.10	6
6709690		A	559882	4810450	309.94	307.87	7
6709695		A	557702	4810087	301.02	299.29	9
6709696		A	557673	4810022	297.64	299.40	9
6709781		A	570746	4810738	307.44	320.46	11
6709784		A	570964	4810635	309.58	318.94	12
6709806		A	562954	4814666	323.90	326.66	9
1301570		A	558814	4794293	265.74	259.65	8
1301573		A	553034	4791313	254.51	249.36	7
6709949		A	558624	4807617	300.26	300.09	9
6710008		A	569484	4811644	309.80	314.19	13
6710044		A	560807	4812870	320.09	315.75	7
6710048		A	570834	4812414	313.57	323.41	9
6710052		A	559124	4804290	301.90	297.54	5
6710076		A	556243	4817787	325.47	316.29	13
6710086		A	567885	4807982	303.32	308.12	6
1301585		A	558264	4794263	264.69	261.18	9
6710120		A	560763	4812634	314.90	314.61	5
6710189		A	559719	4808057	300.82	303.21	9
6710190		A	559555	4808166	300.70	303.17	9
6710193		A	560634	4808488	305.90	306.02	3
6710204		A	562822	4808706	306.90	305.48	6
6710209		A	557377	4812944	295.94	297.13	10
6710210		A	557633	4811412	292.64	299.09	9
6710255		A	564398	4812866	318.76	324.22	6
6710265	TLR-8	A	558876	4805927	303.34	300.58	3
6710266		A	558964	4810119	306.51	304.97	6
6710331		A	557060	4814603	304.62	307.03	10
6710352		A	566749	4808122	303.90	307.65	5
6710389		A	560432	4806518	310.11	303.03	5
1301663		A	553394	4791813	254.89	252.38	9
6710443		A	570891	4810871	314.32	321.09	8
6710562		A	568664	4809522	293.66	312.56	12
1301693		A	553204	4791713	249.94	250.32	9
6710588		A	571136	4806891	284.33	287.78	10

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6710591		A	569870	4808773	308.19	312.51	10
6710679		A	568227	4806364	278.44	289.35	10
6710699		A	561323	4806150	301.85	302.84	5
6710707		A	560305	4807006	302.93	303.72	3
6710709		A	560240	4807004	302.94	303.68	3
6710710		A	562260	4807841	301.21	304.02	9
1301704		A	560089	4793003	237.36	244.50	7
6710760		A	560624	4806659	300.28	303.21	6
6710781		A	559647	4808247	303.67	303.46	9
6710782		A	560027	4808464	304.43	304.52	6
6710846	TLR-22	A	559118	4804909	302.59	299.46	3
6710892		A	560716	4805296	295.78	300.88	5
6710903		A	561448	4810926	309.51	311.60	9
6710907		A	559463	4808180	303.66	302.95	9
6710911		A	559602	4808015	304.73	303.20	6
6710923		A	560601	4813308	314.34	306.22	12
1301819		A	556834	4792303	263.35	267.86	6
6710938		A	565179	4812165	316.50	319.52	8
6710945		A	565588	4811242	304.33	310.97	13
6711019		A	566983	4807820	306.68	307.73	4
6711020		A	560511	4807452	302.67	304.34	3
6711022		A	558684	4807356	304.48	299.58	9
6711023		A	558637	4807406	303.48	299.28	9
6711024		A	557405	4810154	300.54	298.21	5
6711025		A	560192	4807793	302.87	304.20	3
6711086		A	561600	4813188	322.38	320.33	9
6711096		A	563365	4811610	312.02	318.77	10
6711125		A	570908	4810689	308.19	320.32	11
6711134		A	558137	4815151	295.04	305.79	13
6711197		A	568579	4808241	307.50	308.38	8
6711217		A	568976	4807137	282.60	293.13	14
6711233		A	558030	4814444	296.51	305.04	12
6711245		A	560786	4812872	297.53	308.33	11
6711255		A	559657	4814716	312.68	308.11	11
1301933		A	553084	4791623	252.68	248.21	9
6711271		A	562979	4805237	301.56	299.94	6
6711314		A	557659	4815116	299.08	305.75	10
6711317		A	568454	4809354	302.22	312.80	7
6711321		A	558753	4805659	304.17	299.89	5
6711329		A	560081	4807881	304.09	303.58	9
6711367		A	558153	4815055	283.84	305.69	13
6711378		A	569488	4811604	310.71	316.88	10
6711394		A	567617	4812179	305.65	312.37	12
6711417		A	570888	4810595	310.19	320.29	10
1301990		A	552687	4792316	266.40	273.68	4

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6711455		A	569561	4811898	307.93	312.68	13
6711465		A	562068	4811221	303.20	305.52	13
1301991		A	554559	4792305	256.03	254.94	9
6711485		A	558821	4807893	303.08	301.65	9
6711490		A	561022	4806414	300.32	303.23	5
6711545		A	570823	4810768	311.71	320.84	10
6711554		A	569265	4808543	306.93	309.46	10
6711558		A	557817	4812008	300.99	299.96	9
6711570		A	570942	4810908	305.85	320.50	11
6711633		A	559103	4814684	306.12	304.93	12
6711662		A	559777	4810373	300.32	307.53	7
6711670		A	568703	4808305	299.66	308.52	7
6711671		A	568994	4807658	293.64	298.28	10
6711680		A	566332	4808750	303.43	305.67	10
6711686		A	564340	4814678	326.37	319.54	11
1302009		A	552738	4792327	264.26	269.59	6
6711723		A	569656	4811874	311.54	313.24	13
6711724		A	569957	4811308	306.14	317.58	13
6711941		A	558276	4816094	309.33	310.28	10
6711986		A	560294	4806300	304.42	302.30	8
1302084		A	553123	4791473	267.92	249.30	5
6712094		A	557115	4816580	328.09	318.96	9
6712121		A	566524	4808456	303.89	306.89	5
6806669		A	566734	4805783	289.87	296.33	7
6806678		A	568254	4806123	281.02	284.84	10
6806722		A	567254	4802763	267.31	270.40	10
6806726		A	567734	4805703	281.33	284.48	10
6806986		A	570484	4806323	283.46	284.25	10
6807040		A	562254	4795523	250.74	245.54	10
1302126		A	552853	4791778	243.17	251.35	10
6807195		A	568034	4801653	265.42	266.43	10
6807205		A	561774	4796043	254.21	249.45	10
6807278		A	562584	4794433	237.41	239.24	10
6807281		A	562754	4795523	245.06	244.04	10
6807324		A	562724	4795523	245.67	244.11	10
6807357		A	566514	4803973	281.63	282.11	10
6807475		A	568094	4801743	264.53	266.95	10
6807488		A	567114	4800873	263.34	263.55	10
1302131		A	558664	4792807	260.60	257.65	4
6807566		A	566644	4800503	262.44	262.94	10
6807573		A	566264	4800403	263.35	263.03	10
6807651		A	566854	4804943	283.16	286.80	10
6807662		A	569474	4805253	275.85	279.17	10
6807729		A	561964	4796683	258.56	251.38	10
6807805		A	569634	4803953	276.46	274.86	10

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6807902		A	562634	4794433	238.96	239.00	10
6807903		A	562679	4794433	238.08	238.78	10
6807988		A	570469	4803413	275.85	271.69	10
6808030		A	565834	4802483	268.53	269.45	10
6808109		A	563154	4796023	245.06	247.52	10
6808151		A	567794	4802363	267.92	269.32	10
6808206		A	565754	4804503	287.40	289.89	9
6808297		A	567934	4806063	283.14	288.11	9
6808298		A	566154	4805633	298.09	297.86	6
6808337		A	565154	4803303	280.60	281.90	8
6808338		A	562494	4803903	295.05	294.71	9
6808383		A	565580	4805496	298.40	297.58	9
6808452		A	569058	4803260	272.79	274.73	10
6808535		A	570886	4806280	282.33	284.65	10
6808609		A	566343	4804674	282.85	288.75	6
6808650		A	567782	4803512	275.23	276.02	10
6808651		A	567787	4803488	275.54	275.87	10
6808652		A	560834	4799482	272.18	282.85	8
6808704		A	566433	4802621	267.88	268.94	10
6808744		A	561288	4803535	291.72	295.91	8
6808832		A	568543	4803954	275.82	276.42	10
6808846		A	570019	4806551	282.68	285.25	10
6808882		A	566498	4802646	267.92	268.99	10
6808892		A	565007	4799270	261.82	261.38	10
6808908		A	567461	4804911	281.02	282.16	10
6808930		A	565168	4803652	284.07	286.38	10
6809004		A	565919	4799961	263.34	261.98	10
6809006		A	562593	4794795	241.40	240.73	10
6809034		A	570155	4803124	273.77	273.98	10
6809036		A	561333	4797600	254.26	265.28	10
6809045		A	563239	4800994	268.83	273.75	9
6809048		A	563813	4799911	258.47	263.27	10
6809053		A	563742	4798423	253.60	256.47	10
6809089		A	569256	4803452	275.54	275.00	10
6809147		A	566581	4802685	266.31	269.09	10
6809170		A	566590	4800211	263.05	261.74	10
6809285		A	569712	4803505	272.80	274.84	10
6809288		A	560734	4800883	287.43	287.79	9
6809308		A	563196	4800192	264.12	265.48	10
6809317		A	566800	4802722	267.00	269.25	10
6809320		A	563631	4800548	262.12	265.83	10
6809381		A	569972	4805988	281.02	282.48	10
6809438		A	570194	4806583	282.55	285.14	10
6809442		A	571614	4806723	289.87	286.71	10
6809490		A	562134	4796023	250.85	248.57	10

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6809508	DW	A	563213	4799661	262.13	262.24	10
6809512		A	562513	4799501	259.69	268.03	10
6809517		A	566014	4799883	260.90	261.53	10
6809519		A	566134	4800023	263.66	262.34	10
6809520	DW	A	564573	4804221	291.09	290.35	10
6809583		A	563433	4799833	261.34	263.00	10
6809584		A	563784	4798582	252.38	259.36	10
6809651		A	570194	4805143	277.06	279.50	10
6809765		A	564834	4804263	287.12	290.26	10
6809789		A	562194	4796803	255.42	252.19	10
6809866		A	562354	4795423	241.76	244.62	10
1302491		A	553234	4791683	238.46	250.80	9
6809880		A	562514	4794823	237.75	241.23	10
6809902		A	563554	4798363	256.07	258.10	10
6809903		A	563514	4798503	257.86	259.87	10
6809920		A	568454	4802563	263.81	271.61	10
6809988	DW-DETZLER	A	561447	4799410	271.89	280.45	9
6810021		A	566774	4805723	284.08	295.33	10
1302492		A	553074	4791443	271.88	248.99	4
6810023		A	562574	4797983	258.47	257.95	10
6810025		A	567294	4805123	281.94	283.39	10
6810036		A	563274	4797243	252.37	252.91	10
1302493		A	552474	4792263	263.65	260.90	7
6810079		A	562514	4794843	237.74	241.31	10
1302495		A	553114	4791423	257.86	249.50	7
6810198		A	566494	4805703	291.76	297.21	9
6810294		A	565234	4802783	276.09	272.14	10
6810295		A	565554	4804294	284.40	288.25	10
6810324		A	567114	4804003	286.52	279.19	10
6810331		A	567194	4805343	282.59	285.76	10
6810440		A	569494	4803123	271.95	274.85	10
6810445		A	561774	4796063	252.99	249.53	10
1302499		A	552914	4791363	257.19	247.93	5
6810567		A	565934	4800803	261.52	264.13	10
6810581		A	567854	4802143	265.17	268.29	10
6810584	DW-FRATHICANTELI	A	560101	4803067	292.92	292.45	9
1302502		A	552914	4791323	254.51	248.17	5
6810591	DW-ROBERTSON	A	562472	4803706	297.48	294.14	3
6810665		A	565674	4804223	282.85	287.32	8
6810704		A	566374	4802523	266.09	268.63	10
6810707		A	566394	4802563	268.83	268.76	10
6810751	DW-RUNIONS	A	562128	4803740	297.18	295.35	3
6810754	DW-RUNIONS CONSTRUCT	A	562191	4803739	298.40	295.16	3

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6810755	DW-RUNIONS CONSTRUCT	A	562247	4803809	300.23	295.33	3
6810763		A	566614	4802523	265.48	268.54	10
6810764		A	567414	4805383	285.74	283.91	10
6810765		A	568674	4804545	274.62	277.21	11
6810868		A	565563	4804443	289.25	289.49	8
6810921		A	562584	4795565	248.10	244.85	10
6811013		A	565637	4803821	286.81	285.38	10
6811108		A	565664	4804350	285.60	288.67	9
6811109		A	565622	4803860	286.20	285.63	7
6811135		A	563804	4798570	258.05	259.39	10
6811241		A	569386	4806388	283.16	284.33	10
6811326		A	562551	4794291	238.66	238.74	10
6811365		A	561792	4796060	255.12	249.48	10
6811421		A	563285	4797492	253.29	254.03	10
6811525		A	561331	4798243	264.87	270.07	9
6811588		A	565679	4803813	285.29	284.91	10
6811684		A	567207	4802740	268.83	270.16	10
6811754		A	569996	4804693	277.67	277.36	10
1302719		A	551774	4792683	292.61	272.55	9
6811808		A	567017	4804865	281.63	284.10	10
6811814		A	568132	4803075	274.93	273.64	10
6811849		A	562674	4795531	242.24	244.34	10
6811881		A	562018	4795463	249.02	245.97	10
6812015		A	561335	4798080	263.65	268.23	10
6812065		A	563833	4798546	252.96	259.11	10
6812087		A	566217	4804594	281.63	288.70	10
6812186		A	565639	4804081	286.22	286.62	9
6812253		A	567311	4802733	267.31	270.37	10
6812293		A	566456	4802531	268.53	268.61	10
6812335		A	562348	4798103	256.95	258.83	10
6812492		A	561811	4796097	257.25	249.60	10
6812493		A	569198	4803163	270.06	274.71	10
6812502		A	568130	4803073	276.45	273.62	10
6812522		A	568967	4802833	268.23	273.01	11
6812534		A	566159	4804653	284.08	289.60	9
6812649		A	566001	4804272	286.21	286.91	10
6812702		A	565633	4803943	288.34	285.82	10
6812707		A	569582	4805390	280.41	279.91	10
6812781		A	570407	4804325	277.37	276.40	10
6812808		A	565277	4804373	285.26	289.43	8
6812854		A	565565	4802723	276.15	272.05	10
6812946		A	566386	4800703	263.04	263.66	10
1302970		A	552634	4792283	261.71	260.34	9
1302974		A	553634	4791923	252.98	253.88	9

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
1302978		A	553454	4791383	257.47	253.92	8
1302996		A	552046	4792432	270.34	278.37	6
1303052		A	552433	4792327	262.90	262.27	9
9200700		A	548456	4807938	283.94	290.17	7
9200719		A	557001	4810721	295.53	296.86	10
9200721		A	557451	4801640	276.54	279.06	9
9200724		A	556772	4810224	295.05	299.78	10
9200725		A	551658	4807022	282.37	288.29	9
9200726		A	552374	4807526	283.19	290.90	12
1303056		A	553311	4791475	255.66	252.01	8
1303061		A	554338	4791320	269.32	261.15	9
9200780		A	548480	4809788	287.49	295.71	6
9200785		A	554291	4812008	297.42	306.26	10
9200817		A	548923	4801999	297.08	296.08	9
9200818		A	548992	4803452	282.30	284.66	6
9200819		A	547746	4794949	300.33	307.10	5
9200820		A	547868	4793526	293.20	299.54	7
9200821		A	553579	4798647	306.46	299.75	6
9200826		A	563544	4799437	261.18	262.25	10
9200831		A	549507	4795524	308.05	313.56	6
9200833		A	549695	4798014	305.40	322.14	5
9200834		A	548864	4796903	310.12	319.40	5
1303207		A	552668	4792139	259.87	259.96	6
9200841		A	559592	4797053	276.41	273.14	9
9200844		A	560176	4802613	292.23	290.39	8
9200845		A	560160	4802537	293.28	290.17	9
9200846		A	560505	4802504	285.30	291.10	9
9200847		A	561069	4799970	282.50	285.18	9
9200848		A	560033	4801306	286.34	287.41	8
9200849		A	560039	4801353	286.34	287.48	8
9200851		A	563718	4803139	270.85	280.75	10
9200852		A	563480	4803787	291.16	287.71	9
9200853		A	562644	4803987	299.11	294.70	2
9200859		A	549089	4801617	305.46	300.61	9
9200860	PW1	A	548906	4802005	296.65	296.18	9
9200880		A	550023	4811333	305.41	308.22	7
9200889		A	556480	4797084	265.75	271.02	9
9200891		A	560304	4802829	290.83	291.86	9
9200892		A	560790	4800071	285.17	285.82	8
9200894		A	555275	4796334	265.08	269.10	10
1303291		A	552982	4790926	255.58	250.30	5
9200985		A	552091	4807084	285.22	287.32	7
9200988		A	549719	4807759	285.47	293.04	7
1303292		A	553396	4791826	253.63	252.39	8
1303293		A	560572	4793690	241.60	244.80	10

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
9201022		A	552082	4807078	284.63	287.29	7
1303294		A	553011	4791467	249.04	248.13	9
9201077		A	557088	4810792	297.09	300.62	13
9201079		A	558613	4805721	302.64	299.76	5
9201080		A	551656	4810059	301.68	308.24	6
9201087		A	560263	4802943	295.32	292.31	9
9201128		A	560436	4799001	281.30	280.09	7
9201134		A	551523	4798669	309.12	320.34	5
9201137		A	555653	4797319	260.36	263.09	9
9201138		A	550383	4792475	283.81	286.82	6
9201139		A	555734	4798834	260.42	261.88	9
9201140		A	563181	4804918	298.69	298.36	6
9201142		A	563186	4801492	274.55	278.61	8
9201144		A	549038	4804390	276.37	279.24	7
1303375		A	552886	4791362	251.88	247.65	5
9201173		A	547519	4795672	300.77	307.69	6
9201179		A	553423	4807925	282.41	291.77	12
1303379		A	553283	4791370	258.75	252.00	7
9201188		A	563189	4803818	293.49	289.57	7
9201190		A	560133	4803580	293.86	295.53	6
1303380		A	553339	4791389	259.66	252.58	7
9201226		A	549794	4811563	307.51	307.96	9
9201234		A	548861	4801793	296.71	298.33	9
9201242		A	552011	4812693	308.75	314.03	14
9201248		A	548841	4801333	302.61	307.38	6
1303396		A	552511	4792185	261.56	266.94	6
1303425		A	553654	4791945	251.62	253.71	9
1303426		A	556702	4793863	262.87	264.91	7
1303431		A	553201	4791413	252.96	250.80	7
9201575		A	560646	4801013	287.67	287.91	5
1303523		A	556646	4792280	264.85	268.05	7
9201617		A	555546	4813343	315.77	308.93	8
1303525		A	560972	4793415	233.18	238.59	10
9201640		A	550425	4792479	284.89	286.65	6
9201651		A	551331	4806867	287.80	285.84	9
9201656		A	563617	4799198	259.96	261.73	10
1303526		A	552836	4791313	251.75	247.62	5
9201676		A	549422	4806853	305.25	289.72	6
9201757		A	564673	4800512	263.10	264.28	10
1303527		A	556706	4791311	293.25	271.77	3
9201825		A	556926	4795025	246.20	265.64	10
9201826		A	556932	4795092	245.40	265.70	10
9201828		A	560123	4803704	294.90	295.99	9
9201844		A	549845	4805600	286.33	275.99	8
1303640		A	553046	4791153	255.40	250.32	5

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
1303641		A	552271	4792093	263.17	260.76	7
9201948		A	564898	4799469	260.52	261.81	10
9201991		A	549368	4799244	305.18	319.51	6
1303664		A	551710	4792638	282.60	282.05	6
1000372	DW-DREWREY	A	556150	4808240	303.91	296.27	9
1303666		A	560567	4794038	248.43	247.23	8
9202058		A	560052	4795271	270.73	259.11	8
9202125		A	548192	4807323	293.33	289.42	6
9202161		A	548343	4808987	276.05	294.53	9
1303686		A	553072	4791377	254.62	249.25	8
1303687		A	553020	4791148	255.09	250.09	6
1303688		A	553143	4791446	256.97	249.86	9
9202470		A	551531	4800208	316.04	313.00	5
9202579		A	549655	4808725	292.22	299.49	7
1303724		A	553126	4791427	257.05	249.67	9
1303781		A	560922	4794879	249.90	249.07	10
1303782		A	553054	4791241	256.09	249.99	7
1303783		A	553001	4791152	255.40	249.93	6
1303903		A	553032	4791473	258.80	248.30	8
1303904		A	553103	4791493	261.15	249.09	7
1303927		A	552965	4791143	255.70	249.65	6
1303934		A	553016	4791313	255.09	249.17	8
1304002		A	558454	4792392	250.76	258.60	10
1304136		A	552794	4791171	251.09	248.00	8
1304225		A	552862	4791184	251.09	248.41	9
1304226		A	552773	4791085	250.40	248.21	8
1304264		A	552837	4791139	254.14	248.54	6
1304265		A	552815	4791103	254.44	248.54	6
1304316		A	552841	4791262	252.75	247.91	6
1304364		A	552835	4791241	251.92	247.92	8
1304542		A	553353	4791550	259.94	252.57	6
1304544		A	552924	4791281	250.40	248.42	8
1304545		A	553373	4791503	258.76	252.79	6
1304557		A	556694	4791367	290.20	271.77	3
1304718		A	552585	4792198	261.26	266.52	6
1304833		A	554743	4792379	252.19	256.46	9
1304944		A	555654	4791955	263.32	267.84	6
1304948		A	552185	4792011	260.26	260.72	7
1304949		A	552837	4790613	249.26	249.60	7
1304950		A	552412	4792178	262.34	267.82	6
1304951		A	552384	4792175	262.43	267.83	6
1304952		A	552556	4792192	260.34	266.67	6
1305076		A	553410	4791406	257.97	253.38	9
1305143		A	551954	4792039	262.04	263.55	6
1305144		A	553225	4792672	260.03	265.91	7

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
1305200		A	552725	4791075	249.84	247.87	6
1000406	DW-PIONEER SPORTSMAN	A	547796	4806301	284.90	283.78	7

Table D-2 Groundwater level targets derived from Regional of Waterloo Monitoring Wells

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6500605	G8TW	A	558343	4802630	281.70	282.82	8
1000545	OW57-95	A	558498	4805228	302.33	297.71	6
1000328	OW1-95	B	554877	4809921	295.03	299.52	14
1000328	OW1-95	C	554877	4809921	299.86	299.82	10
1000328	OW1-95	D	554877	4809921	303.85	300.38	9
1000328	OW1-95	E	554877	4809921	303.91	300.83	6
1000328	OW1-95	F	554877	4809921	310.51	301.82	4
1000329	OW2-95	B	556154	4808464	294.13	297.76	14
1000329	OW2-95	C	556154	4808464	294.68	297.22	10
1000329	OW2-95	D	556154	4808464	302.53	296.85	8
1000329	OW2-95	E	556154	4808464	302.70	298.81	6
1000329	OW2-95	F	556154	4808464	302.92	302.55	6
6504495	TW4-76	A	550331	4807756	288.83	294.32	6
6504527	AVE-10	A	558681	4803844	291.88	292.52	9
1000330	OW3-95	B	555560	4806662	288.67	292.08	12
1000330	OW3-95	C	555560	4806662	288.76	292.13	12
1000330	OW3-95	D	555560	4806662	293.65	292.17	10
1000330	OW3-95	E	555560	4806662	297.00	291.80	9
1000330	OW3-95	F	555560	4806662	296.72	291.81	9
1000330	OW3-95	G	555560	4806662	296.22	291.89	6
1000332	OW5-95	B	553286	4802223	264.64	274.10	13
1000332	OW5-95	C	553286	4802223	264.69	269.03	10
1000332	OW5-95	D	553286	4802223	291.85	282.12	8
6506243	DW-RUNIONS CONSTRUCT	A	562286	4803923	298.71	295.74	3
1000332	OW5-95	E	553286	4802223	292.45	283.10	8
6506418	DW-RUNIONS CONSTRUCT	A	562124	4803692	297.82	295.09	3
1000332	OW5-95	F	553286	4802223	299.49	291.11	3
1000333	OW6ABCD-95	A	553898	4803784	265.50	270.16	15
1000333	OW6ABCD-95	B	553898	4803784	265.00	269.75	14
1000333	OW6ABCD-95	C	553898	4803784	265.40	269.60	10
1000936	C2	A	559342	4806841	303.57	302.08	6
1000333	OW6ABCD-95	D	553898	4803784	274.10	275.96	9
1000333	OW6ABCD-95	E	553898	4803784	284.04	281.27	9
6507562	OW6-94 (TW2-91)	B	556283	4804365	271.33	281.58	14
6507562	OW6-94 (TW2-91)	C	556283	4804365	271.03	281.41	13
6507562	OW6-94 (TW2-91)	D	556283	4804365	271.30	281.68	10
6507564	BH2B-93	A	555597	4799776	253.30	258.07	13
6507564	BH2B-93	B	555597	4799776	253.30	258.07	13
6507565	BH2A-93	A	555589	4799783	255.70	258.04	10
6507565	BH2A-93	B	555589	4799783	259.81	258.07	9
6507566	BH1-93	A	555130	4799683	252.63	255.40	15
6507566	BH1-93	B	555130	4799683	253.01	255.13	13

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6507566	BH1-93	C	555130	4799683	251.78	255.00	10
1000335	OW8-95	B	557571	4799770	271.32	273.03	14
1000335	OW8-95	C	557571	4799770	271.25	273.17	11
1000335	OW8-95	D	557571	4799770	271.22	276.89	10
1000335	OW8-95	E	557571	4799770	277.86	277.24	9
1000335	OW8-95	F	557571	4799770	277.37	277.56	6
1000335	OW8-95	G	557571	4799770	277.87	277.81	3
1000336	OW9-95	B	552449	4801218	300.69	297.23	13
1000336	OW9-95	C	552449	4801218	300.84	298.80	12
1000336	OW9-95	D	552449	4801218	304.22	304.23	9
1000336	OW9-95	E	552449	4801218	304.75	304.37	8
1000336	OW9-95	F	552449	4801218	303.14	305.08	6
6710783	DW-ST.PIERRE	A	559620	4808220	304.57	303.49	6
1000336	OW9-95	G	552449	4801218	308.35	306.69	3
1000336	OW9-95	H	552449	4801218	308.78	306.70	3
1000354	BH4-94	A	554494	4799393	251.60	258.35	10
1000354	BH4-94	B	554494	4799393	266.70	258.35	10
1000354	BH4-94	C	554494	4799393	272.56	264.08	9
1000354	BH4-94	D	554494	4799393	277.52	275.96	9
6810757	DW-REYNOLDS	A	562554	4804324	298.99	296.89	3
9070064	OW104-90	A	554573	4800873	265.89	273.65	14
9070064	OW104-90	B	554573	4800873	267.46	277.51	10
9070064	OW104-90	C	554573	4800873	270.42	283.93	9
9070064	OW104-90	D	554573	4800873	278.80	285.05	7
9200047	MI-OW1-92	A	555241	4799600	259.90	255.66	9
9200047	MI-OW1-92	B	555241	4799600	261.49	255.81	9
9200047	MI-OW1-92	C	555241	4799600	261.47	255.88	9
9200048	MI-OW2-92	A	555245	4799601	251.75	255.37	10
9200048	MI-OW2-92	B	555245	4799601	252.39	255.44	9
9200048	MI-OW2-92	C	555245	4799601	258.58	255.62	9
9200458	SM3-93	B	557938	4803410	286.47	288.37	6
9200473	BH6-94	A	555238	4799599	261.94	255.93	9
9200572	CM-OW5ABCDEFGF-94	B	558346	4806731	303.80	297.54	14
9200572	CM-OW5ABCDEFGF-94	D	558346	4806731	303.91	297.66	9
9200572	CM-OW5ABCDEFGF-94	E	558346	4806731	304.12	297.74	7
9200573	SM-OW2BC-93	B	557822	4803116	285.09	286.08	3
1000369	TW9-78	A	560267	4804378	296.96	298.43	9
1000370	MW1-94	A	562785	4806823	298.78	301.24	4
1000370	MW1-94	B	562785	4806823	298.98	300.47	4
9201544	BM1SD-02	A	559654	4795434	269.05	263.27	9
9201544	BM1SD-02	B	559654	4795434	270.53	263.60	3
9201548	SM1-02	A	557344	4802731	280.95	280.45	3
1000370	MW1-94	C	562785	4806823	298.65	300.39	3
9202045	CMTW1-05	A	558403	4805357	300.43	296.03	15

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
9202047	SMTW1ABC-05	B	558520	4803673	289.55	291.42	10
9202047	SMTW1ABC-05	C	558520	4803673	286.42	290.73	9
9202049	CMTW3ABC-05	A	557560	4805181	299.75	293.76	15
9202049	CMTW3ABC-05	C	557560	4805181	302.81	294.02	9
9202055	DW-REED	A	558578	4806477	303.08	298.47	8
9202639	CMOW3ABC-06	A	558916	4804867	300.33	296.06	15
9202639	CMOW3ABC-06	B	558916	4804867	301.07	297.39	9
9202639	CMOW3ABC-06	C	558916	4804867	301.07	297.98	9
9202642	HPOW1AB-06	A	558786	4804395	298.80	297.14	5
9202643	PBOW1ABC-06	A	559286	4806058	301.36	297.76	15
9202643	PBOW1ABC-06	B	559286	4806058	303.26	299.71	9
9202646	PBTW2-06	A	558220	4806227	302.93	296.76	15
9202647	PLOW1AB-06	B	559403	4806775	304.71	303.05	2
9202664	CMOW1D-06	D	558724	4805313	302.53	298.66	8
9202668	CMOW2E-06	E	559588	4804174	297.22	297.65	5
9202670	PBOW1D-06	D	559284	4806058	303.26	300.42	8
9202672	PBOW1F-06	F	559282	4806059	303.93	301.55	3
9202673	PBOW2ABC-06	A	558535	4806924	302.94	298.04	15
9202673	PBOW2ABC-06	B	558535	4806924	303.07	298.16	10
9202673	PBOW2ABC-06	C	558535	4806924	303.16	298.17	9
9203304	DW-FROSCHE JR	A	558943	4805820	303.20	299.74	8
9203421	SM3ABC-07	A	557938	4803410	287.67	287.85	15
9203421	SM3ABC-07	B	557938	4803410	286.34	287.63	10
9203421	SM3ABC-07	C	557938	4803410	286.49	287.64	9
1000381	OW1-87	A	555159	4798045	258.37	259.88	8
1000382	CM-OW1BC-92	B	557015	4805952	298.15	294.37	6
1000322	TW3-94	A	557325	4802669	274.34	276.44	5
1000382	CM-OW1BC-92	C	557015	4805952	300.75	296.08	4
1000383	CM-OW1A-92	A	557019	4805950	297.25	292.65	13
1000384	CM-OW2BC-92	B	558283	4805068	301.99	297.07	6
1000388	OW3-92	A	557251	4803752	275.50	285.81	14
1000389	OW4A-92	A	558908	4804870	301.03	297.96	9
1000390	OW4BC-92	B	558911	4804870	300.94	298.05	6
1000390	OW4BC-92	C	558911	4804870	301.02	299.01	4
1000384	CM-OW2BC-92	C	558283	4805068	302.70	297.86	5
9203557	PBTW3-07	A	557466	4805908	302.74	294.51	15
6500146	H4	A	556693	4808882	302.00	298.99	12
6500132	P10	A	556951	4806839	295.17	289.53	8
1000021	P17	A	557129	4806112	284.30	289.02	13
6500130	P11	A	557138	4806113	289.50	284.63	13
6500109	G9	A	557175	4800261	260.00	263.32	11
1000388	OW3-92	B	557251	4803752	276.30	285.80	13
1000018	G38	A	557302	4802637	277.65	279.00	8
1000019	G39	A	557324	4802665	264.00	272.68	8

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
1000013	G18	A	557327	4804287	275.00	284.14	12
9200458	SM3-93	C	557938	4803410	286.80	289.65	3
9203611	NDTW1ABC-08	A	562367	4804125	300.85	297.48	15
9200458	SM3-93	A	557938	4803410	286.80	287.80	15
9200006	G7	A	558059	4802474	278.50	280.32	8
1000277	G16	A	558336	4804722	285.40	290.32	13
9200007	G8	A	558343	4802600	279.70	282.32	6
9203611	NDTW1ABC-08	B	562367	4804125	299.79	297.01	10
9203611	NDTW1ABC-08	C	562367	4804125	299.19	296.59	9
9203612	NDTW2-08	A	562220	4804076	301.11	297.47	15
9203613	NDTW1D-08	D	562368	4804123	298.88	296.57	6
9203614	NDTW1E-08	E	562369	4804124	298.71	296.47	3
9203616	NDOW1ABC-08	A	562254	4804736	301.86	297.61	15
9203616	NDOW1ABC-08	B	562254	4804736	299.50	299.29	10
9203616	NDOW1ABC-08	C	562254	4804736	299.54	299.35	9
9200056	CM-OW2-92 (SW)	D	558280	4805065	303.84	298.28	3
9203617	NDOW1D-08	D	562255	4804733	299.78	299.74	6
9203618	NDOW1E-08	E	562255	4804732	299.31	299.85	2
9203619	NDOW2ABC-08	C	561220	4803776	300.82	297.10	9
9203619	NDOW2ABC-08	A	561220	4803776	297.46	297.39	15
9203619	NDOW2ABC-08	B	561220	4803776	296.88	297.19	10
9203620	NDOW2D-08	D	561223	4803779	296.58	297.11	8
9203633	NDOW2E-08	E	561221	4803779	297.53	297.46	2
9203634	NDOW3A-08	A	562046	4803929	298.40	296.63	3
9203635	NDOW3B-08	B	562045	4803930	298.07	296.69	2
9203636	NDOW4A-08	A	562601	4803606	297.69	292.85	3
9200438	TW10-79	A	562405	4806913	302.85	302.25	9
9203637	NDOW4B-08	B	562601	4803608	297.30	292.89	2
9203638	NDOW5-08	A	563380	4805019	294.82	297.98	6
9202047	SMTW1ABC-05	A	558520	4803673	297.42	293.01	14
1000386	CM-OW2A-92	A	558285	4805067	301.99	295.26	13
6500647	DW-GRCA-66	A	558340	4802989	283.59	285.47	8
6505664	DW-ROMEO	A	557330	4803381	284.94	285.62	6
6708333	DW-OLIVER	A	559078	4804839	300.98	299.34	3
9070062	OW102-90	A	556059	4802433	272.76	278.68	15
9070062	OW102-90	C	556059	4802433	272.90	280.59	9
9202048	CMTW2-05	A	558964	4804452	299.95	295.81	15
9200466	SM-OW2A-93	A	557811	4803112	285.49	286.01	3
9200467	SM1-93	A	557683	4802847	280.81	283.17	3
9200467	SM1-93	B	557683	4802847	282.17	284.09	3
9200572	CM-OW5ABCDEFG-94	F	558346	4806731	305.18	301.95	2
9200728	DW-SOCCER WELL	A	558371	4805241	302.50	297.17	8
9201413	TW1-58	A	558047	4802475	280.16	281.08	8
9202046	PBTW1-05	A	558317	4806857	302.92	297.64	15

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
9202637	CMOW1ABC-06	A	558726	4805314	300.86	296.33	15
9202637	CMOW1ABC-06	B	558726	4805314	302.74	297.77	9
9202637	CMOW1ABC-06	C	558726	4805314	302.42	298.46	9
9202389		A	562166	4804197	299.03	297.51	9
9202638	CMOW2ABC-06	A	559589	4804174	299.82	296.46	15
9202638	CMOW2ABC-06	C	559589	4804174	297.45	297.43	9
9202643	PBOW1ABC-06	C	559286	4806058	303.98	300.17	9
9202647	PLOW1AB-06	A	559403	4806775	304.20	302.85	3
9202648	PLOW2ABC-06	A	559066	4806672	304.04	302.22	3
9202648	PLOW2ABC-06	B	559066	4806672	304.69	302.51	2
9202648	PLOW2ABC-06	C	559066	4806672	304.72	303.17	2
9202666	CMOW1F-06	F	558725	4805314	304.44	299.37	3
9202669	CMOW2F-06	F	559590	4804173	298.90	297.65	3
9202638	CMOW2ABC-06	B	559589	4804174	297.66	296.95	9
9202671	PBOW1E-06	E	559283	4806059	303.33	301.15	6
9202676	PBOW2F-06	F	558534	4806922	304.00	301.79	3
9203305	DW-FORD	A	558663	4803841	291.99	295.91	3
9203307	DW-SMALLBONE	A	559689	4804281	297.00	297.89	5
9203308	DW-ZIMMERMAN	A	558603	4806220	304.43	301.18	3
9203312	PTOW1	A	560024	4808548	305.26	304.65	3
9203419	SMTW2ABC-07	A	557669	4802638	285.00	284.15	15
9203419	SMTW2ABC-07	B	557669	4802638	281.57	284.12	13
9203419	SMTW2ABC-07	C	557669	4802638	281.21	281.41	7
9203420	SMTW3ABC-07	A	558418	4802867	288.60	288.32	14
9202667	CMOW2D-06	D	559590	4804174	297.16	297.52	7
9203420	SMTW3ABC-07	B	558418	4802867	288.49	284.86	10
9203420	SMTW3ABC-07	C	558418	4802867	284.91	284.70	9
9203540	SMOW1A-08	A	558970	4803479	291.65	291.41	8
9203541	SMOW1B-08	B	558971	4803478	291.13	289.98	3
9203542	SMOW1C-08	C	558971	4803480	291.90	290.11	2
9203543	SMTW4ABC-08	A	559026	4803335	299.50	294.47	15
6505937	P16	A	550347	4807752	291.00	293.06	7
6500110	G4	A	553871	4802268	265.00	266.29	10
1000333	OW6ABCD-95	F	553898	4803784	284.30	281.37	3
6507563	BH3A-93	D	554016	4799651	295.06	294.54	9
9202674	PBOW2D-06	D	558535	4806926	303.37	298.18	7
6507563	BH3A-93	C	554016	4799651	293.65	292.32	9
6507563	BH3A-93	B	554016	4799651	292.73	287.51	9
6507563	BH3A-93	A	554016	4799651	292.62	281.08	10
1000356	BH3B-93	C	554017	4799650	307.48	298.32	6
1000356	BH3B-93	B	554017	4799650	296.16	296.48	6
1000356	BH3B-93	A	554017	4799650	295.22	295.13	8
9200472	BH5-94	A	554040	4799655	292.78	286.76	9
6501155	P6	A	554159	4804014	260.40	264.77	13

OBJ_NUM	CONSLT ID	Screen ID	Easting	Northing	Observed water level (masl)	Simulated water level (masl)	Model Layer
6500129	G5	A	555084	4806561	285.00	289.85	9
9200001	G15	A	555170	4798053	253.00	259.66	11
9202675	PBOW2E-06	E	558536	4806927	303.49	299.38	6
9200004	G3	A	555183	4799708	240.00	251.63	10
9200003	G2	A	555213	4799684	238.00	250.33	10
9200002	G1	A	555242	4799693	246.00	251.90	13
6500142	H3	A	555314	4808183	287.00	294.30	13
6500148	H5	A	555327	4810826	299.39	301.09	13
9200010	P15	A	555758	4806627	287.30	291.48	12
6500128	P9	A	555792	4806582	284.70	290.99	12
1000017	G17	A	556271	4804365	270.00	278.57	14
6500131	G6	A	556355	4805062	275.00	279.63	13

Appendix E

Re-Interpretation of the Pinebush 15-day Pumping Test (October 2011)

Re-interpretation of the Pinebush 15-day pumping test (October 2011)

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Last update: October 6, 2015

Overview

A 15-day constant-rate pumping test was conducted at the Pinebush wellfield between October 4 and October 19, 2011 (Stantec, 2013). Deep bedrock wells TW1-10 and P10A were pumped, with subsequent pumping from the Contact Aquifer well P10B. The test is particularly important because it provides insights into the response of the deep bedrock to pumping in an area that is the focus of water resource development but that has not been investigated extensively. In this technical note the analyses conducted to constrain the refinement of aquifer properties in the Cambridge East area are documented. Well P10A was pumped during a separate five-day test in May 2010 (Burnside, 2010) and the results of that test are also analyzed. The data from step tests conducted on TW1-10 and P10A have also been analyzed. These data are used to isolate the linear and nonlinear components of the drawdowns in the pumping wells. It is important to isolate these components when attempting to reproduce well performance with a numerical groundwater flow model.

The data from the testing of TW1-10 and P10A suggest that the deep bedrock aquifer in the vicinity of these wells has a complex structure. However, it is possible to develop a coherent conceptual model that is consistent with the interpreted aquifer properties. The key elements of the conceptual model are:

- TW1-10 pumps from a zone in the deep bedrock that has a transmissivity of about 520 m²/d. This zone extends about 200 m from TW1-10. Beyond this distance the deep bedrock has a relatively lower transmissivity of about 150 m²/d; and
- A transmissivity of 160 m²/d is estimated from the P10A testing conducted in May 2010. The estimated transmissivity suggests that P10A is located close to the transition between the two zones inferred in the analysis of TW1-10.

1. Introduction

The constant-rate pumping test at the Pinebush well field began at 1:45 PM on October 4, 2011 and continued until 10:00 AM on October 19, 2011, a duration of 15 days. The pumping wells used for the test included TW1-10 and P10A, and the Contact Aquifer well P10B. Well P17 was also pumping cyclically during the test. The locations of the pumping wells are indicated in Figure 1.

The interpretation here is focused on the response to pumping in the deep bedrock. The drawdowns for the analysis are interpreted as the changes in water levels caused only by pumping from TW1-10 and P10A.

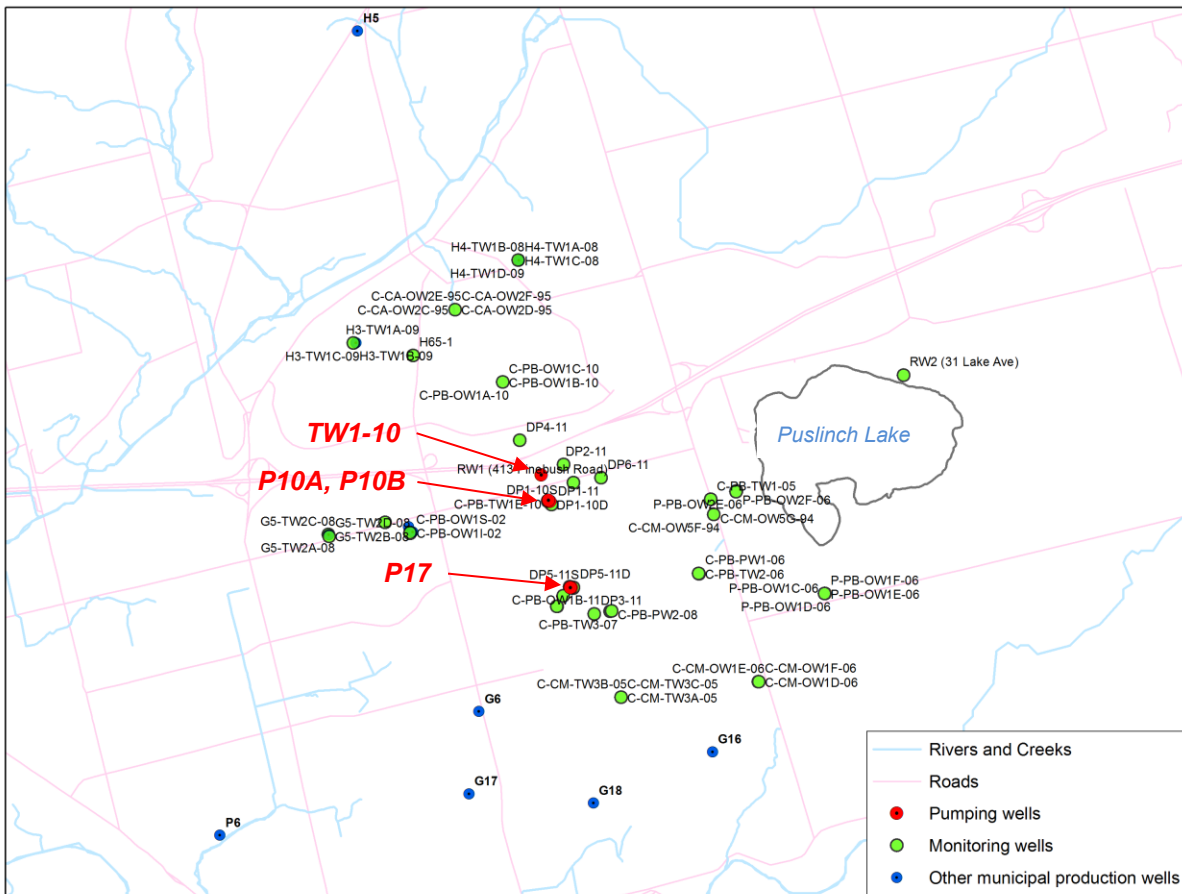


Figure 1. Locations of the pumping wells during the Pinebush 15-day test

2. Conceptual hydrogeologic model of the Pinebush area

The local hydrostratigraphic sections at the locations of test wells TW1-10 and P10A are shown in Figures 2 and 3. These sections have been extracted from the regional hydrostratigraphic model developed by S.S. Papadopoulos & Associates, Inc., based on the geologic model of Frank Brunton of the Ontario Geological Survey. The blue lines in the figures denote the open intervals of the wells. The red lines in the figures denote the major flow zones inferred from borehole flowmeter proofing conducted on the wells.

The flowmeter profiling identified three major flow zones in TW1-10:

- Flow zone at 67 m Below Ground Surface (BGS), in the Reformatory Quarry Member of the Eramosa Formation: 18% of the total flow;
- Flow zone at 72 m BGS, in the Goat Island Formation: 88% of the total flow; and
- Flow zone at 97 m BGS, in the upper portion of the Gasport Formation: 18% of the total flow.

The flowmeter profiling identified three major flow zones in P10A:

- Flow zones at 70 m and 80 m BGS, in the upper portion of the Gasport Formation: 75% of the total flow; and
- Flow zone at 106 m BGS, in the middle portion of the Gasport Formation: 15% of the total flow.

The yield from a fourth flow zone at 118 m BGS was not significant.

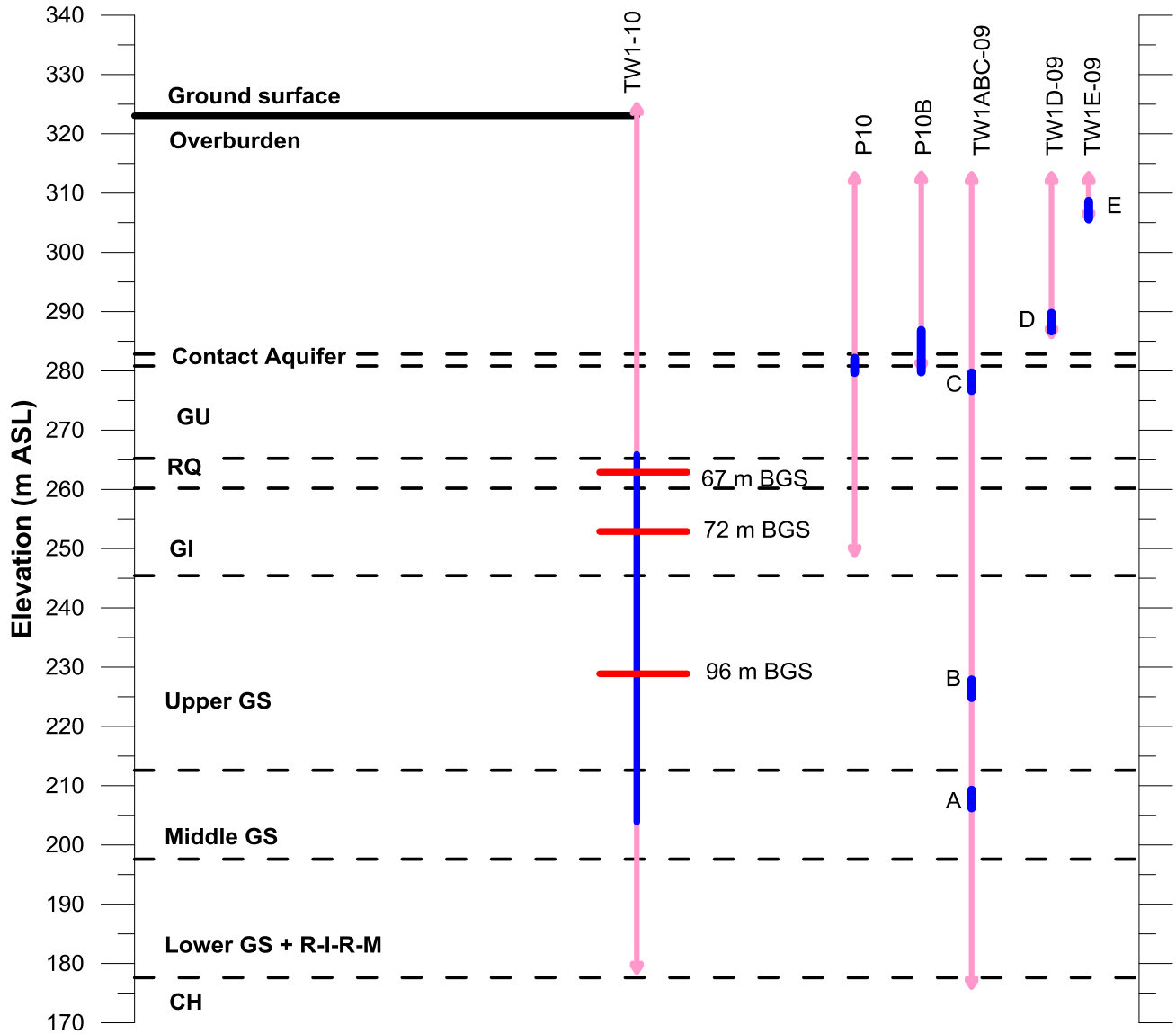


Figure 2. Local hydrostratigraphy at TW1-10

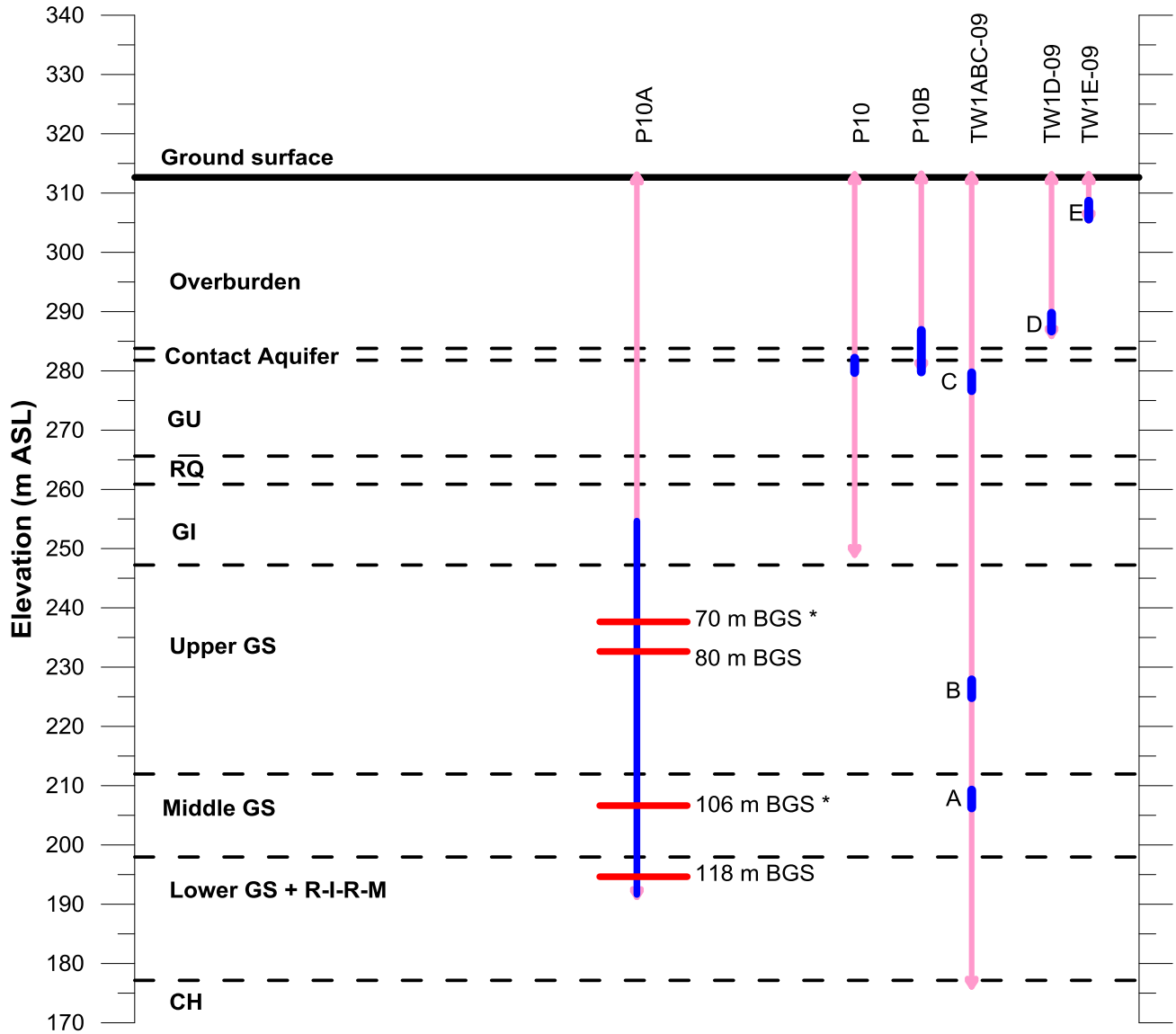


Figure 3. Local hydrostratigraphy at P10A

3. Pumping records of the Pinebush wells during the testing period

The pumping histories for TW1-10 and P10A during the Pinebush testing are summarized on Table 1. The pumping histories are plotted in Figure 4.

Table 1 Pumping rates of the test wells during the test

Time	Elapsed time (days)	Pumping rate (L/s)		
		TW1-10	P10A	Total
2011-10-04 13:45	0.0000	40	0	40
2011-10-07 12:00	2.9271	35	0	35
2011-10-07 12:30	2.9479	35	0	35
2011-10-12 9:00	7.8021	0	0	0
2011-10-12 10:15	7.8542	35	0	35
2011-10-12 11:00	7.8854	35	70	105
2011-10-12 11:15	7.8958	30	70	100
2011-10-12 11:25	7.9028	25	70	95
2011-10-12 11:35	7.9097	20	70	90
2011-10-12 12:40	7.9549	15	70	85
2011-10-12 13:45	8.0000	20	70	90
2011-10-17 16:00	13.0938	0	70	70
2011-10-19 10:00	14.8438	0	0	0

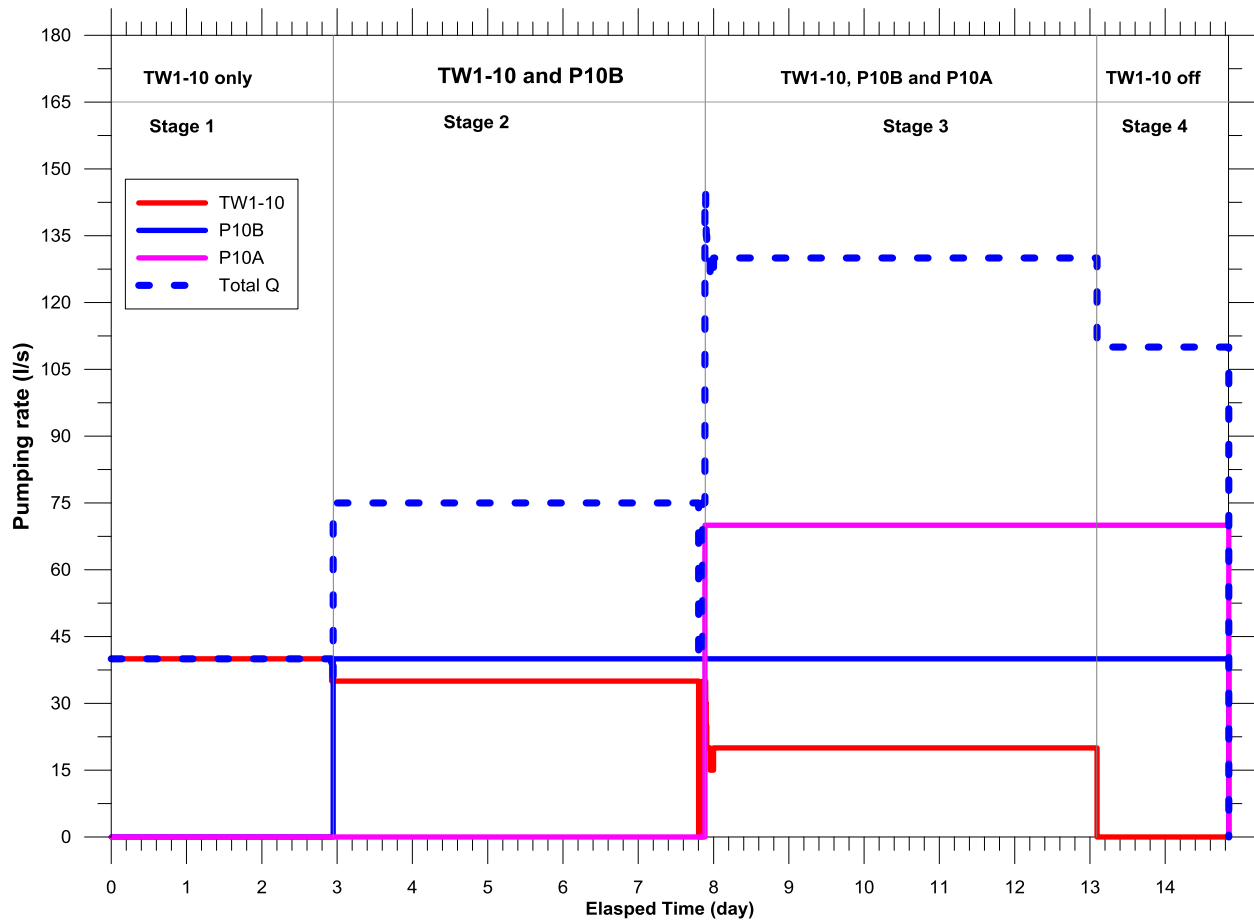


Figure 4. Pumping records for the Pinebush wells during the testing period

4. Interpretation of the response to pumping TW1-10

4.1 TW1-10 step test

A three-step pumping test was conducted at well TW1-10 on August 11, 2011. The steps were conducted at rates of 20 L/s, 30 L/s and 40 L/s. The durations of the first two steps were 30 minutes and the last step lasted 55 minutes. Detailed information on the test and the observed water levels during the step test are included in Stantec (2013; Appendix 1, Figure 19). The data for each step of the test are summarized on Table 2.

Table 2 TW1-10 step test data

Step test #	Pumping rate, Q (L/s)	Drawdown at the end of each step, s_w (m)
1	20	7.83
2	30	12.83
3	40	18.67

A first-cut transmissivity at TW1-10 is estimated from the results of the step test. The transmissivity is estimated from a calculation of the specific capacity with the nonlinear well losses removed. The separation of the linear and nonlinear well losses is accomplished with the Hantush-Bierschenk analysis (Hantush, 1964; Bierschenk, 1964). It is assumed that water levels in the pumping well stabilized by the end of each step.

In the Hantush-Bierschenk analysis it is assumed that the drawdown can be approximated by the Jacob (1947) model:

$$s_w = BQ + CQ^2 \quad (1)$$

Here s_w is the drawdown, Q is the pumping rate, B is the linear well loss coefficient and C is the nonlinear well loss coefficient. Dividing both sides of Equation (1) by Q yields:

$$\frac{s_w}{Q} = B + CQ \quad (2)$$

The values of the specific drawdowns, s_w/Q , are listed on Table 3.

Table 3 TW1-10 step test results

Step test #	Pumping rate, Q (L/s)	Specific drawdown, s_w/Q (m/L/s)
1	20	0.392
2	30	0.428
3	40	0.467

The results of fitting Equation (2) to the observations are plotted in Figure 5. The step test data are approximated closely by a straight line. The specific drawdown at the end of the first interval of the 15-day pumping test, during which only TW1-10 was pumping, is also plotted in Figure 5. The specific drawdown for the constant-rate pumping test is larger than for the step test at the same pumping rate. This reflects the fact that the duration of the step test was about 2 hours, while the duration of the first interval of the constant-rate pumping test was about 3 days.

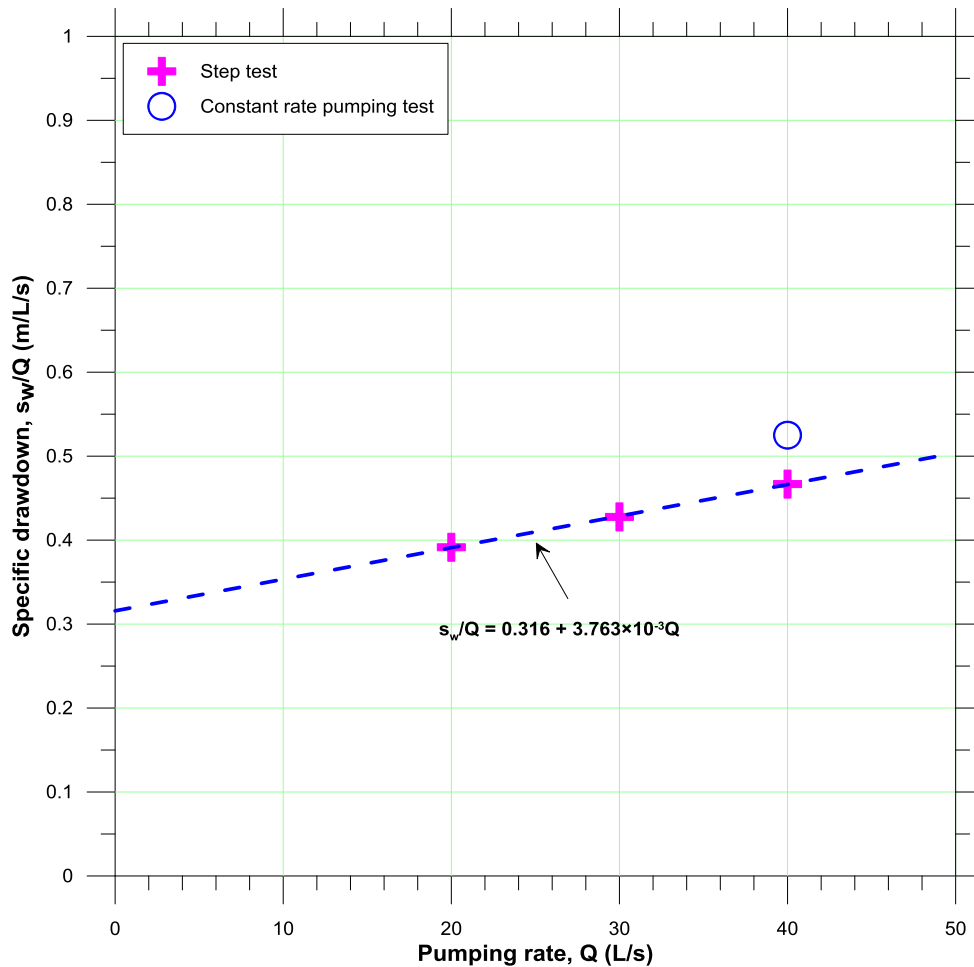


Figure 5. TW1-10 step test results: Hantush-Bierschenk analysis

The estimated linear and nonlinear well loss coefficients are:

- $B = 0.32 \text{ m/L/s}$; and
- $C = 3.8 \times 10^{-3} \text{ m/(L/s)}^2 = 3.8 \times 10^3 \text{ s}^2/\text{m}^5$.

As a check on the fit shown in Figure 5, the pumping rates predicted with Equation (1) for a range of drawdowns are compared with the observations in Figure 6. The observations are matched closely, confirming that the line of best fit developed in Figure 5 is appropriate. For a pumping rate of 40 L/s, the Hantush-Bierschenk analysis yields the following components of the total drawdown in the pumping well:

- Linear component of drawdown: $BQ = 12.6 \text{ m}$
- Nonlinear component of drawdown: $CQ^2 = 6.0 \text{ m}$

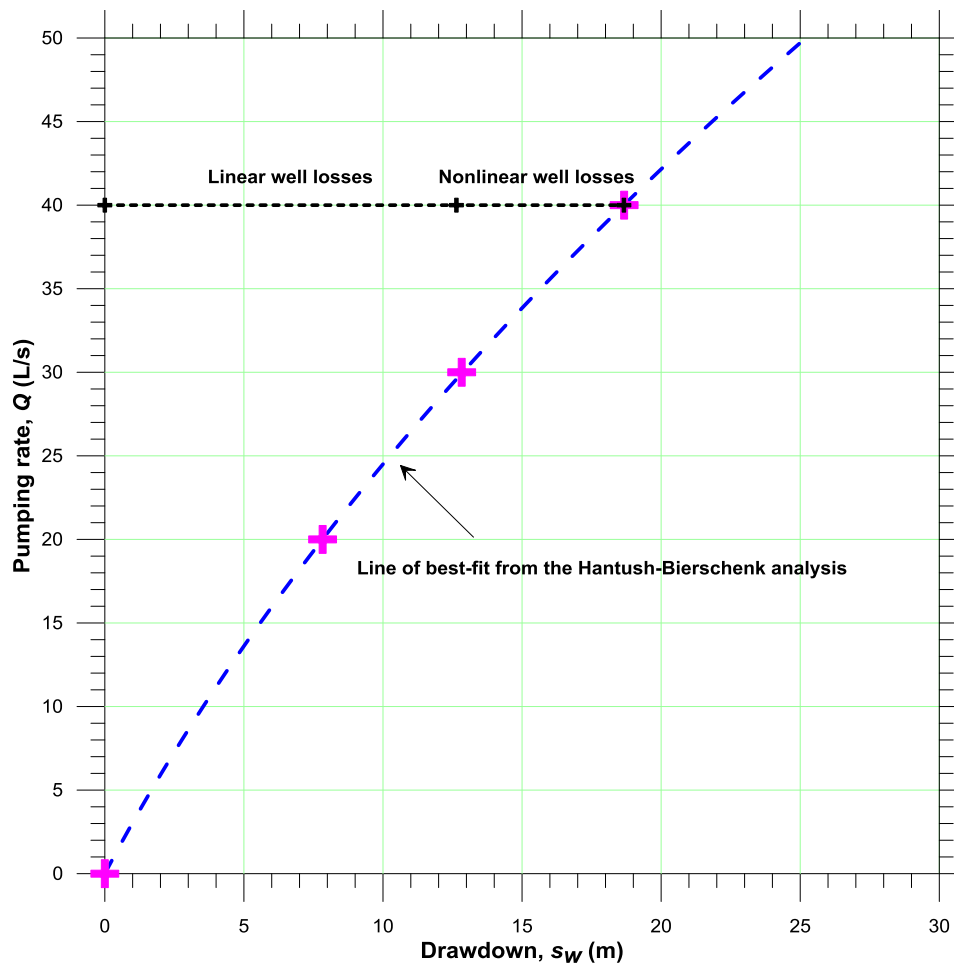


Figure 6. TW1-10 step test results – Check on Hantush-Bierschenk analysis

A first-cut estimate of the transmissivity in the immediate vicinity of TW1-10 is developed from the specific capacity with the nonlinear well losses removed. The adjusted specific capacity of TW1-10 is equal to the reciprocal of the linear well loss coefficient:

$$SC_{lin} = \frac{1}{B} \quad (3)$$

Substituting the fitted value of B into Equation (3) and converting units yields:

$$SC_{lin} = \frac{1}{B} = \frac{1}{0.316 \text{ m/L/s}} \left| \frac{\text{m}^3}{1000 \text{ L}} \right| \left| \frac{86400 \text{ s}}{\text{d}} \right| = 270 \left(\frac{\text{m}^3}{\text{d}} \right) / \text{m}$$

Following the approach of Theis and others (1963), Walton (1970) and Driscoll (1986), the transmissivity is estimated as:

$$T \sim 1.3 \times SC_{lin} \quad (4)$$

The first-cut transmissivity is estimated as:

$$T \sim 1.3 \times 270 \left(\frac{\text{m}^3}{\text{d}} \right) / \text{m} = \mathbf{350 \text{ m}^2/\text{d}}$$

4.2 Constant-rate pumping from TW1-10

During the first 2.9 days of the 15-day test, TW1-10 was pumped at an average rate of 40 L/s. The drawdowns observed at deep bedrock wells TW1-10, P10A, TW1A-09, TW1B-09, PB-TW2-06 and PB-OW1A-10 are assembled on a composite plot in Figure 7. The drawdowns observed at P10A, TW1A-09, TW1B-09 and TW1-10 with the nonlinear well losses removed can be matched with a single parallel line on the semi-log plot, yielding a consistent estimate of the bulk-average transmissivity of **510 m²/d**. The drawdowns from the two more distant observation wells, PB-TW2-06 and PB-OW1A-10, are about 5 m less than would be expected for a homogenous aquifer.

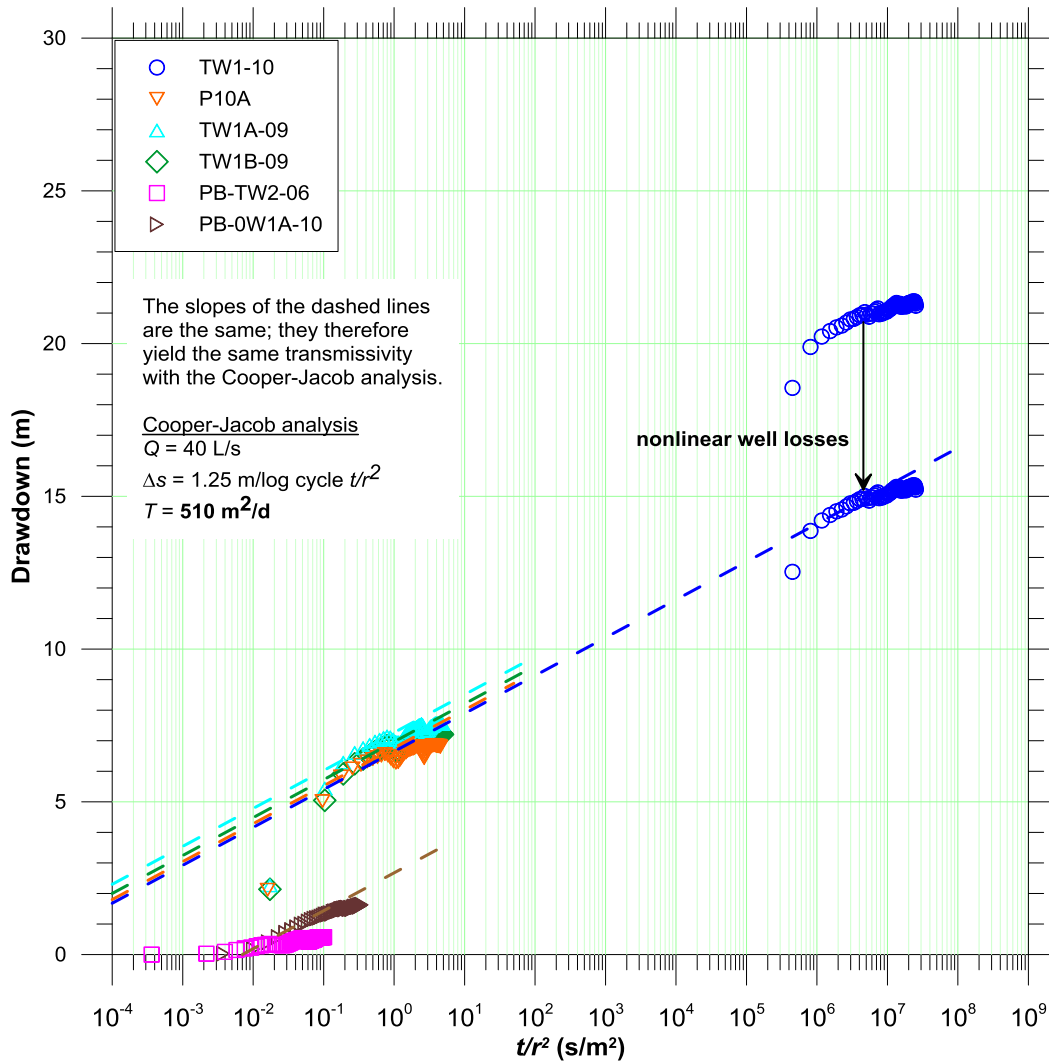


Figure 7. Analysis of the first stress period of the 15-day pumping test

The drawdowns observed at the end of the first 2.9 days of pumping are plotted against the distance from TW1-10 in Figure 8. The data do not appear to approximate a single straight line.

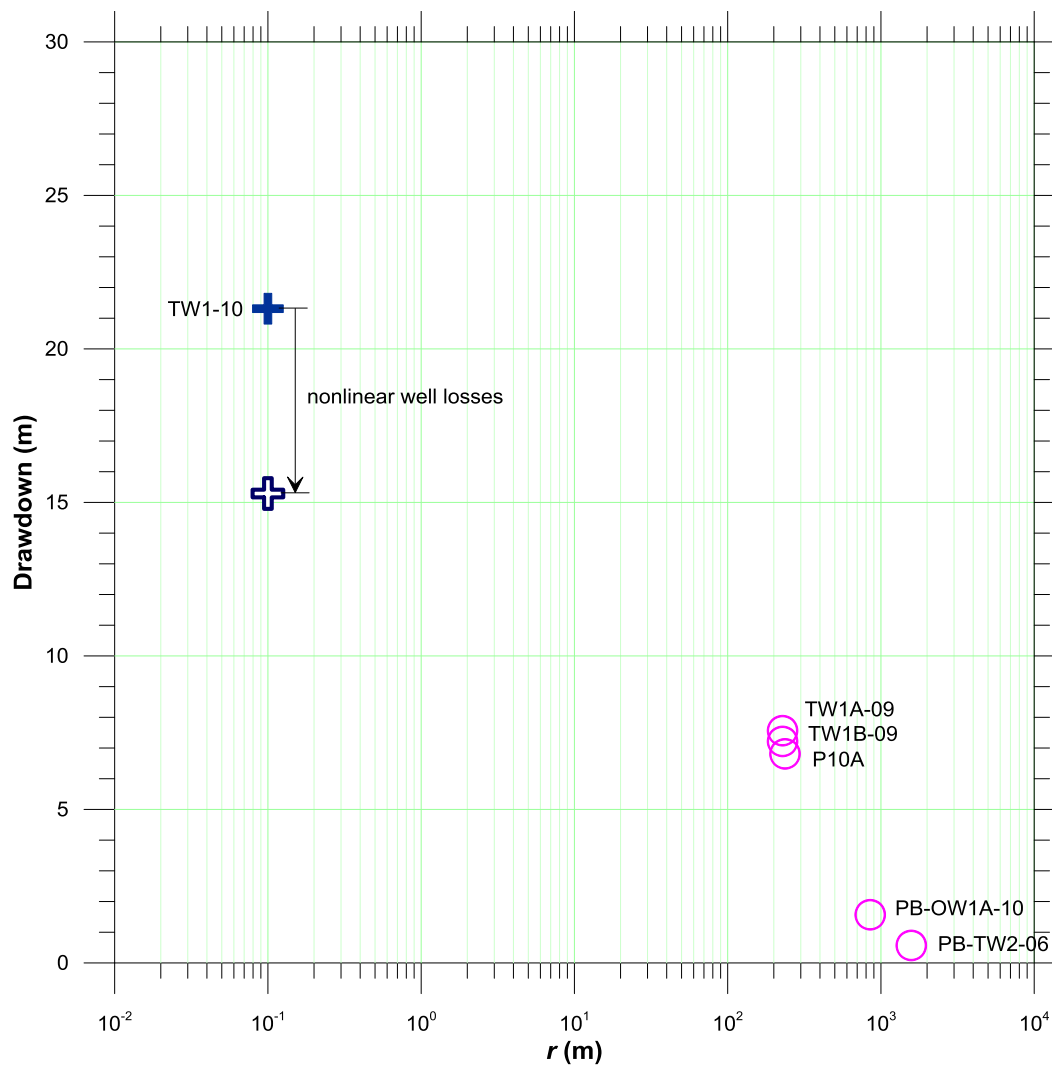


Figure 8. TW1-10 pumping test – Distance-drawdown plot

The distance-drawdown suggests that the pumping well and observation wells P10A, TW1A-09 and TW1B-09 might be located within an “inner” zone and that the two more distant observation wells, PB-TW2-06 and PB-OW1A-10, are located in an “outer” zone of different transmissivity. Cooper-Jacob straight-line analyses shown in Figure 9 yield transmissivities of 530 m²/d and 150 m²/d for the inner and outer zones, respectively.

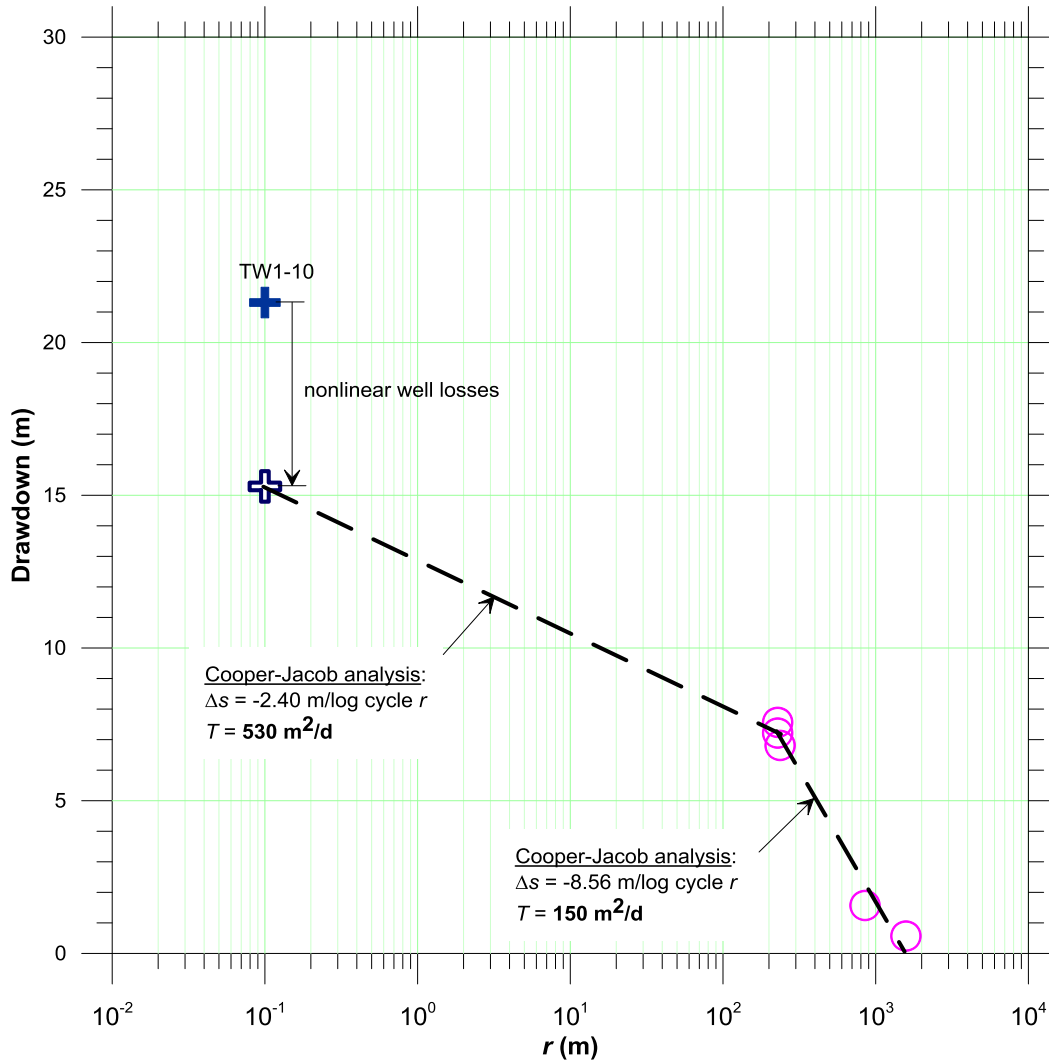


Figure 9. TW1-10 pumping test – Distance-drawdown analysis #1

The locations of the wells in the “inner” and “outer” zones are indicated in Figure 10. As shown in the figure, the inner zone wells are clustered around TW1-10. The “outer” zone wells, PB-TW2-06 and PB-OW1A-10, are located within similar distances from TW1-10, but in opposite directions.

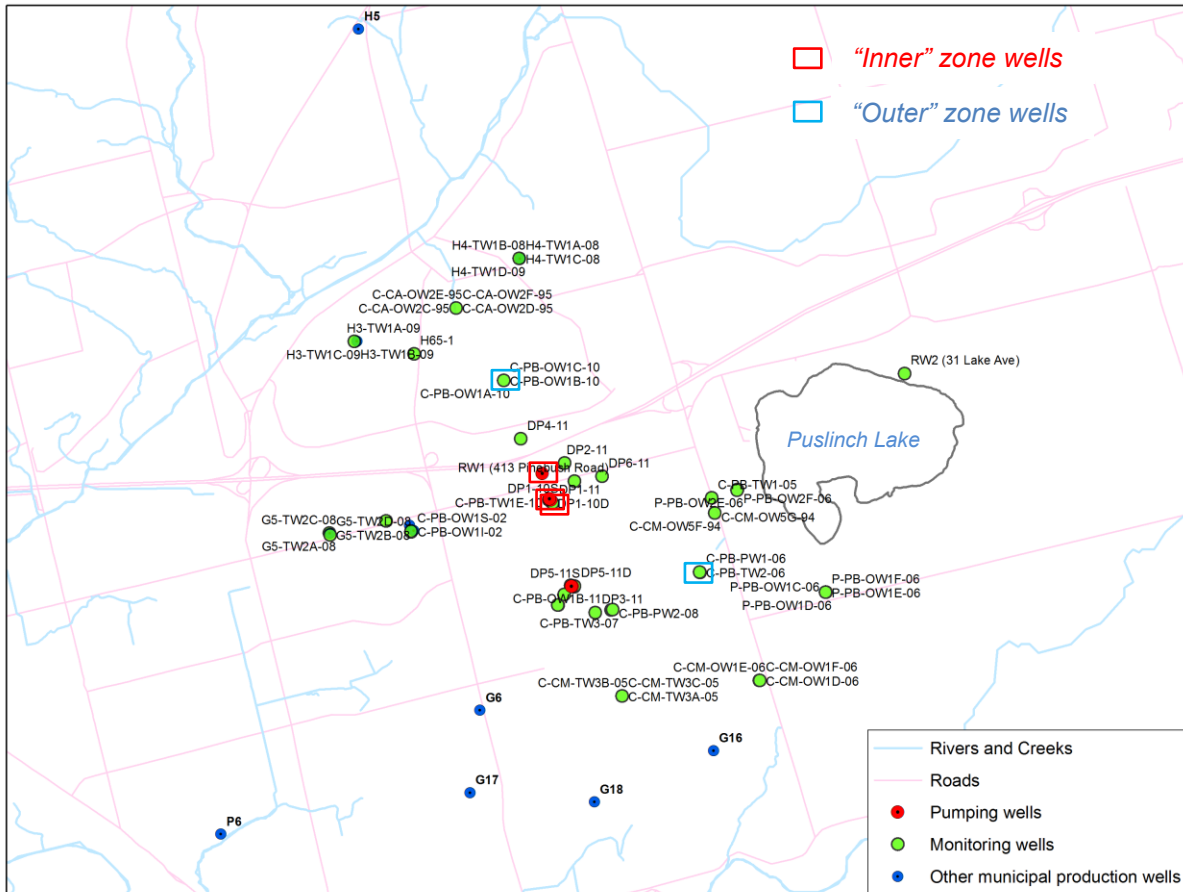


Figure 10. Inferred “inner” and “outer” zone wells

Only those observation wells in the deep bedrock that exhibited the clearest response to pumping have been considered in the preceding analyses. As a check on the interpretation, the responses at all of the deep observation wells monitored during that testing are incorporated in a revised distance-drawdown plot in Figure 11. The interpretation appears to be consistent with the extended set of observations.

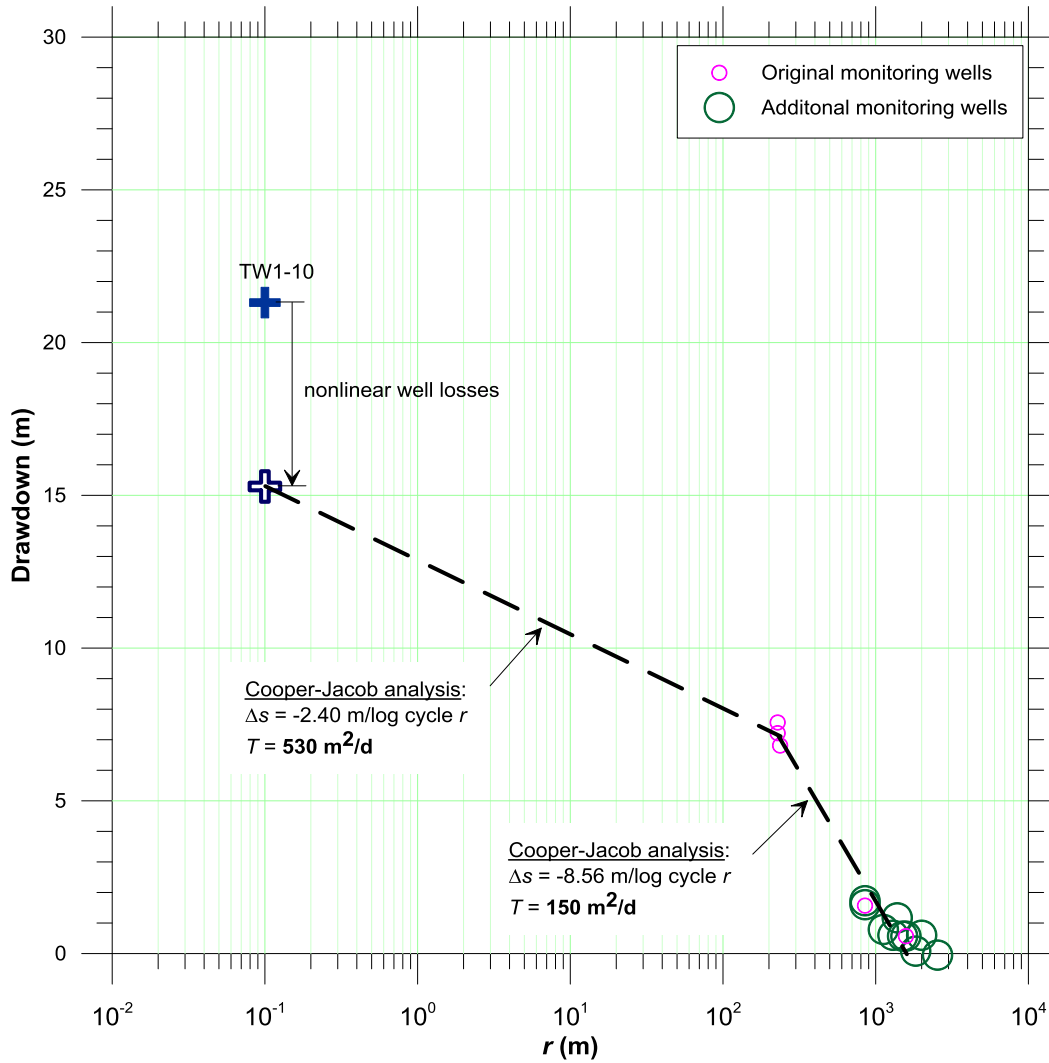


Figure 11. TW1-10 pumping test – Distance-drawdown analysis #2

5. Interpretation of previous testing at P10A

To further assess whether the interpretations of the TW1-10 testing are representative, the data collected during previous testing at P10A are revisited. Aquifer testing was conducted at well P10A in May 2010 (Burnside, 2010). P10A is about 250 m from TW1-10. A step test was conducted at the well, followed by a constant-rate pumping test.

5.1 P10A step test

A three-step pumping test was conducted at well P10A on May 6, 2010. The steps were conducted at rates of 25 L/s, 50 L/s and 75 L/s, with each step lasting 60 minutes. The results of the Hantush-Bierschenk analysis are shown in Figure 12.

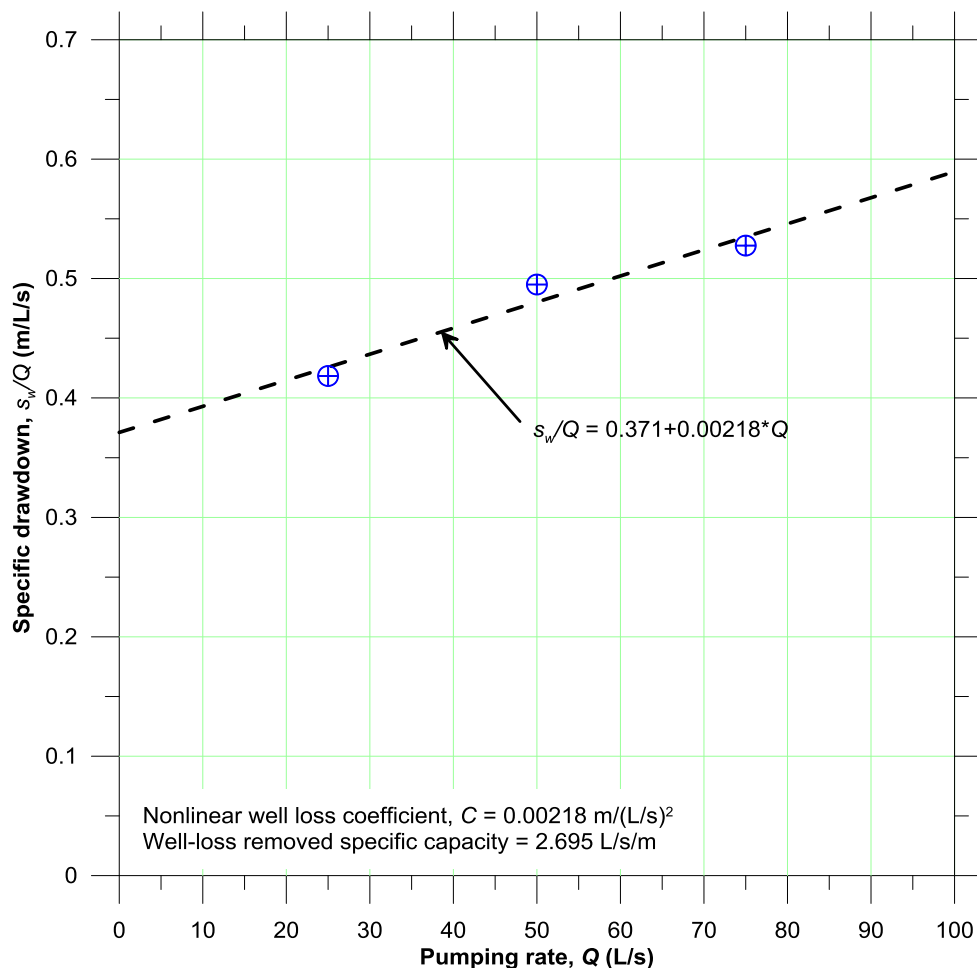


Figure 12. P10A step test, May 2010: Hantush-Bierschenk analysis

The results of the step test are approximated closely by a straight line of the form of Equation (2). The estimated linear and nonlinear well loss coefficients are:

- $B = 0.37 \text{ m/L/s}$; and
- $C = 2.2 \times 10^{-3} \text{ m/(L/s)}^2 = 2.2 \times 10^3 \text{ s}^2/\text{m}^5$.

The linear well loss coefficient, corresponding to the intercept of the best-fit line on the Hantush-Bierschenk plot, is 0.371 m/L/s . The specific capacity with the nonlinear well losses removed is the reciprocal of this value. Converting units yields:

$$SC_{lin} = \frac{1}{B} = \frac{1}{0.371 \text{ m/L/s}} \left| \frac{\text{m}^3}{1000 \text{ L}} \right| \left| \frac{86400 \text{ s}}{\text{d}} \right| = 230 \frac{\text{m}^3/\text{d}}{\text{m}}$$

Therefore, the first-cut transmissivity based on Equation (4) is estimated as:

$$T \sim 1.3 \times SC_{lin} = 1.3 \times 230 \left(\frac{\text{m}^3}{\text{d}} \right) / \text{m} = \mathbf{300 \text{ m}^2/\text{d}}$$

Estimation of the linear and nonlinear sources of drawdown in P10A

The predicted pumping rates for a range of drawdowns in P10A are shown in Figure 13. For a pumping rate of 70 L/s , the linear and nonlinear components of the total drawdown in P10A are estimated as:

- Linear component of drawdown: $BQ = 26.0 \text{ m}$; and
- Nonlinear component of drawdown: $CQ^2 = 10.7 \text{ m}$.

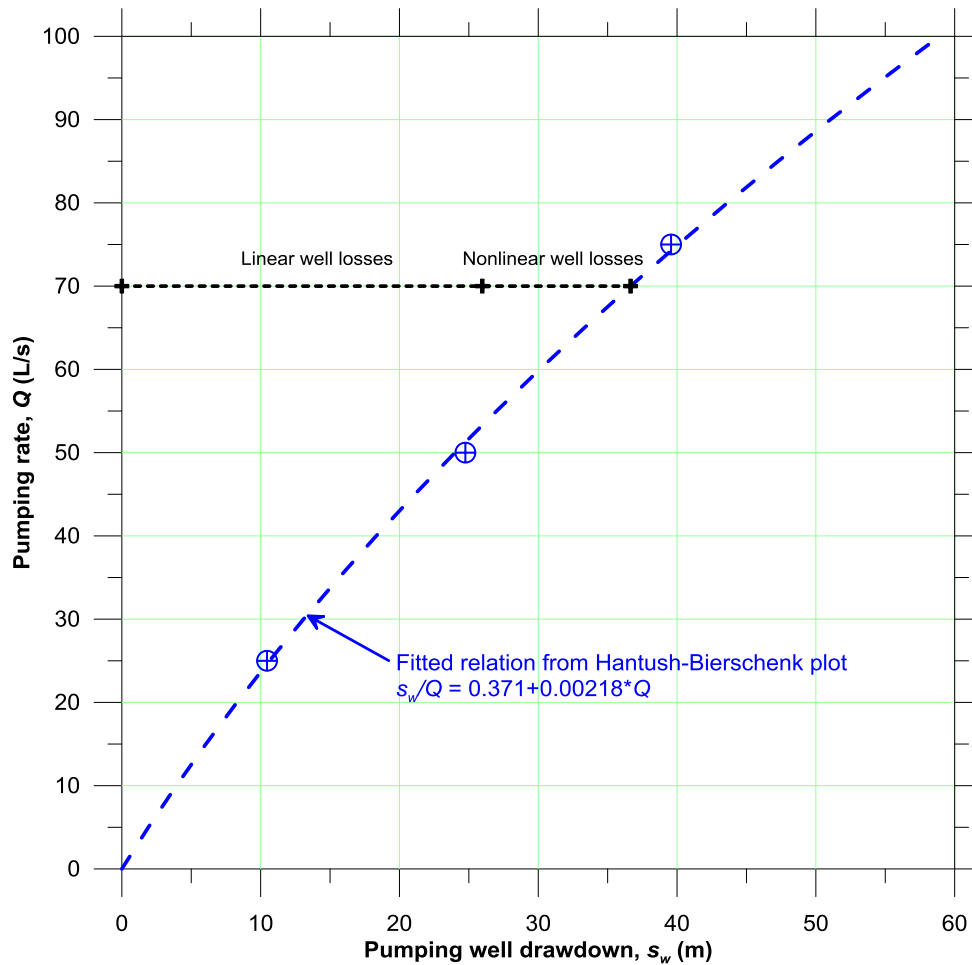


Figure 13. P10A – Check on Hantush-Bierschenk analysis

5.2 P10A constant-rate pumping test

A 5-day constant-rate pumping test was conducted at P10A on May 8, 2010. P10A was pumped at an average rate of 70 L/s.

A composite plot of the drawdowns observed at wells P10A, P10-TW1A-09 and P10-TW1B-09 is shown in Figure 14. The data from these wells can be matched by parallel lines on the semi-log composite plot, which indicates that an internally consistent, large-scale estimate of the transmissivity can be inferred from the data. The bulk-average transmissivity estimated with the Cooper-Jacob straight line analysis is about 160 m²/d.

The drawdowns for P10A plotted in Figure 14 appear to be shifted to the right with respect to the observation wells. This implies that the drawdowns observed at the pumping well are significantly smaller than would be expected for a homogeneous aquifer. The response to pumping suggests that P10A is surrounded by a zone that has an elevated transmissivity relative to the bulk properties of the deep bedrock in this area.

The inference of a zone of elevated transmissivity surrounding P10A is consistent with the other data available for this well. The results of the borehole flowmeter profiling of P10A are reproduced in Figure 15. Between depths of 70 m and 80 m, there is a major discontinuity in the flowmeter profiles, suggestive of a large cavern. It is possible that the transmissivity of this feature could be very much larger than 160 m²/d; however, it is important to bear in mind that the transmissivity inferred from the pumping test is representative of the bulk-average properties of the formation outside of this feature.

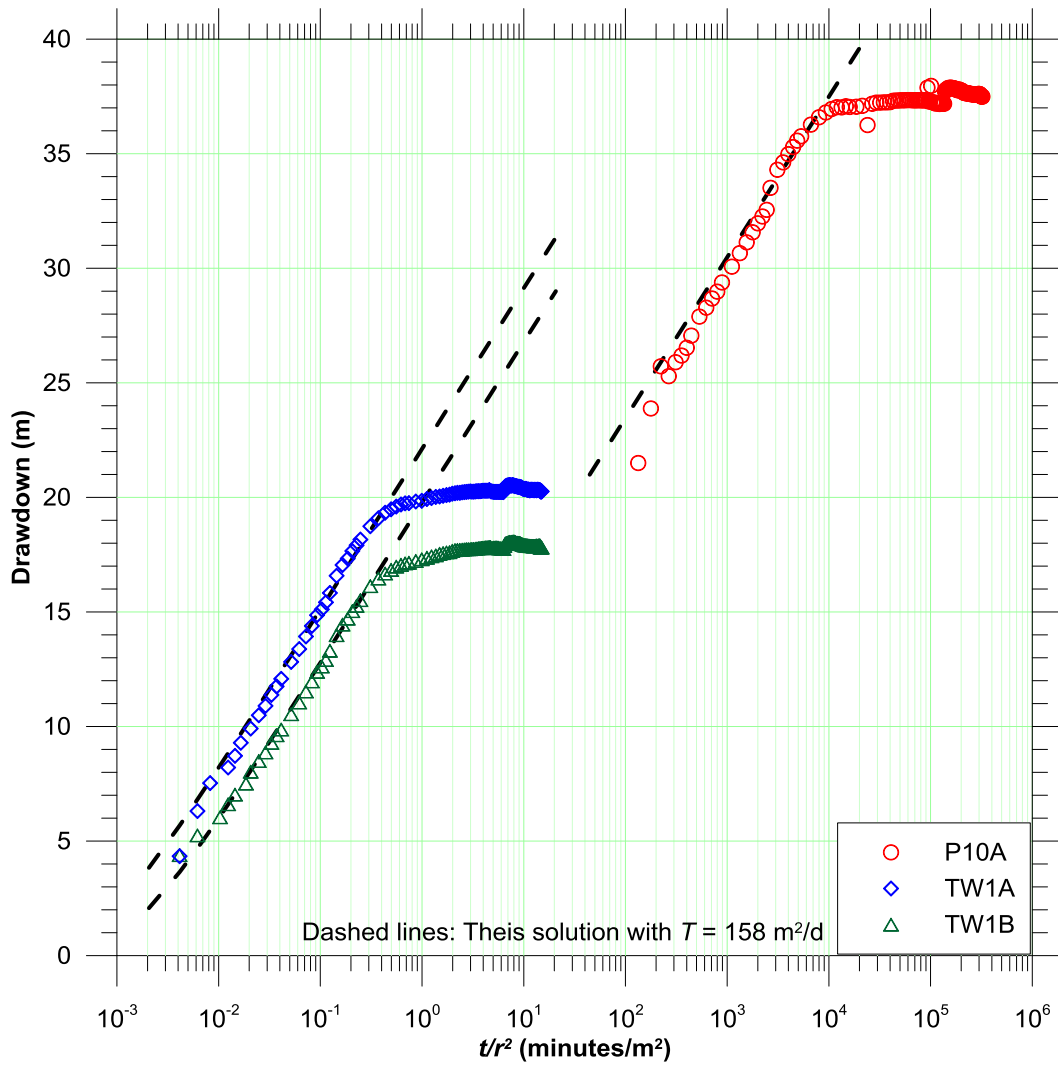


Figure 14. P10A constant-rate pumping test, May 2010

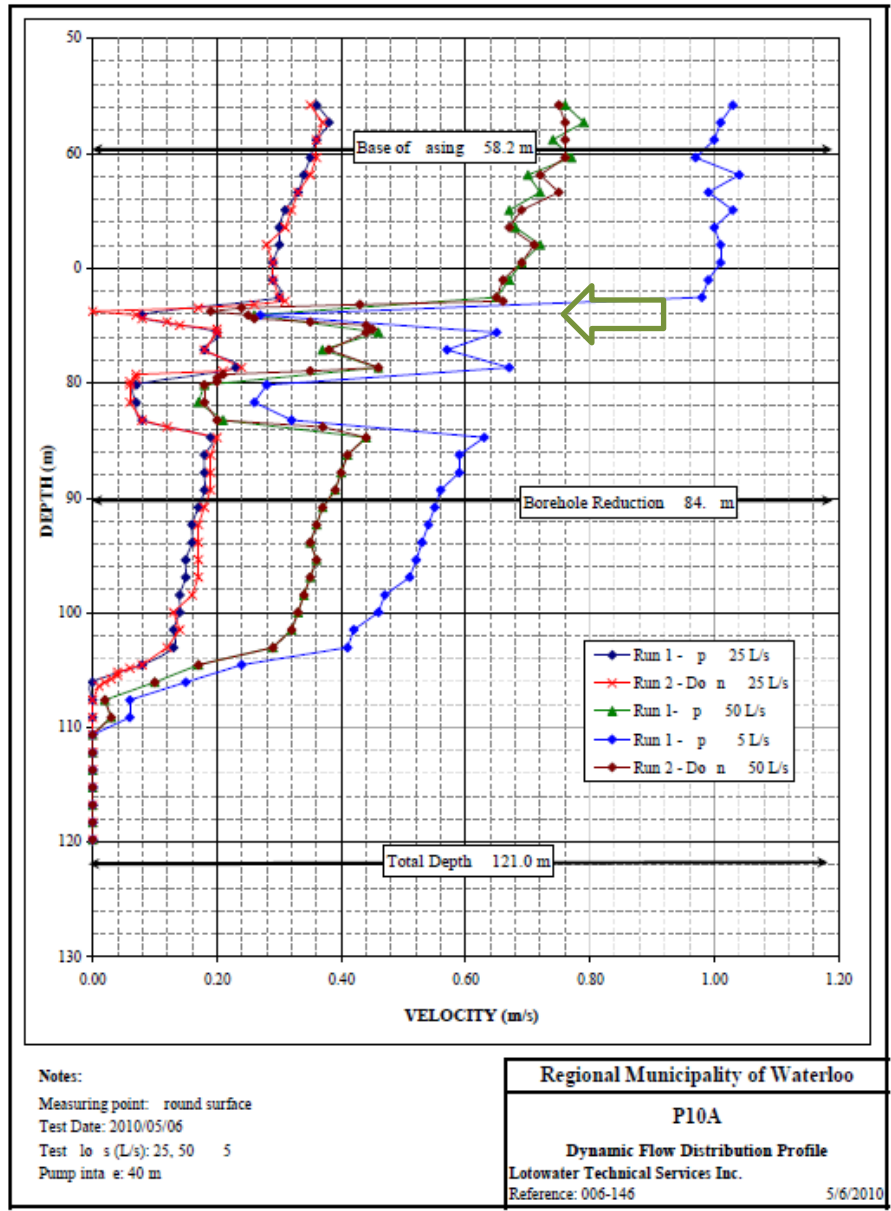


Figure 15. Borehole flowmeter profiles for P10A

6. Summary of the transmissivity estimates in the vicinity of TW1-10 and P10A

The following estimates of bulk-average transmissivity have been obtained.

Pumping TW1-10

- $T \sim 520 \text{ m}^2/\text{d}$ (“inner” zone)
- $T \sim 150 \text{ m}^2/\text{d}$ (“outer” zone)

The results of the TW1-10 constant-rate test suggested that the “inner” zone extends about 200 m from TW1-10.

Pumping P10A

- $T \sim 160 \text{ m}^2/\text{d}$

The transmissivity of $160 \text{ m}^2/\text{d}$ estimated from the P10A test is similar to the estimate obtained for the “outer” zone around TW1-10. Well P10A is located about 240 m from TW1-10. The analyses suggest that P10A is located close to the transition between the TW1-10 “inner” and “outer” transmissivity zones.

7. References

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Appendix F

Model Sensitivity Analysis

Cambridge East Groundwater Model Sensitivity Analysis

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Last update: July 11, 2014

1 Introduction

To assist in assessing the most important aspects of the Cambridge East groundwater model, the computer assisted calibration code, PEST, was used to conduct a formal sensitivity analysis. The results of the sensitivity analysis are reported in terms of the relative change in the measure of the overall goodness of fit (the sum of the squared residuals, referred to as the objective function) versus changes in the hydraulic conductivity values from those inferred during calibration. The relative changes are referred to as the “sensitivities”; hydraulic conductivities with the largest sensitivities are those that have the most impact on the overall match to the calibration targets.

The analysis was conducted by assigning NOPTMAX a value of -1 in the PEST control file. With a NOPTMAX value of -1, PEST terminates immediately after it has calculated the Jacobian matrix for the first time. The parameter covariance, correlation coefficient and eigenvector matrices are written to the run recorder file, and parameter sensitivities are written to the sensitivity file. The calibrated hydraulic conductivity values for the parameter zones within the Cambridge East area were specified as the baseline hydraulic conductivity values for the PEST analysis. By doing so, the sensitivities of the hydraulic conductivities calculated by PEST are based on the calibrated model. The sensitivities calculated by PEST are the composite sensitivities, s_j defined as:

$$s_j = \frac{(J^t Q J)_{jj}^{1/2}}{m}$$

Here m corresponds to the number of groundwater level calibration targets, j denotes the j^{th} parameter (the value of hydraulic conductivity specified for each zone) and J is the Jacobian matrix. Q is the ‘cofactor matrix’, an $m \times m$ diagonal matrix whose elements are the squared observation weights, with. Relative weights of 1.0 have been assigned for the high-quality steady-state calibration targets, and weights of 0.1 have been assigned for the targets derived from the water well records and production wells.

2 Results

The calculated composite sensitivities of the overall calibration with respect to changes in the calibrated hydraulic conductivity values are presented in **Figure 1**. The red bar represents the sensitivity of the horizontal hydraulic conductivity of the parameter zone, while the blue bar represents the sensitivity of the vertical hydraulic conductivity. Details on the most sensitive horizontal and vertical hydraulic conductivities are summarized on **Table 1** and **Table 2**, respectively. The locations of the most sensitive hydraulic conductivity zones are presented in **Figures 2 to 6**. Special attention is focused on the parameter zones with composite sensitivities more than 400 for the horizontal hydraulic conductivity, and 50,000 for the vertical hydraulic conductivity. These criteria are based on inspection of the distribution of sensitivities shown in **Figure 1**.

Efforts have been directed towards understanding why particular zones have the greatest impact on the calibration. These efforts have assisted in identifying areas for additional review of the conceptual model and refined calibration, and in identifying areas of the model where subsequent adjustments would not yield any significant improvement in the match to the groundwater level targets.

2.1 Overburden units (OB)

The PEST results show that the match to the calibration targets for 2003 average conditions are most sensitive to the hydraulic conductivity values of five parameter zones within the Overburden layers. The composite sensitivities of these zones and their hydraulic conductivity values are listed on **Table 1** and **Table 2**. The locations of the sensitive parameter zones in the overburden are presented in **Figures 2a, b, c** and discussed below.

Zone 10101340

As shown in **Figure 2a**, zone 10101340 is a strip that follows Mill Creek. The match to the calibration targets is affected only by the horizontal hydraulic conductivity of this zone. A low hydraulic conductivity value has been inferred for this zone during calibration, limiting the interaction between the groundwater system and Mill Creek.

Zone 10101341

Zone 10101341, shown in **Figure 2a**, is a small zone west of Puslinch Lake that consists of Port Stanley Till. The match to the calibration targets is sensitive to the horizontal hydraulic conductivity of this zone. A relatively low value of hydraulic conductivity is required in the model to limit the leakage from Puslinch Lake towards the west.

Zone 10202170

Zone 10202170 is shown in **Figure 2a** and represents the materials at the bottom of Puslinch Lake. The match to the calibration targets is sensitive to both the horizontal and vertical hydraulic conductivity of this zone. This parameter zone controls the leakage of Puslinch Lake to the groundwater system. The hydraulic conductivity of this zone has been critical for matching the observed lake level.

Zone 10104060

The extents of zone 10104060 are shown in **Figure 2b**. The zone represents the materials of the Port Stanley Till. The model results were sensitive to both the horizontal and vertical hydraulic conductivity of the zone. The Port Stanley Till is one of the main aquitards in the overburden that separates the overburden aquifers.

Zone 10306180

The limits of zone 10306180 are shown in **Figure 2c**. The zone includes sediments that are labeled Maryhill Till equivalents and Catfish Creek Till. It is another major aquitard in the overburden. The hydraulic conductivity of this zone has been found to exert an important influence on the differences in groundwater levels between the overburden units.

2.2 Bedrock units

The hydraulic conductivity zones of the bedrock units that have the greatest effect on the match to the calibration targets are primarily the Reformatory Quarry Member and the Vinemount Member of the Eramosa Formation. In general, the match to the targets is most sensitive to the vertical hydraulic conductivity of these units. Details of the sensitive parameter zones in the bedrock units are presented on **Table 1** and **Table 2**, and the locations of these zones are presented in **Figures 3 to 6**.

2.2.1 Reformatory Quarry Member (RQ)

Zone 41010040

The limits of zone 41010040 are shown in **Figure 3**. This zone includes the Hespeler and Shades Mills Well Fields. Mapping of estimates of transmissivity developed from the water well records suggested that these two areas had low to intermediate transmissivities. At the start of the calibration, these areas were lumped as one parameter zone along with the area in the center of Cambridge East. During the calibration of the 28-day pumping test, a separate zone for the center of the Cambridge East area was delineated to help achieve a better match to the observations from the pumping test.

Zone 41010060

As shown in **Figure 3**, this parameter zone is located at the southwest corner of the Cambridge East zone. The match to the calibration targets is more sensitive to the vertical hydraulic conductivity than to the horizontal hydraulic conductivity of this zone. The area covers the majority of the southwest of the model domain. Some multilevel monitoring data in this area, for example from OW5-95 and OW6-95, show large water level differences between the shallow and the deep bedrock aquifers in this area. The vertical hydraulic conductivity exerts the dominant control on these differences.

Zone 41010070

Parameter zone 41010070 is located in the northeastern portion of the Cambridge East area (**Figure 3**). Downward gradients from the shallow bedrock aquifer to the deep bedrock aquifer are inferred for this area. The match to the calibration targets is sensitive to the vertical hydraulic conductivity of this zone.

2.3 Vinemount Member (VM)

The Vinemount Member is the main aquitard that separates the shallow bedrock aquifers (the Guelph Formation and Reformatory Quarry Member of the Eramosa Formation) from the deep bedrock aquifers (the Goat Island and the Gasport Formation). The PEST results showed that the match to the calibration targets is sensitive to all the parameter zones of the Vinemount Member within the Cambridge East area. The large composite sensitivities of the hydraulic conductivity parameters representing the Vinemount Member indicate the importance of this unit.

Zone 51111010

As shown in **Figure 4**, this parameter zone extends across northeast of the Cambridge East zone. The match to the calibration targets is most sensitive to the vertical hydraulic conductivity of this zone. Golder Associates (2012) report a strong downward gradient between the shallow bedrock aquifer and the deep bedrock aquifer in this area. A low vertical hydraulic conductivity is inferred for this zone, to support the difference in water levels between the shallow and deep aquifers.

Zone 51111030

Parameter zone 51111030 covers the eastern portion of the model domain (**Figure 4**). The match to the calibration targets is sensitive to the vertical hydraulic conductivity of this zone. Golder Associates (2012) report a strong upward gradient in the Shades Mills area. The low vertical hydraulic conductivity inferred for this zone is necessary to maintain the water level difference between the shallow and deep bedrock aquifers.

Zone 51111040

As seen in **Figure 4**, zone 51111040 is located at the center of the Cambridge East area. This zone has been delineated during calibration at the 28-day pumping test. The match to the calibration targets is sensitive to the vertical hydraulic conductivity. The low vertical hydraulic conductivity limits the propagation of the effects of pumping in the Gasport Formation to the shallow bedrock aquifer.

Zone 51111050

Parameter zone 51111050, shown in **Figure 4**, is a north-south direction strip that represents a transition zone marking the limits of the Vinemount Member. The thickness of the Vinemount Member within this zone is less than 1 m. The vertical hydraulic conductivity of this zone has been increased to reflect its thinness.

2.4 Gasport Formation (GS)

Only two parameter zones of the Gasport Formation, 71313090 and 81414090, have composite sensitivities higher than 400 for the horizontal hydraulic conductivity. They cover the same western portion of the model in the Upper Gasport Formation and the Middle Gasport Formation, shown in **Figures 5 and 6**. Data are relatively sparse to constrain the hydraulic conductivity value of these zones. However, the thickness of the Gasport Formation decreases sharply toward the west.

Table 1 Horizontal conductivity zones with composite sensitivities greater than 400

Zone ID	Description	Horizontal Hydraulic Conductivity (m/s)	Vertical Hydraulic Conductivity (m/s)	Composite Sensitivity
10202170	Bottom materials of Puslinch Lake (Layer 2)	10^{-8}	10^{-10}	19860
51111050	Vinemount Member of Eramosa Formation, thickness is less than 1 m	10^{-7}	10^{-8}	1987
10101341	Port Stanley Till at the west of Puslinch Lake (Layer 1)	10^{-7}	10^{-8}	1666
51111010	Vinemount Member of Eramosa Formation, north portion of the model domain	10^{-7}	10^{-9}	1469
51111040	Vinemount Member of Eramosa Formation, center of the Cambridge East area	10^{-7}	10^{-9}	1418
41010060	Reformatory Quarry Member of Eramosa Formation, west and southwest of the model domain	10^{-6}	5×10^{-9}	1415
10104060	Port Stanley Till (Layer 4)	2×10^{-7}	2×10^{-8}	945
41010040	Reformatory Quarry Member of Eramosa Formation, transition zone at Hespeler well field and Shades Mills well field	1.87×10^{-7}	9×10^{-10}	789
71313090	Upper Gasport Formation, west portion of the model domain, large area with little information	4.2×10^{-6}	4.2×10^{-7}	750
10101340	Hydraulic conductivity zone along Mill Creek (Layers 1 to 4).	10^{-6}	10^{-7}	724
81414090	Middle Gasport Formation, west portion of the model domain, large area with little information	2×10^{-7}	10^{-9}	610
10306180	Maryhill Till and equivalent, Catfish Creek Till (Layers 3 to 6)	5×10^{-7}	5×10^{-8}	499
51111030	Vinemount Member of Eramosa Formation, east central portion of the model domain	1.5×10^{-7}	1.5×10^{-9}	416

Table 2 Vertical conductivity zones with composite sensitivities greater than 50,000

Zone ID	Description	Horizontal Hydraulic Conductivity (m/s)	Vertical Hydraulic Conductivity (m/s)	Composite sensitivity
51111010	Vinemount Member of Eramosa Formation, north portion of the model domain	10^{-7}	10^{-9}	1815757
41010060	Reformatory Quarry Member of Eramosa Formation, west and southwest of the model domain	10^{-6}	5×10^{-9}	1121633
10202170	Bottom materials of Puslinch Lake (Layer 2)	10^{-8}	10^{-10}	679415
51111030	Vinemount Member of Eramosa Formation, east central portion of the model domain	1.5×10^{-7}	1.5×10^{-9}	599437
51111040	Vinemount Member of Eramosa Formation, center of the Cambridge East area	10^{-7}	10^{-9}	483415
41010040	Reformatory Quarry Member of Eramosa Formation, transition zone at Hespeler well field and Shades Mills well field	1.87×10^{-7}	9×10^{-10}	355445
10306180	Maryhill Till and equivalent, Catfish Creek Till (Layers 3 to 6)	5×10^{-7}	5×10^{-8}	100265
10104060	Port Stanley Till (Layer 4)	2×10^{-7}	2×10^{-8}	90218
51111050	Vinemount Member of Eramosa Formation, thickness is less than 1 m	10^{-7}	10^{-8}	65040
41010070	Reformatory Quarry Member of Eramosa Formation, thin zone, in the northeast of the model domain, close to the model boundary	2×10^{-6}	1×10^{-8}	55568

3 Sensitivity Analyses for the Transient Simulations

For the calibration of the Cambridge East model, hydraulic conductivity estimates have been constrained relatively tightly by considering multiple calibration scenarios. The parameters about which the least data are available are the storage coefficients, in particular the specific storage. A key scenario for the model calibration is the high-quality 28-day pumping test conducted between September 5 and October 2, 2006. The storage coefficients for the transient simulations have been assigned values based on guidance in the hydrogeologic literature. Additional sensitivity analyses have been conducted to assess the significance of the storage parameters with respect to match to the transient calibration targets.

A specific storage (S_s) of 10^{-6} /m has been specified for the bedrock units of the model during the simulations of the 28-day pumping test. This is a "textbook" value (Lohman, 1972; p. 8; Boonstra, 1989; p. 17). To examine the significance of the specific storage for the bedrock layers, the following cases have been considered:

- Base Case: $S_s = 10^{-6}$ /m;
- Case 1: $S_s = 10^{-5}$ /m; and
- Case 2: $S_s = 10^{-7}$ /m.

Scatterplots of the drawdowns calculated after 28 days for the three cases are presented in **Figures 7 to 9**. The results suggest that a specific storage of 10^{-5} /m yields a slightly better match to the observations. There are no obvious differences between the final drawdowns calculated for the Base Case and for Case 2. The match to the final drawdown is not the only criterion that has been considered when assessing the results of the transient calibrations. Three observation wells, CMOW1-06, CMOW2-06 and PBOW2-06, have been selected to compare the entire transient records over the duration of the test. These wells are in the immediate vicinity of the pumping wells, where the observed response to pumping is greatest. The simulated and observed hydrographs for the "A" monitoring intervals (Gasport Formation) at CMOW1-06, CMOW2-06 and PBOW2-06 are presented in **Figures 10 to 12**. The hydrographs suggest that better matches are achieved for the Base Case. For Case 1, although the final simulated drawdowns are closer to the final observed drawdowns, the results of the simulation do not match the full records as well as for the Base Case and Case 2.

The observations suggest that the water levels at the "A" monitoring intervals are nearly stable at the end of each pumping stage. In contrast, for Case 1 the simulated water levels are still declining at the end of each pumping stage. The results for both the Base Case and Case 2 reproduce the near-stabilization by the end of each pumping stage. The results for Case 2 suggest that when a specific storage of 10^{-7} /m is specified, the water levels in the observation wells reach their final levels almost immediately after the start of each new pumping stage. The observations suggest that this is too sudden.

Observed and simulated hydrographs for monitoring wells in the Guelph Formation and the overlying overburden are shown in **Figure 13** through **Figure 18**. The "B" monitoring intervals are open across the Guelph Formation and the "D" monitoring intervals are screened in the overburden. The "C" intervals are not discussed, as they are also open across the Guelph Formation.

The hydrographs for CMOW1-06, CMOW2-06 and PBOW2-06 are shown in **Figures 13 to 15**. At CMOW1B-06, all three analyses overestimate the final drawdowns. However, the results from both the Base Case and Case 2 closely match the drawdown of the second stage of pumping (pumping by PBPW1-06), and stable groundwater levels are predicted at the end of each stage. The drawdowns simulated for Case 1 show continuing declines in water levels at the end of each pumping stage. The simulated drawdowns for PBOW2B-06 with the Base Case and Case 2 match the observed drawdowns more closely than do the results for Case 1.

The simulated and observed hydrographs for the "D" monitoring intervals in overburden sediments at CMOW1-06, CMOW2-06 and PBOW2-06 are shown in **Figures 16 to 18**. At all three wells, the results for the Base Case and Case 2 match the observed drawdowns better than do the results for Case 1.

The final simulated drawdowns in the Contact Aquifer and the Middle Gasport Formation are shown for all three cases in **Figures 19 to 24**. The final simulated drawdowns are similar for specific storages of $10^{-6}/m$ and $10^{-7}/m$. The differences between the simulations are more obvious for specific storages of $10^{-6}/m$ and $10^{-5}/m$.











It is important to note that the pumping during the 28-day Cambridge East test was from the deeper bedrock units. Therefore, the specific storage is more significant with respect to the response in the bedrock aquifers than the overburden. It is also important to note that in the long term, the transient responses to pumping stabilize and the results do not depend on the storage coefficients.

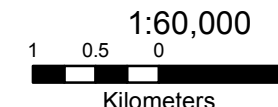
The key finding from the sensitivity analysis are summarized below.

- The assumed specific storage value of $10^{-6}/m$ for the bedrock layers is defensible. The value of $10^{-6}/m$ is consistent with guidance in the literature, and yields an acceptable match to the observed trends in the drawdowns. Varying the specific storage to $10^{-7}/m$ has little effect on the calibration results; however, varying the specific storage to $10^{-5}/m$ yields larger differences in calculated drawdowns and produces a closer match to the final observed drawdowns. The match to the final drawdowns is improved when the specific storage is increased to $10^{-5}/m$; however, the match to the complete drawdown record deteriorates compared to the results when specific storage is assigned a value of $10^{-6}/m$.
- For the 28-day pumping test, the drawdowns in the deep bedrock aquifer (Gasport Formation) are more sensitive to the specific storage than the shallow bedrock aquifers. This reflects the fact that the main water production zones of the deep bedrock pumping wells are within the Gasport Formation.

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

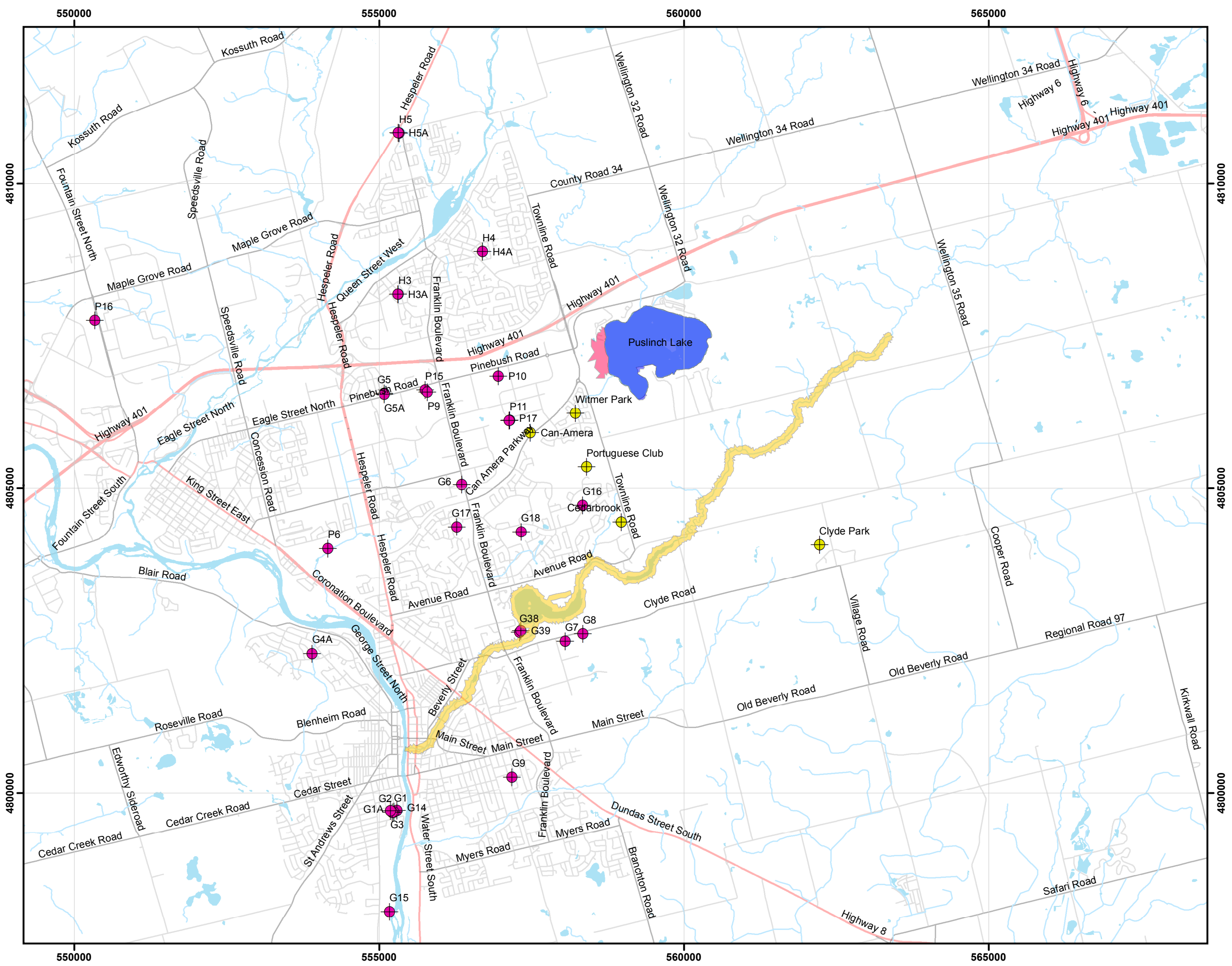
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 -  Test Production Well
 -  Expressway / Highway
 -  Major Roads
 -  Roads (collectors)
 -  Rivers / Streams
 -  Lakes and Ponds
- Hydraulic conductivity zone**
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 -  10202170



Projection: UTM Zone 17N, NAD 83
Map Version: 2, Map data 2014-05-27 J. Zhang











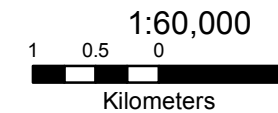
Figure 2a
Locations of Sensitive Hydraulic Conductivity Zones in Overburden — Map 1



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

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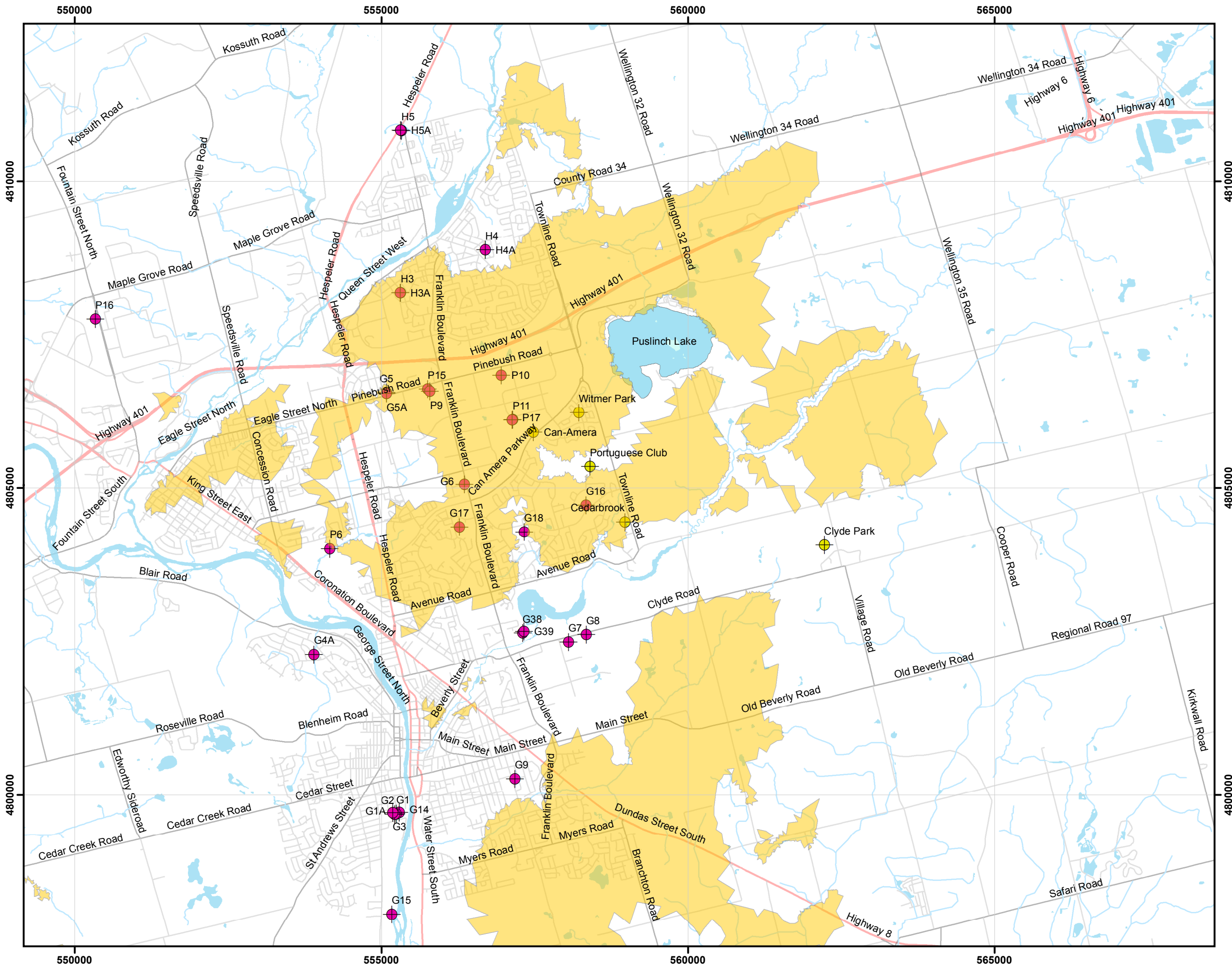
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-  Lakes and Ponds
- Hydraulic conductivity zone
-  10104060



Projection: UTM Zone 17N, NAD 83
Map Version: 2, Map data 2014-05-27 J. Zhang



Figure 2b
Locations of Sensitive Hydraulic Conductivity Zones in Overburden — Map 2



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

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- Test Production Well
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- Major Roads
- Roads (collectors)
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- Lakes and Ponds
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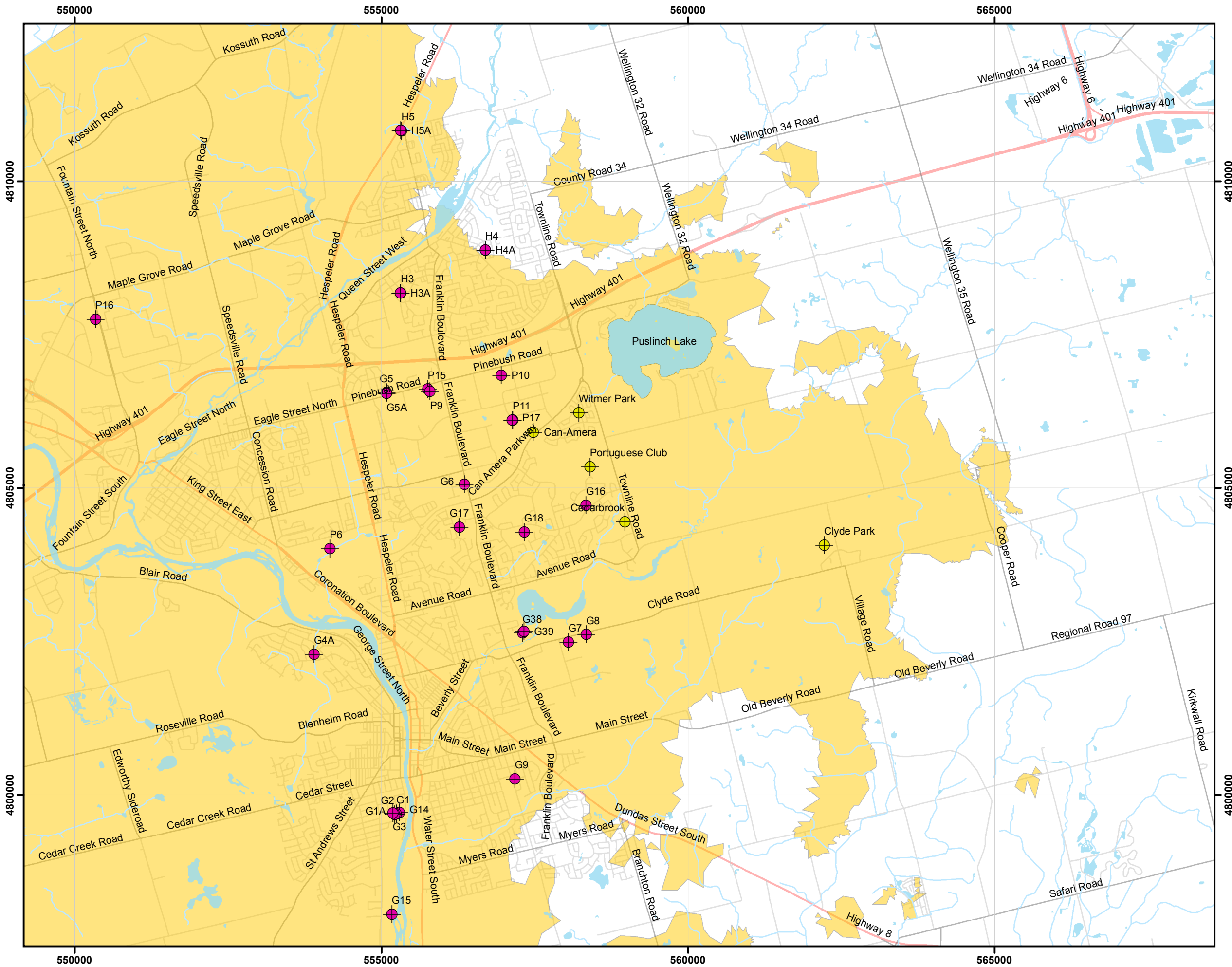
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Figure 2c
 Locations of Sensitive Hydraulic Conductivity Zones in Overburden — Map 3

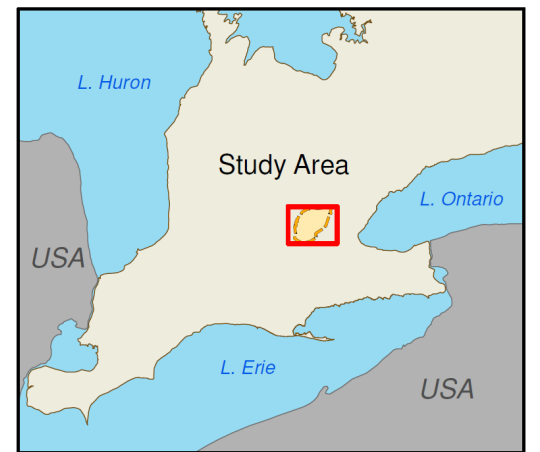


Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

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-  Test Production Well
-  Expressway / Highway
-  Major Roads
-  Roads (collectors)
-  Rivers / Streams
-  Lakes and Ponds
- Hydraulic conductivity zone
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 -  41010070

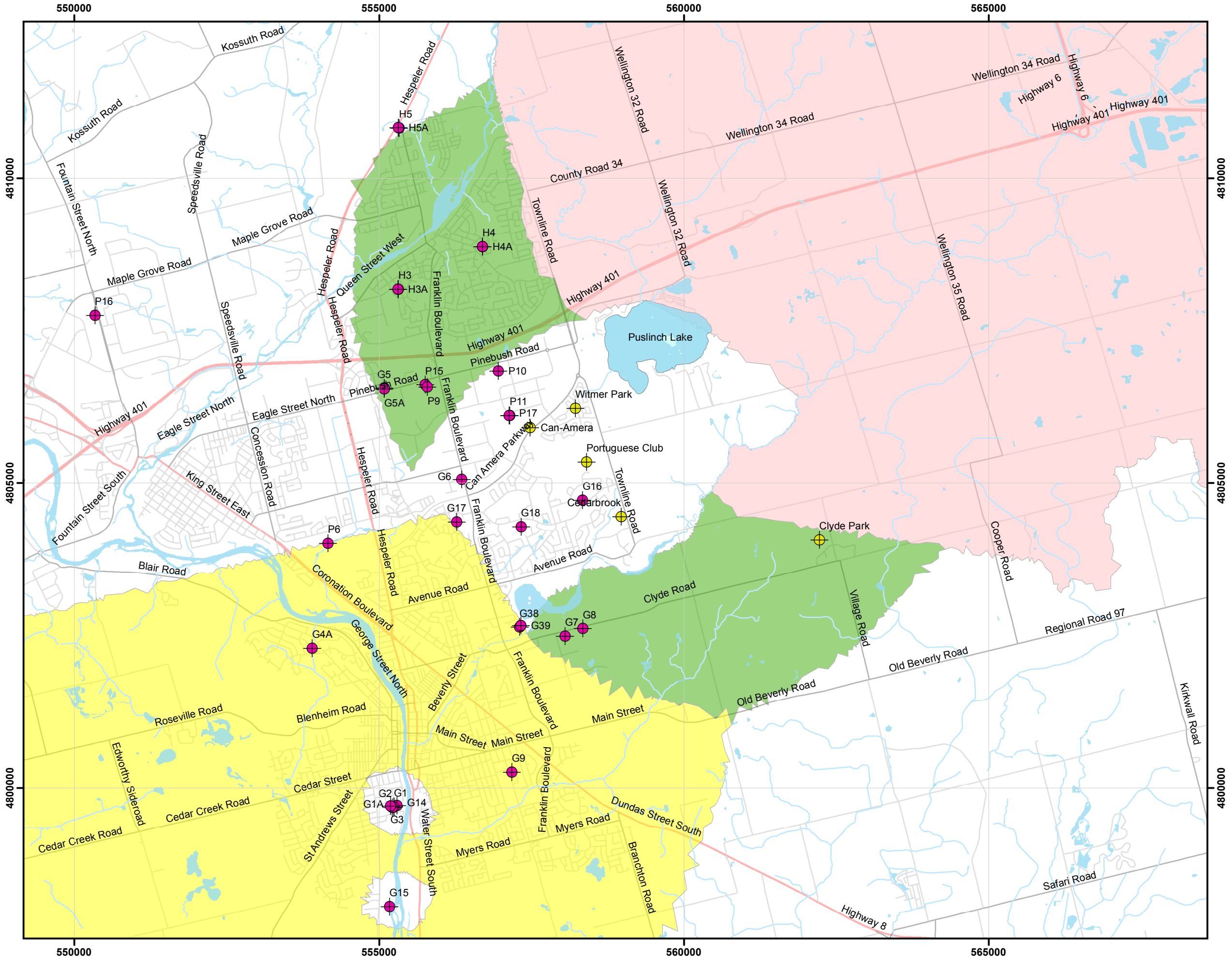
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Projection: UTM Zone 17N, NAD 83
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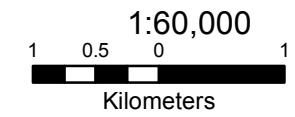
Figure 3
 Locations of Sensitive Hydraulic Conductivity Zones in the Reformatory Quarry Member



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

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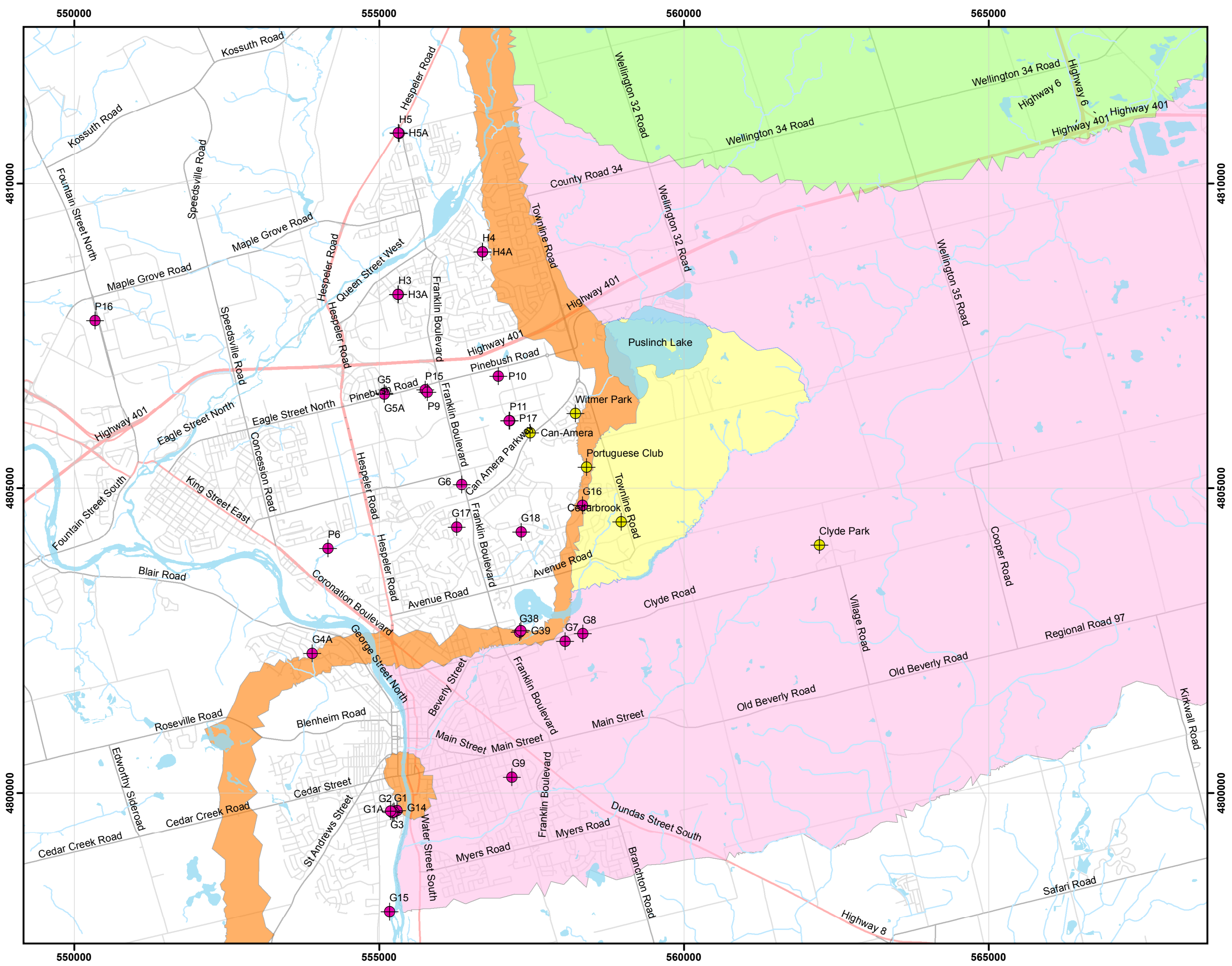
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-  Expressway / Highway
-  Major Roads
-  Roads (collectors)
-  Rivers / Streams
-  Lakes and Ponds
- Hydraulic conductivity zone
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Projection: UTM Zone 17N, NAD 83
Map Version: 2, Map data 2014-05-27 J. Zhang












Figure 4
Locations of Sensitive Hydraulic Conductivity Zones in the Vinemount Member



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LEGEND

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-  Test Production Well
-  Expressway / Highway
-  Major Roads
-  Roads (collectors)
-  Rivers / Streams
-  Lakes and Ponds
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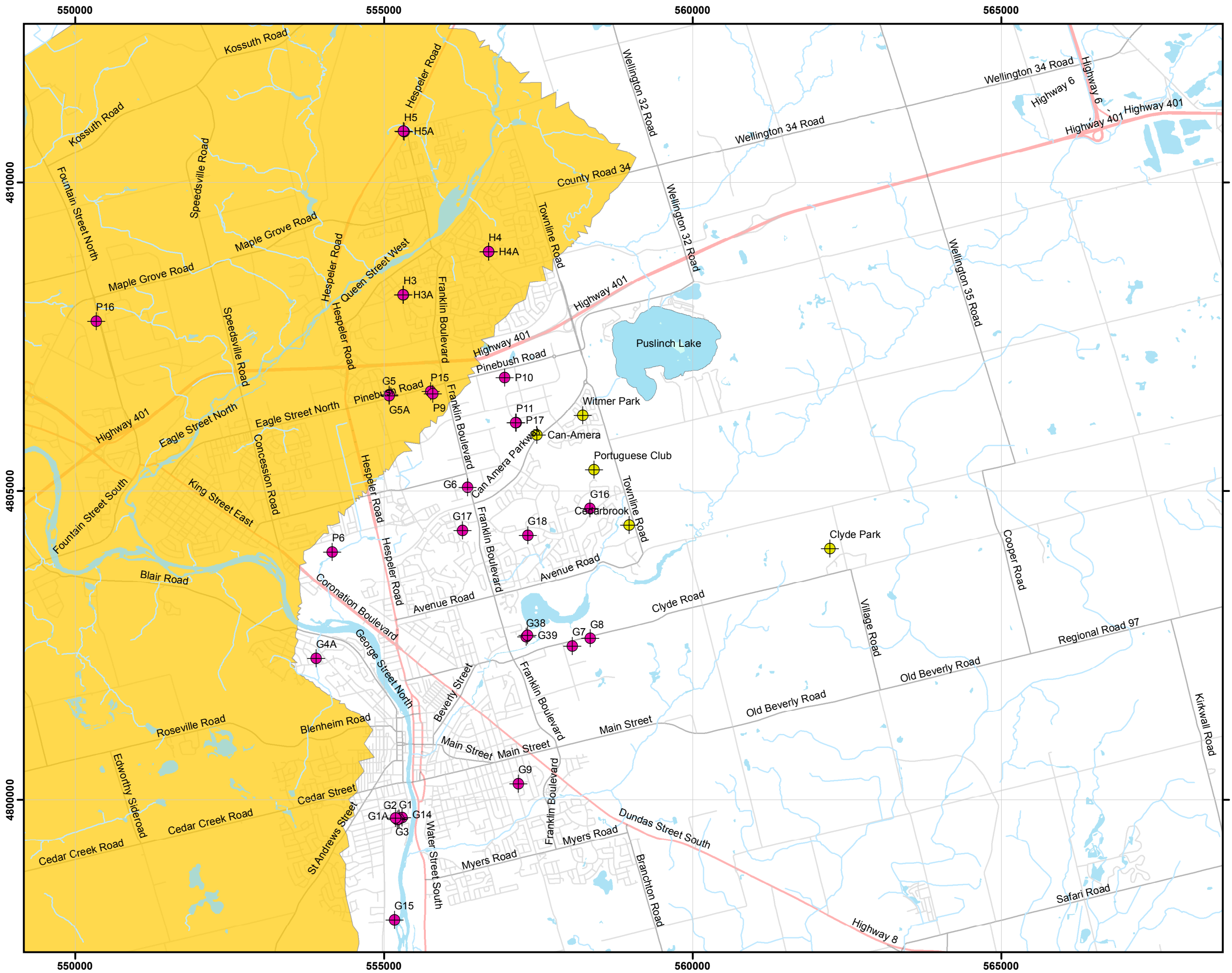


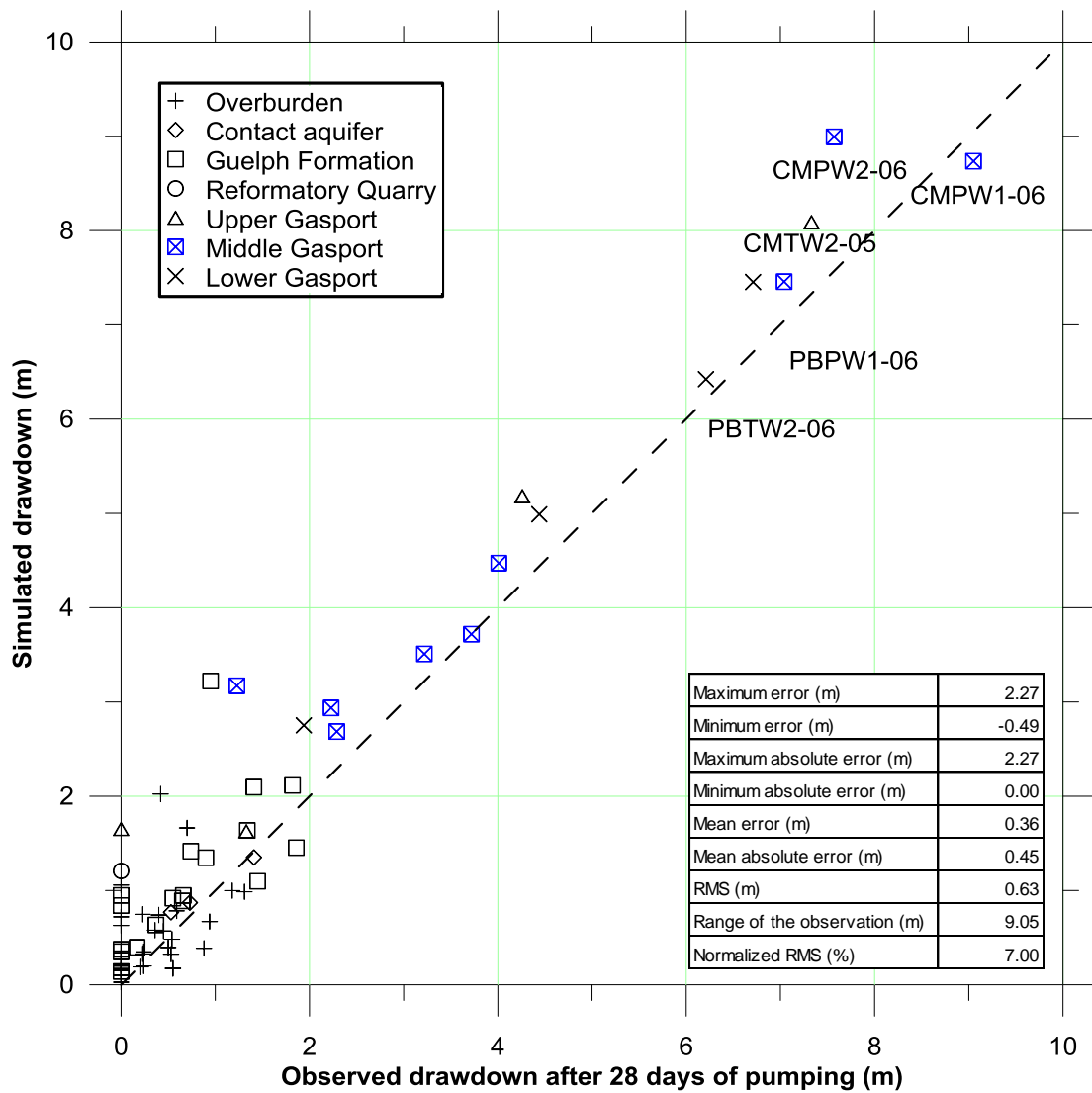
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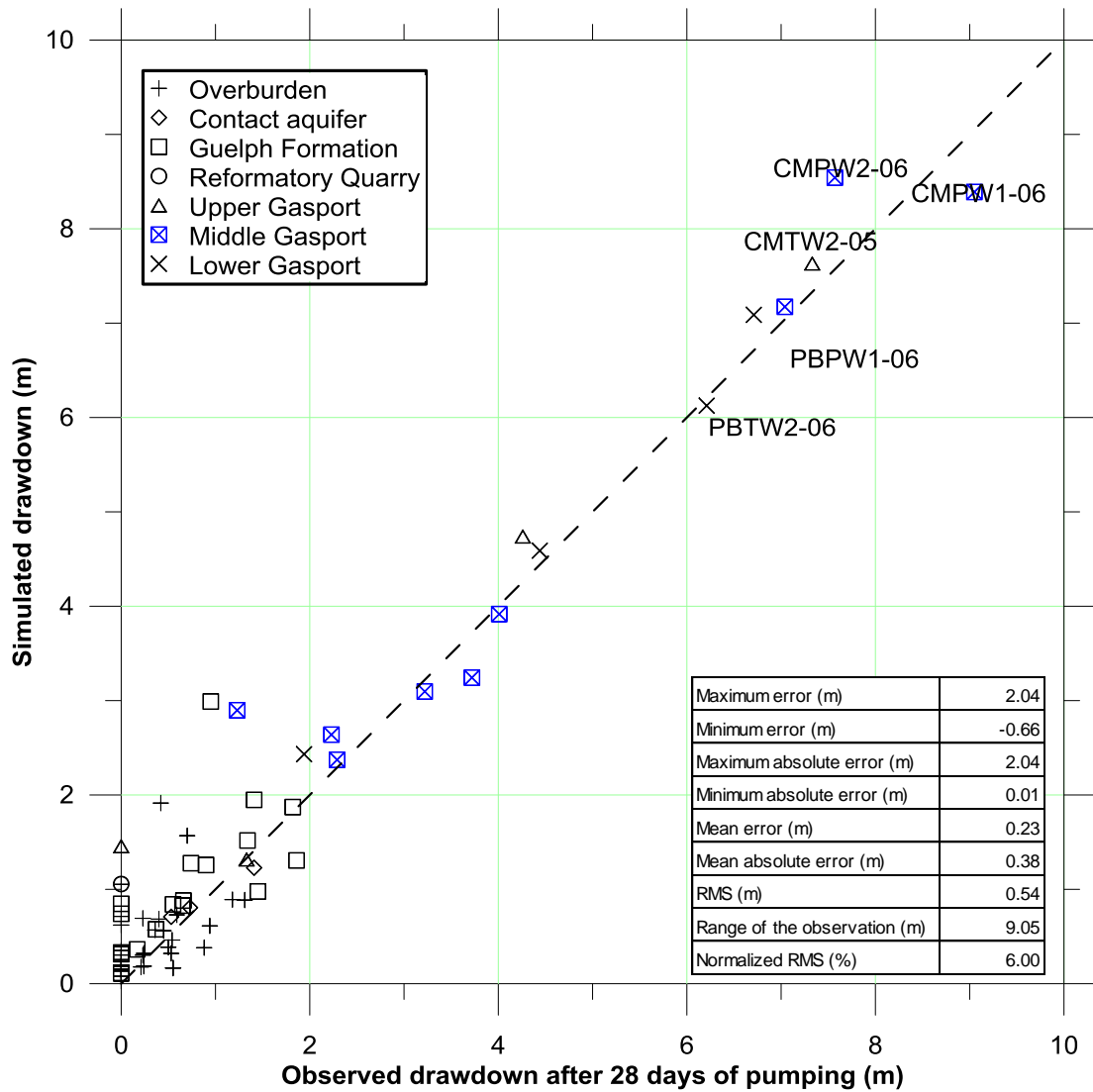


Figure 5

Locations of Sensitive Hydraulic Conductivity Zones in the Upper Gasport Formation







**Figure 8: Final Drawdowns for
 Case 1 ($S_s=10^{-5}/m$)**

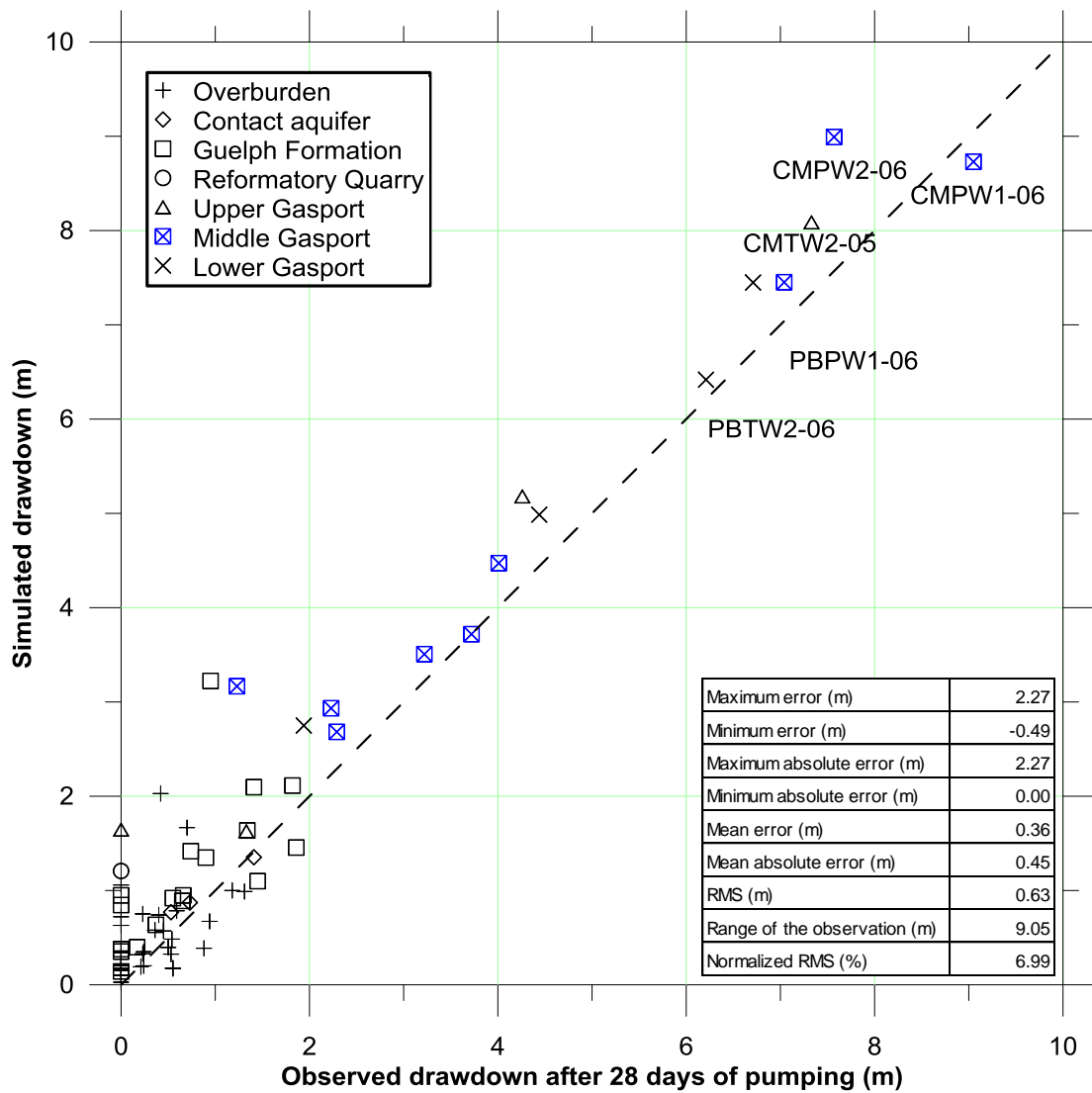
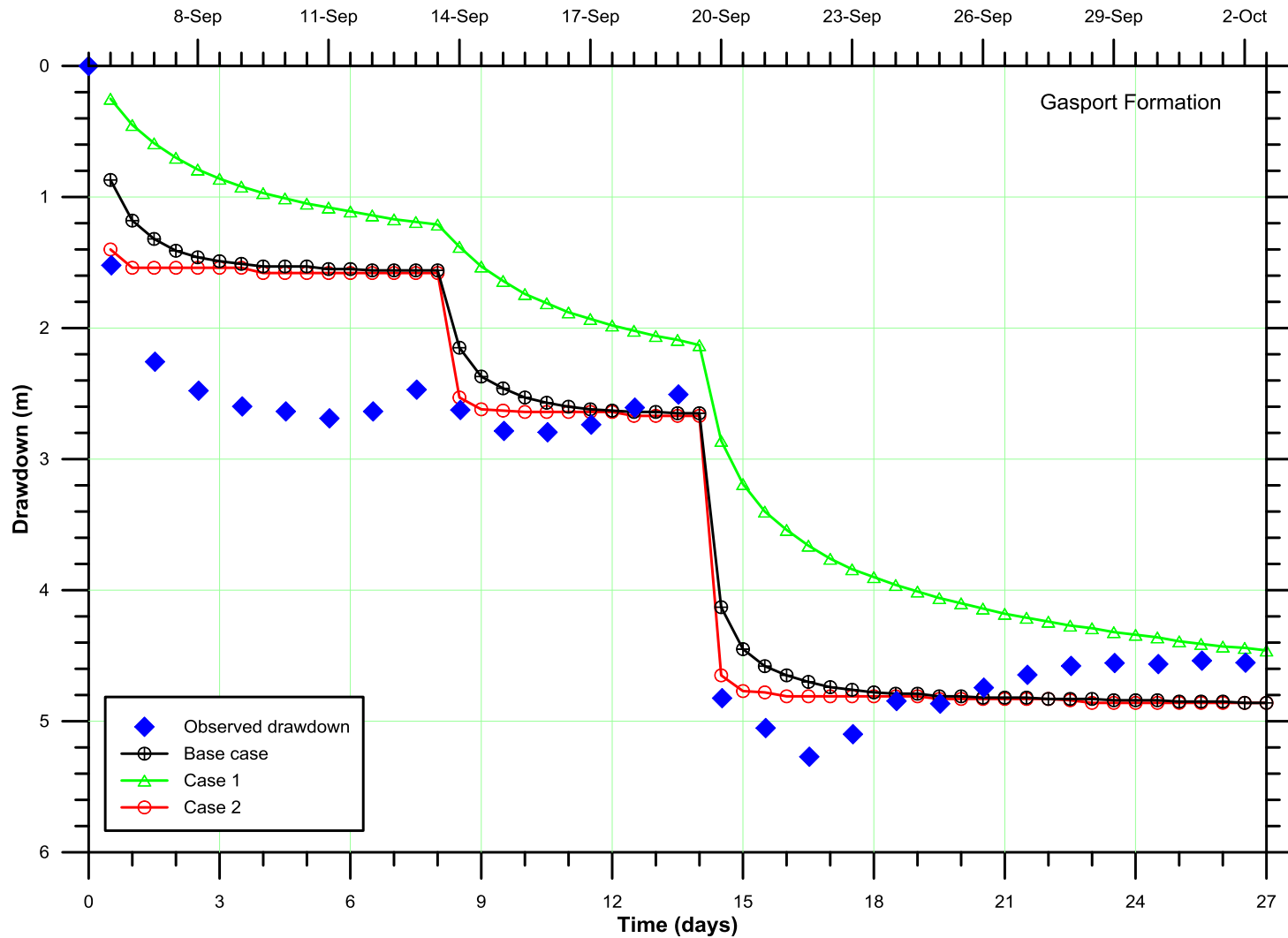


Figure 9: Final Drawdowns for Case 2 ($S_s=10^{-7}/m$)



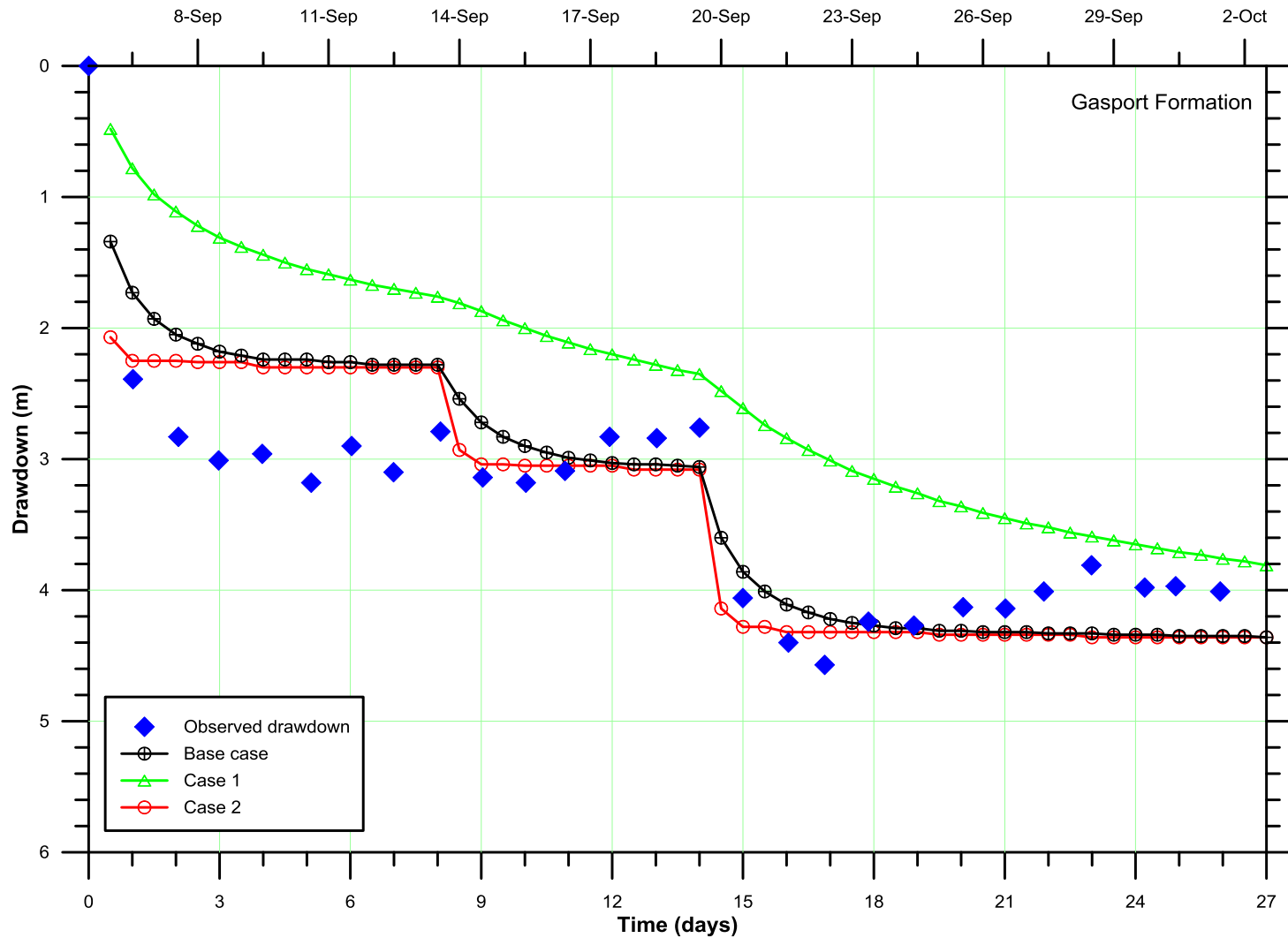
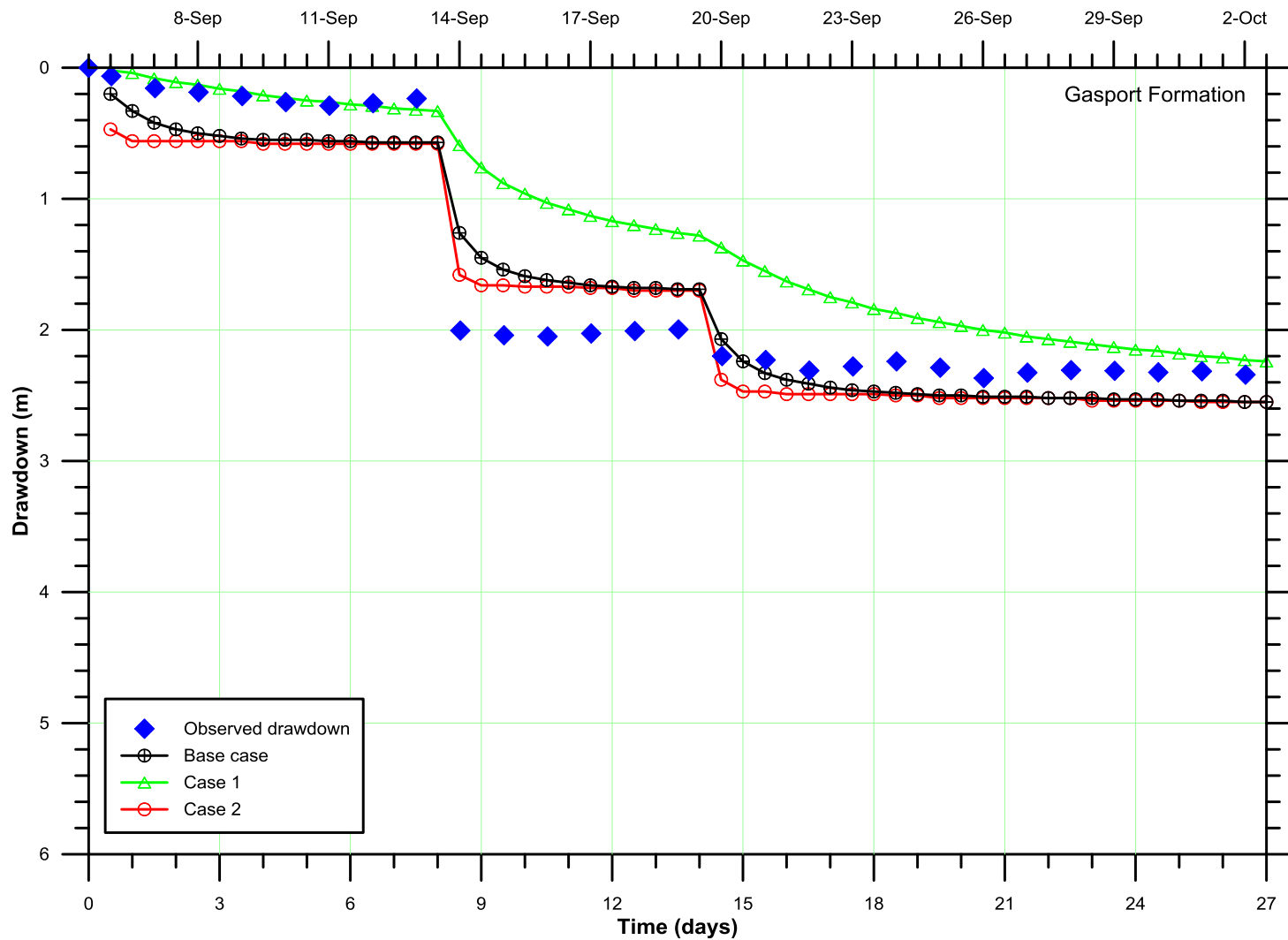
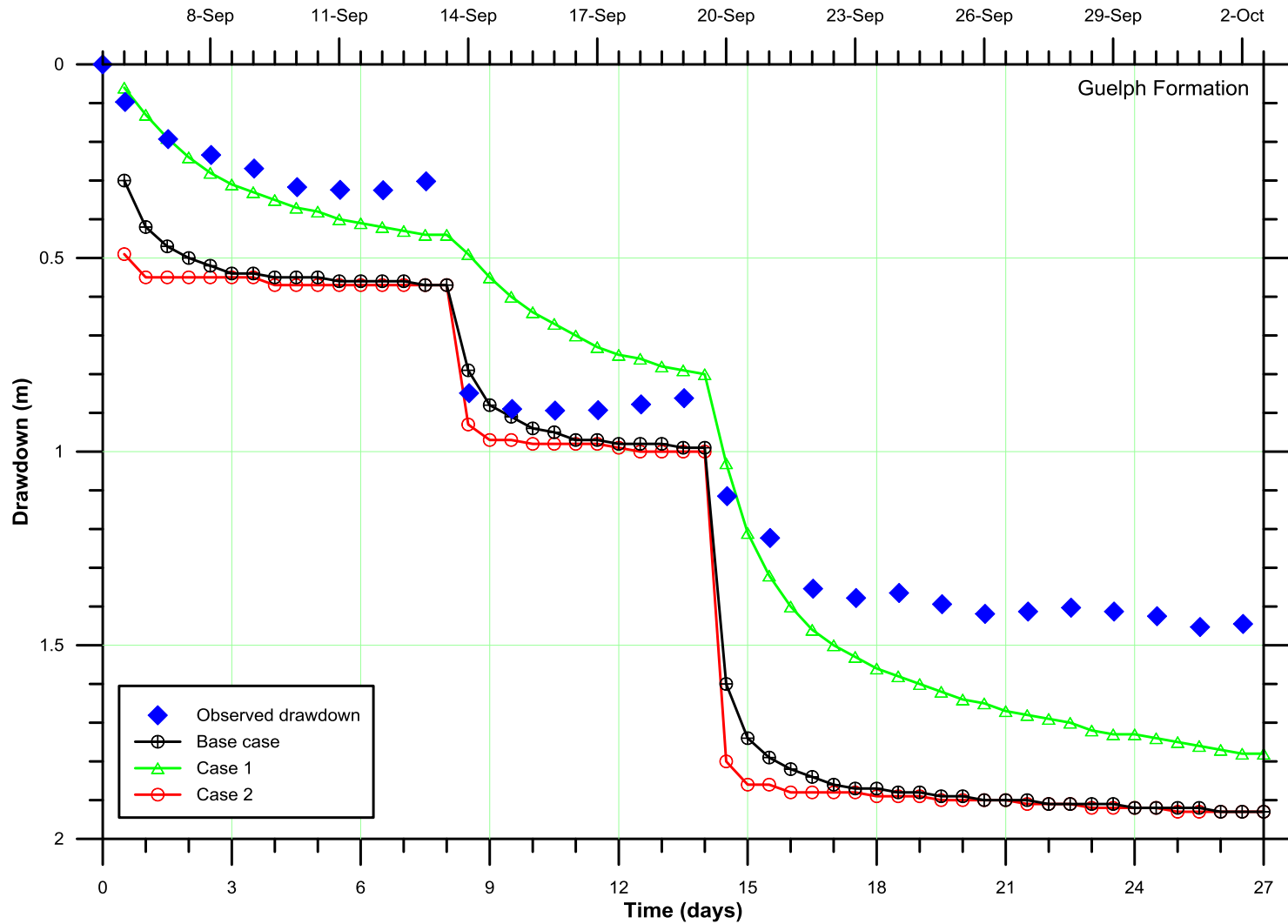


Figure 11 Simulated and Observed Hydrographs
for CMO2A-06





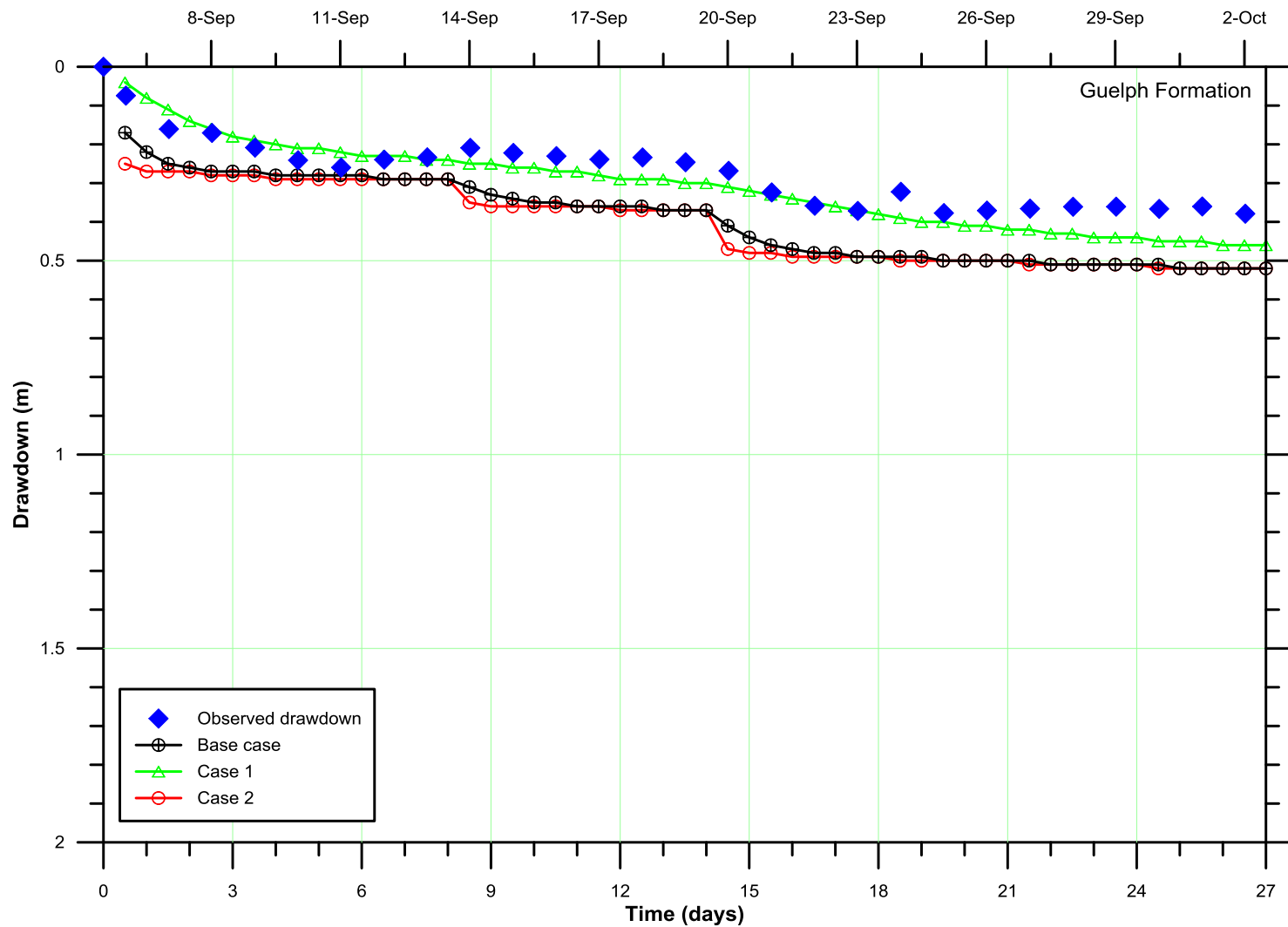
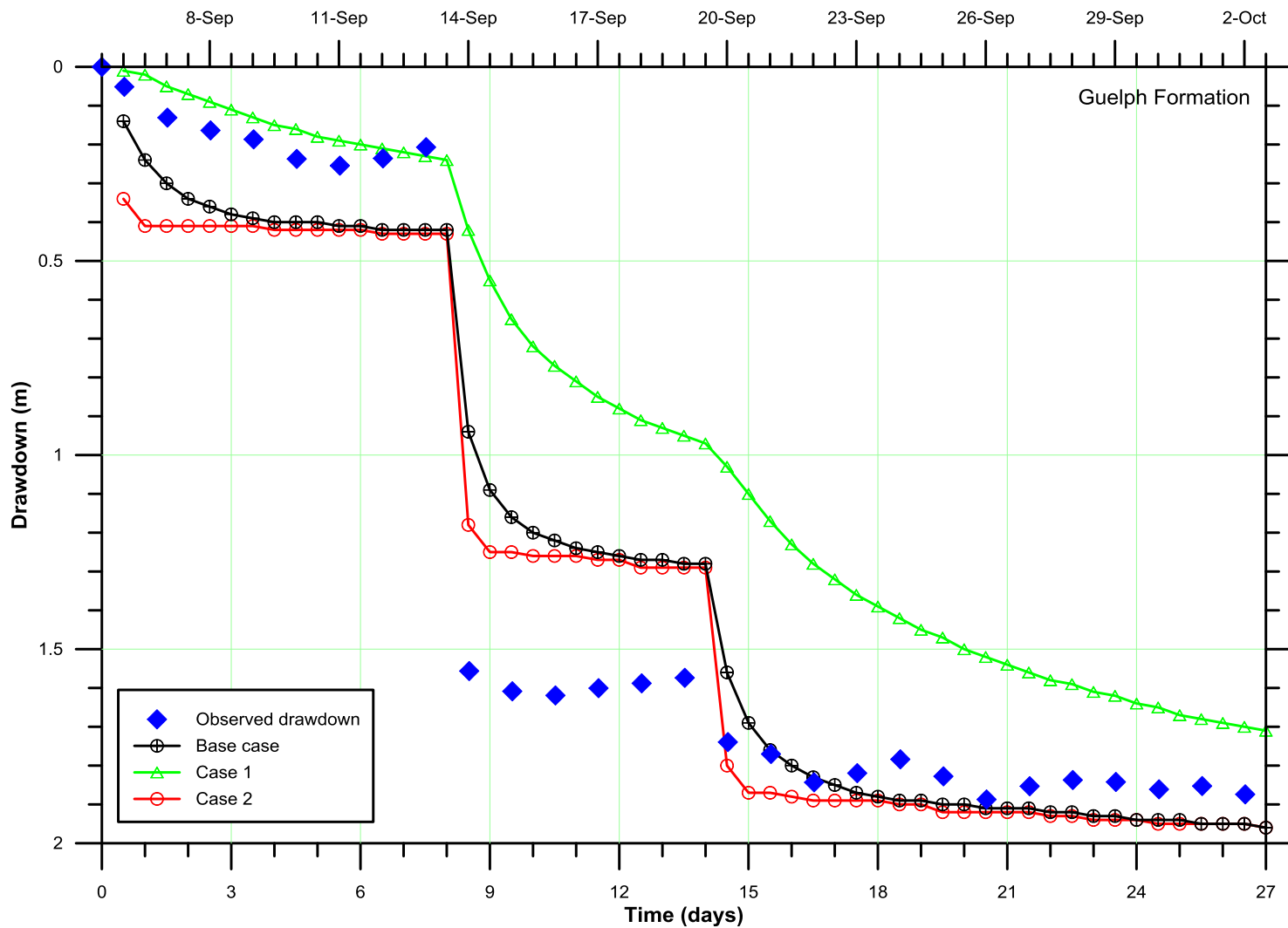


Figure 14 Simulated and Observed Hydrographs
for CMOW2B-06



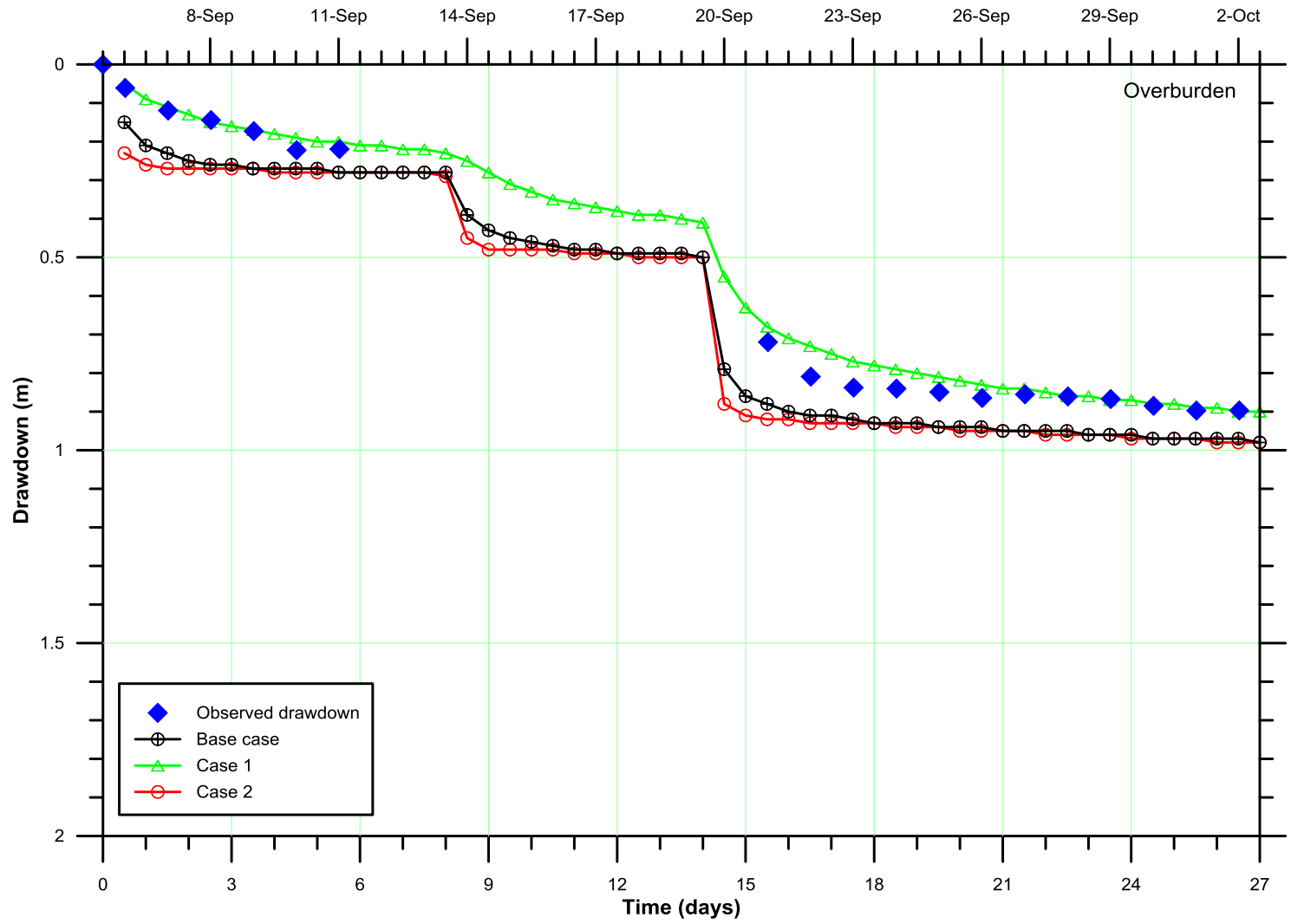


Figure 16 Simulated and Observed Hydrographs for CMOw1D-06

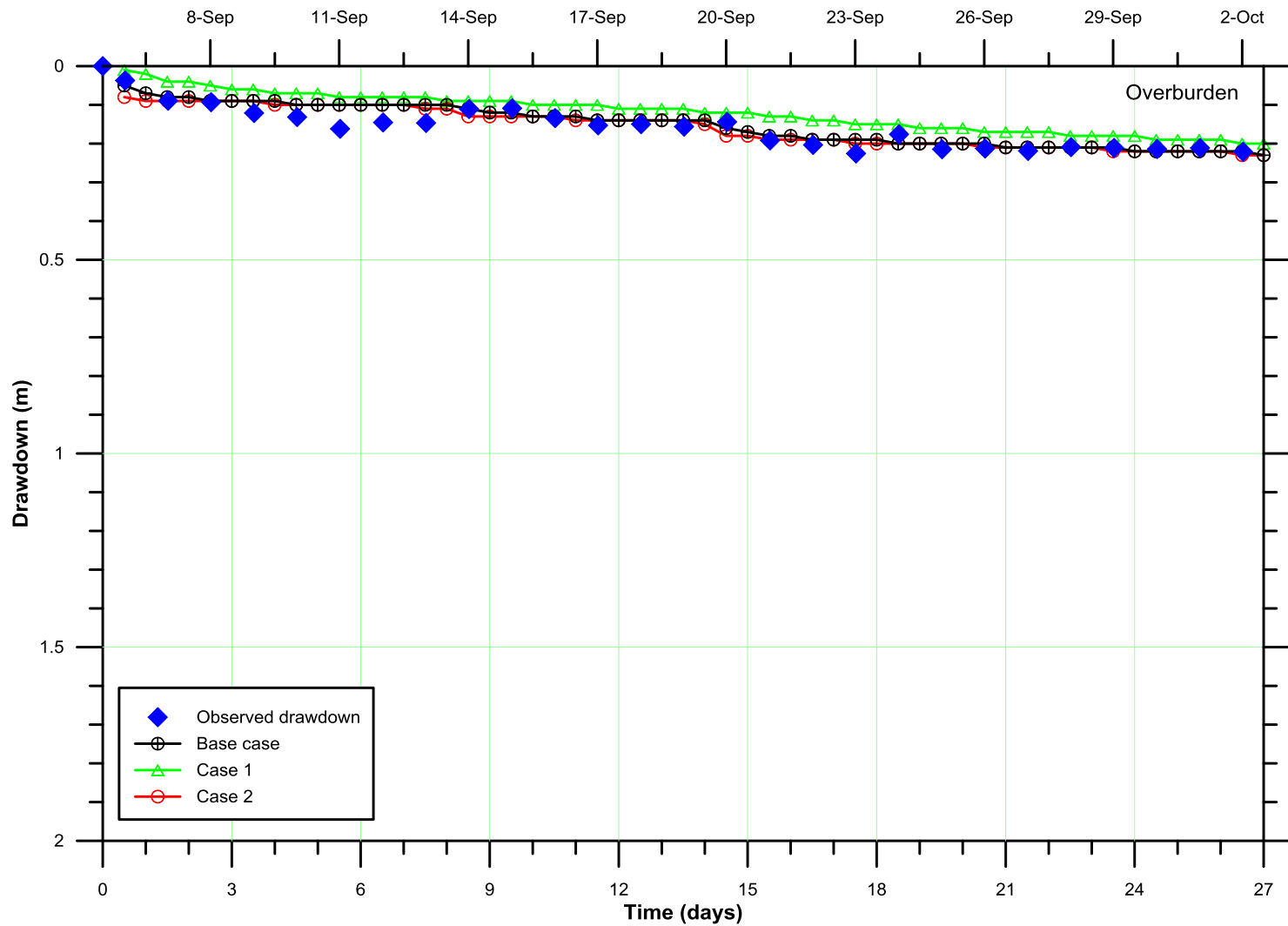
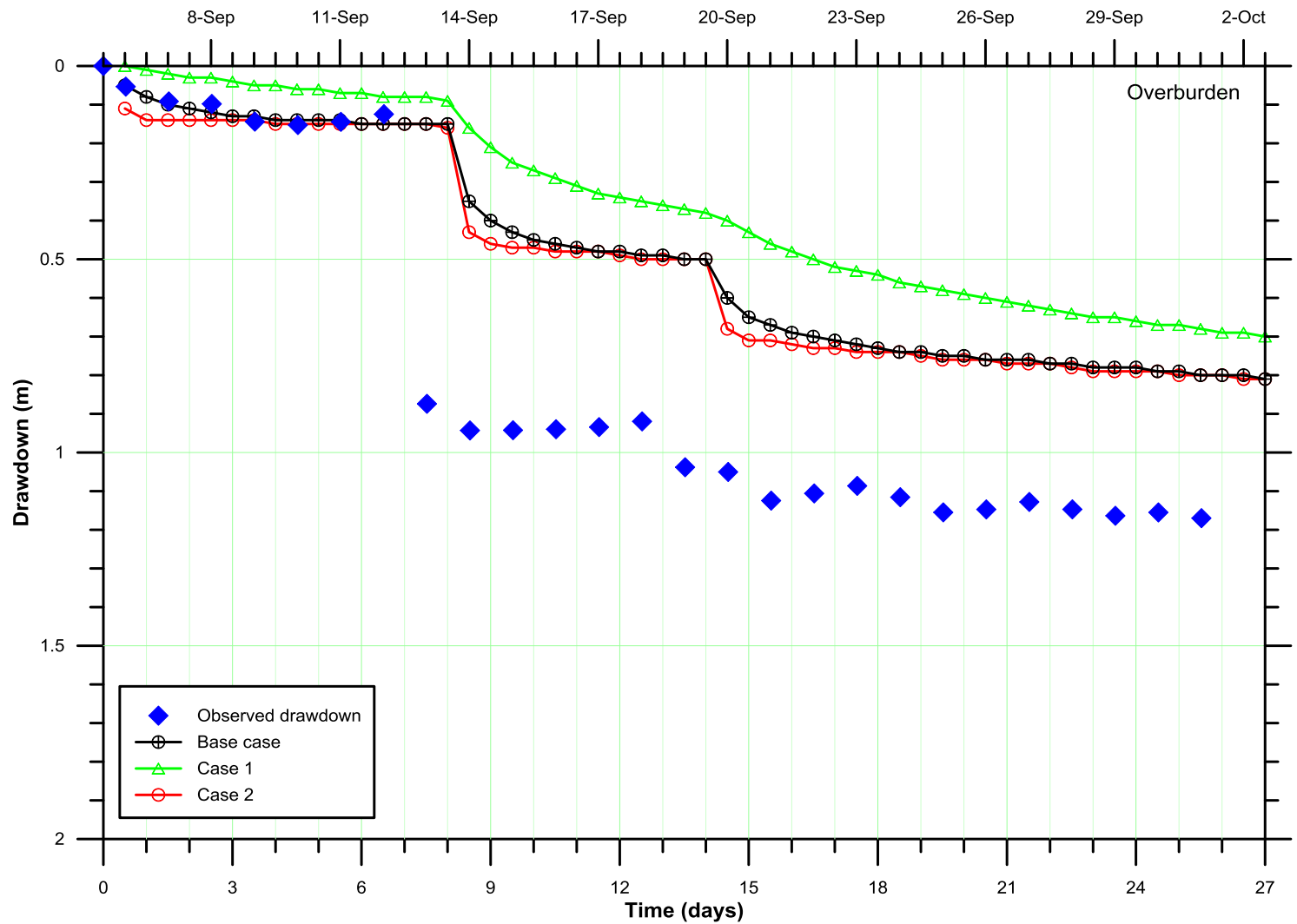


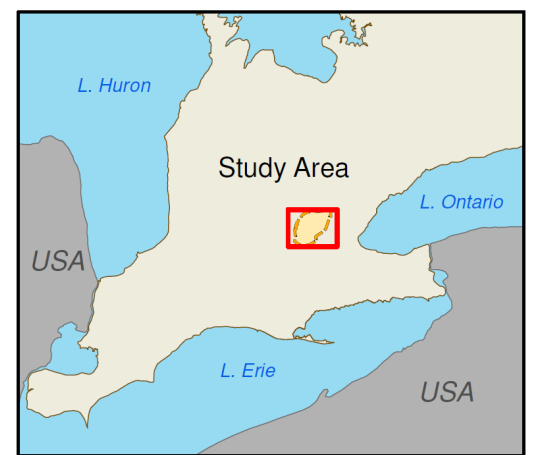
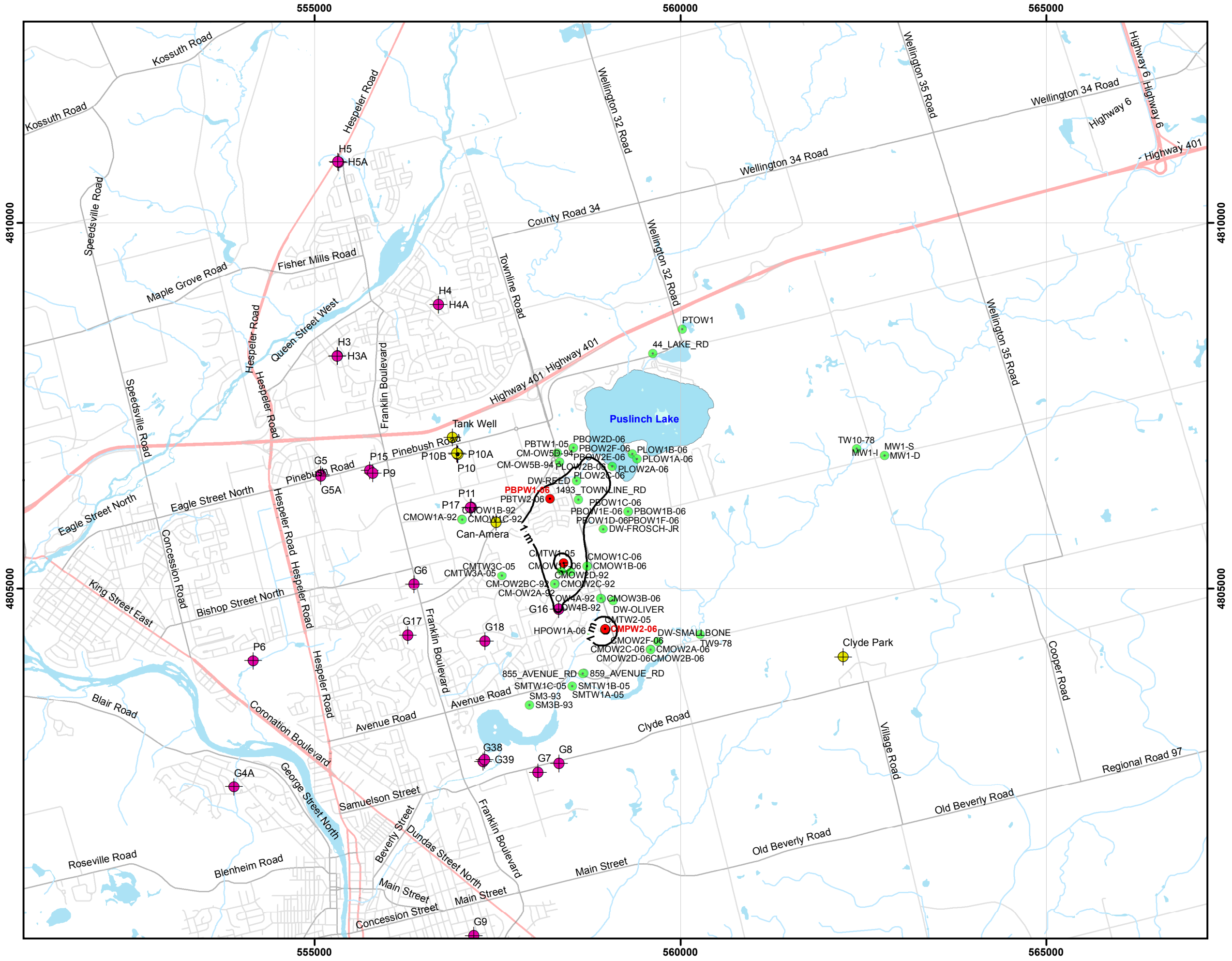
Figure 17 Simulated and Observed Hydrographs for CMOw2D-06



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

- Wells Pumped during the 28-day Test
- Observation Wells
- Municipal Supply Well
- Test Production Well
- Drawdown Contours at the End of the Test
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds



Projection: UTM Zone 17N, NAD 83
 Map Version: 2, Map data 2014-05-27 J. Zhang

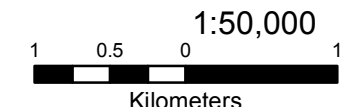
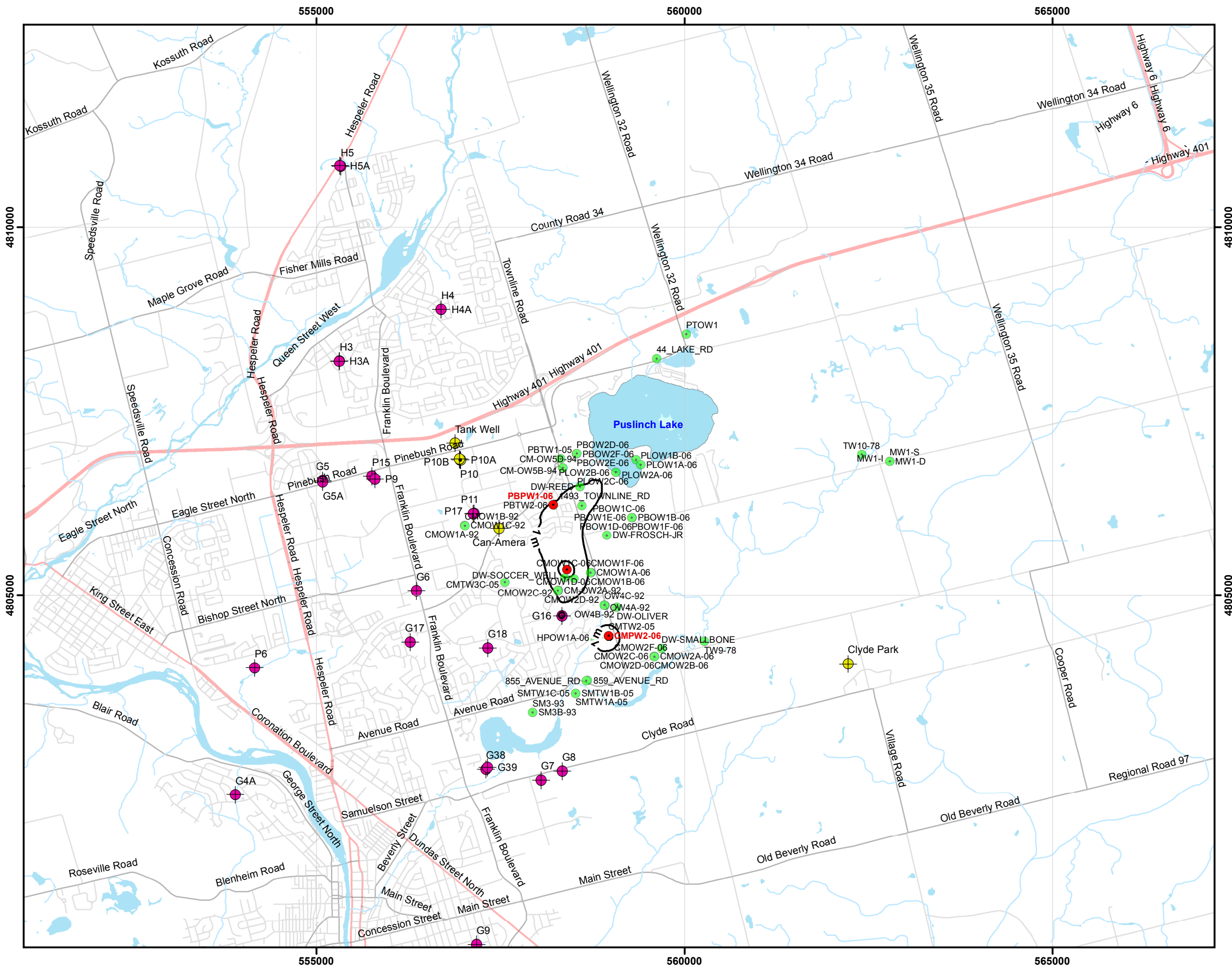


Figure 19
 Simulated Final Drawdowns in the Contact Aquifer for the Base Case (Ss=10⁻⁶ /m)

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

- Wells Pumped during the 28-day Test
- Observation Wells
- Municipal Supply Well
- Test Production Well
- Drawdown Contours at the End of the Test
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds



Projection: UTM Zone 17N, NAD 83
 Map Version: 2, Map data 2014-05-27 J. Zhang

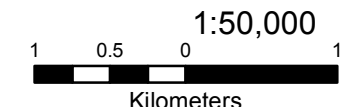
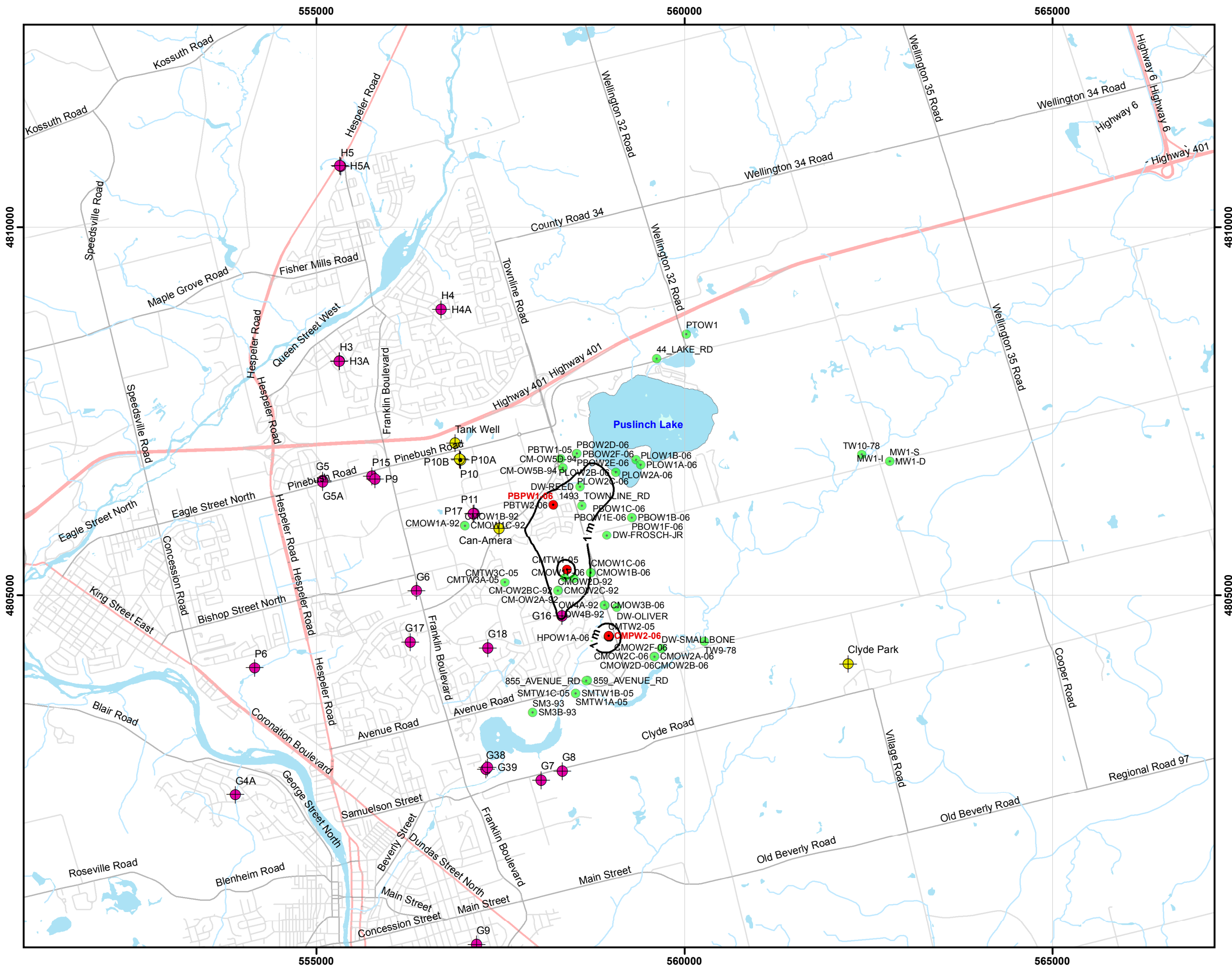


Figure 20
 Simulated Final Drawdowns in the Contact Aquifer for Case 1
 ($S_s=10^{-5}$ /m)

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

- Wells Pumped during the 28-day Test
- Observation Wells
- Municipal Supply Well
- Test Production Well
- Drawdown Contours at the End of the Test
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds



Projection: UTM Zone 17N, NAD 83
 Map Version: 2, Map data 2014-05-27 J. Zhang

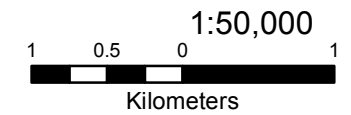


Figure 21
 Simulated Final Drawdowns in the Contact Aquifer for Case 2
 ($S_s=10^{-7}$ /m)

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

- Wells Pumped during the 28-day Test
- Observation Wells
- Municipal Supply Well
- Test Production Well
- Drawdown Contours at the End of the Test
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds

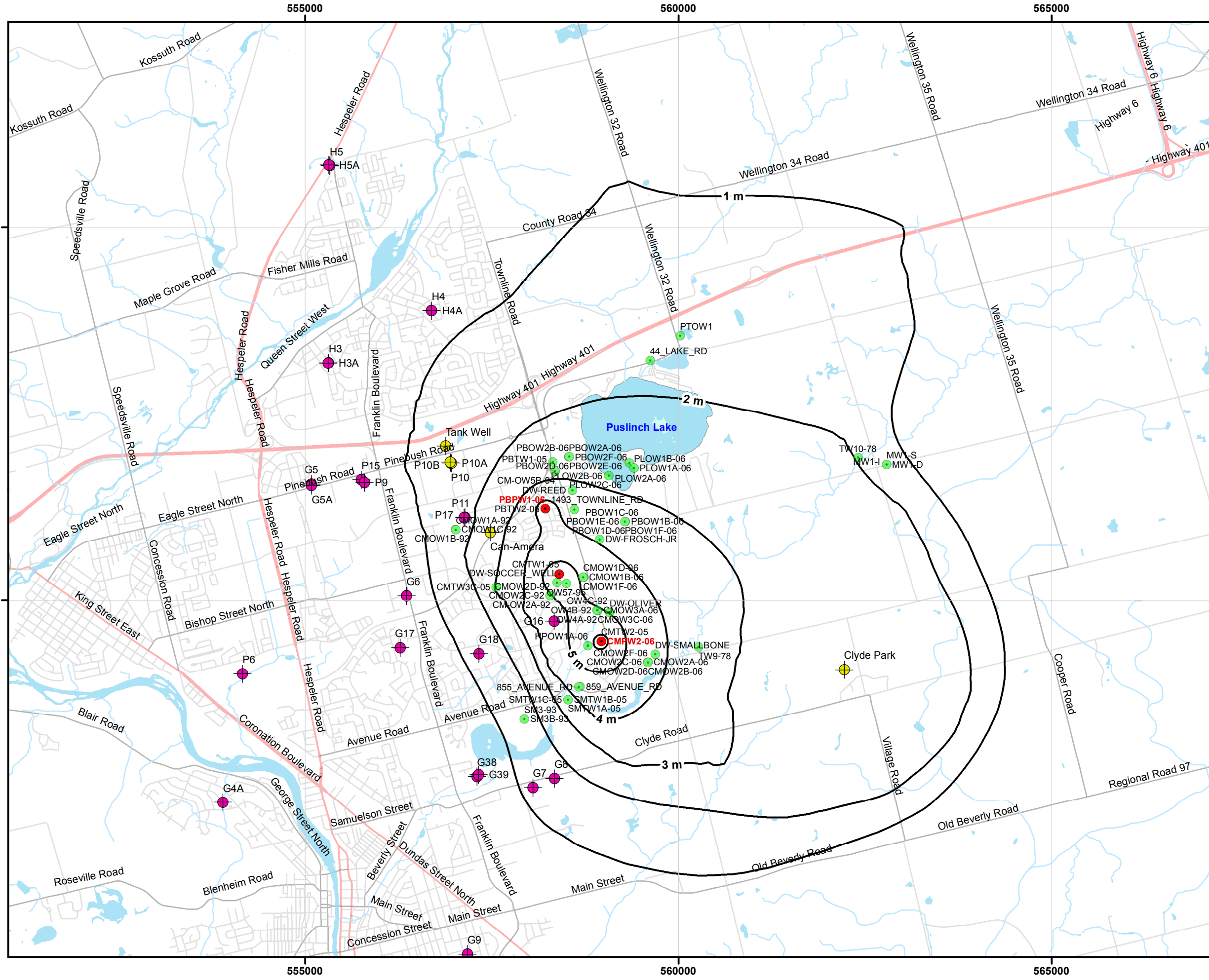


Projection: UTM Zone 17N, NAD 83
 Map Version: 2, Map data 2014-05-27 J. Zhang



Figure 22

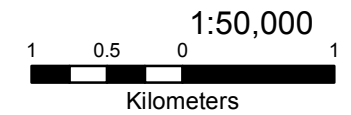
Simulated Final Drawdowns in the Middle Gasport Formation Aquifer for the Base Case ($S_s=10^{-6}/m$)



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

- Wells Pumped during the 28-day Test
- Observation Wells
- Municipal Supply Well
- Test Production Well
- Drawdown Contours at the End of the Test
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds

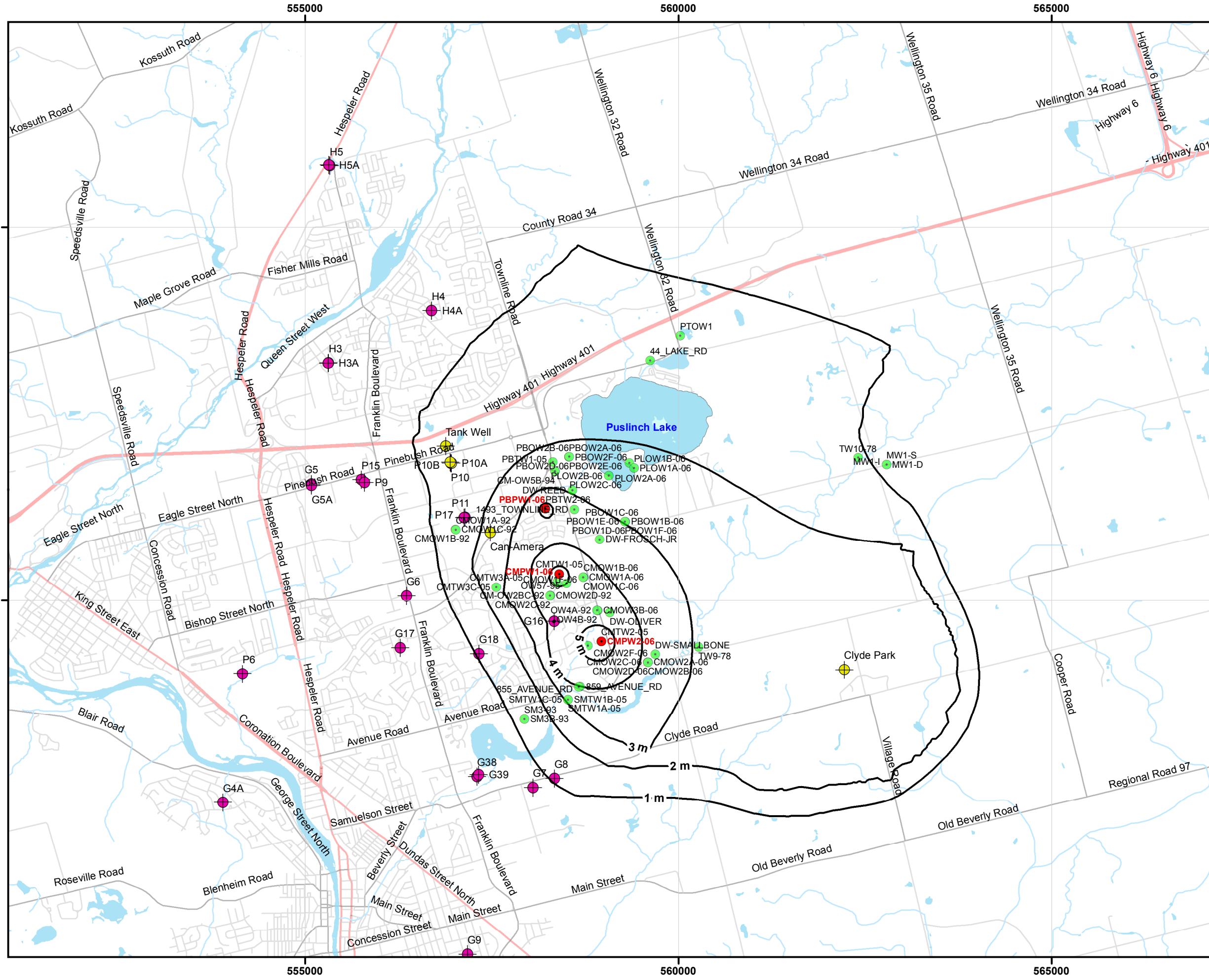


Projection: UTM Zone 17N, NAD 83
Map Version: 2, Map data 2014-05-27 J. Zhang



Figure 23

Simulated Final Drawdowns in the Middle Gasport Formation Aquifer for Case 1 ($S_s=10^{-5} / m$)



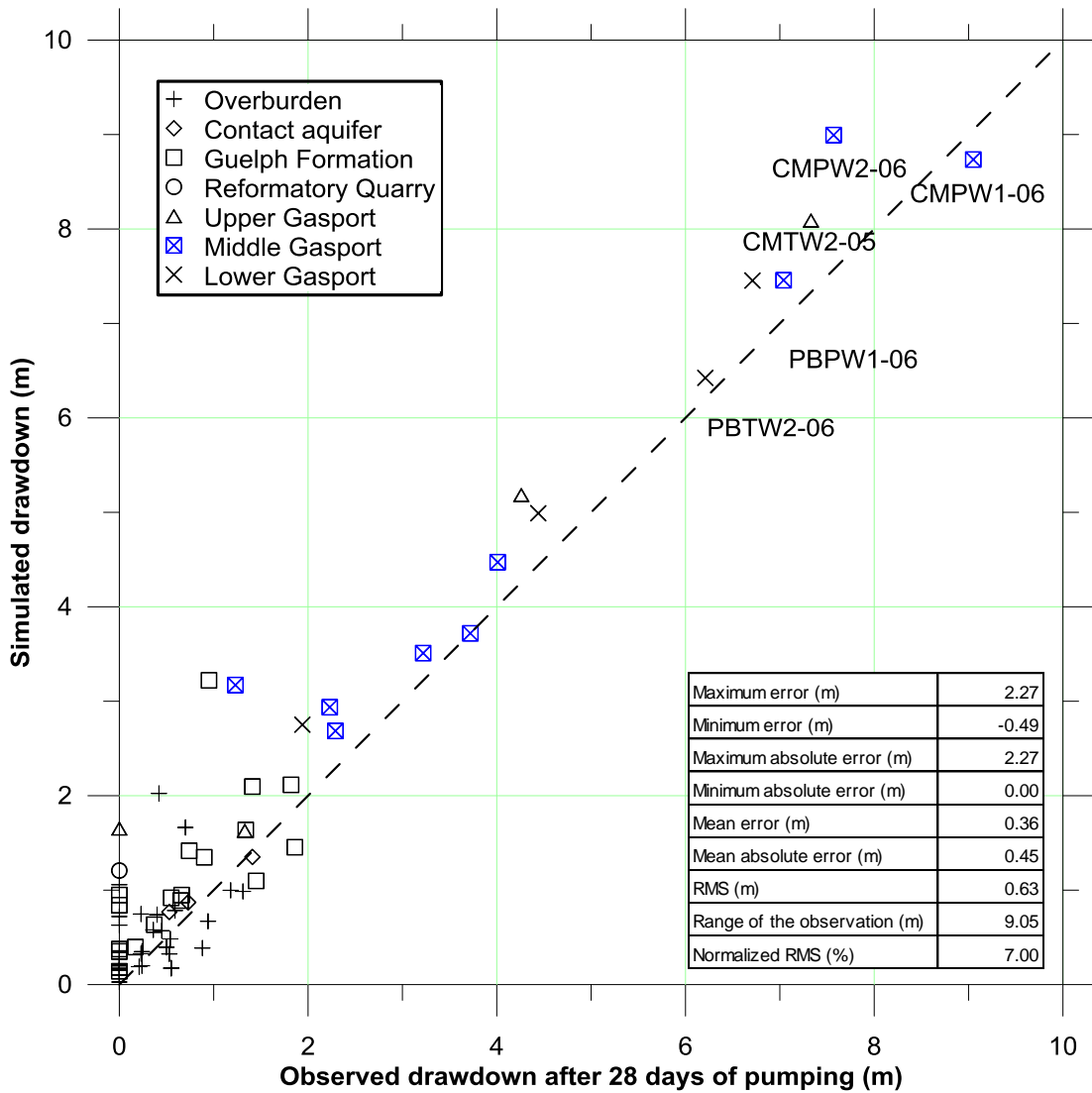
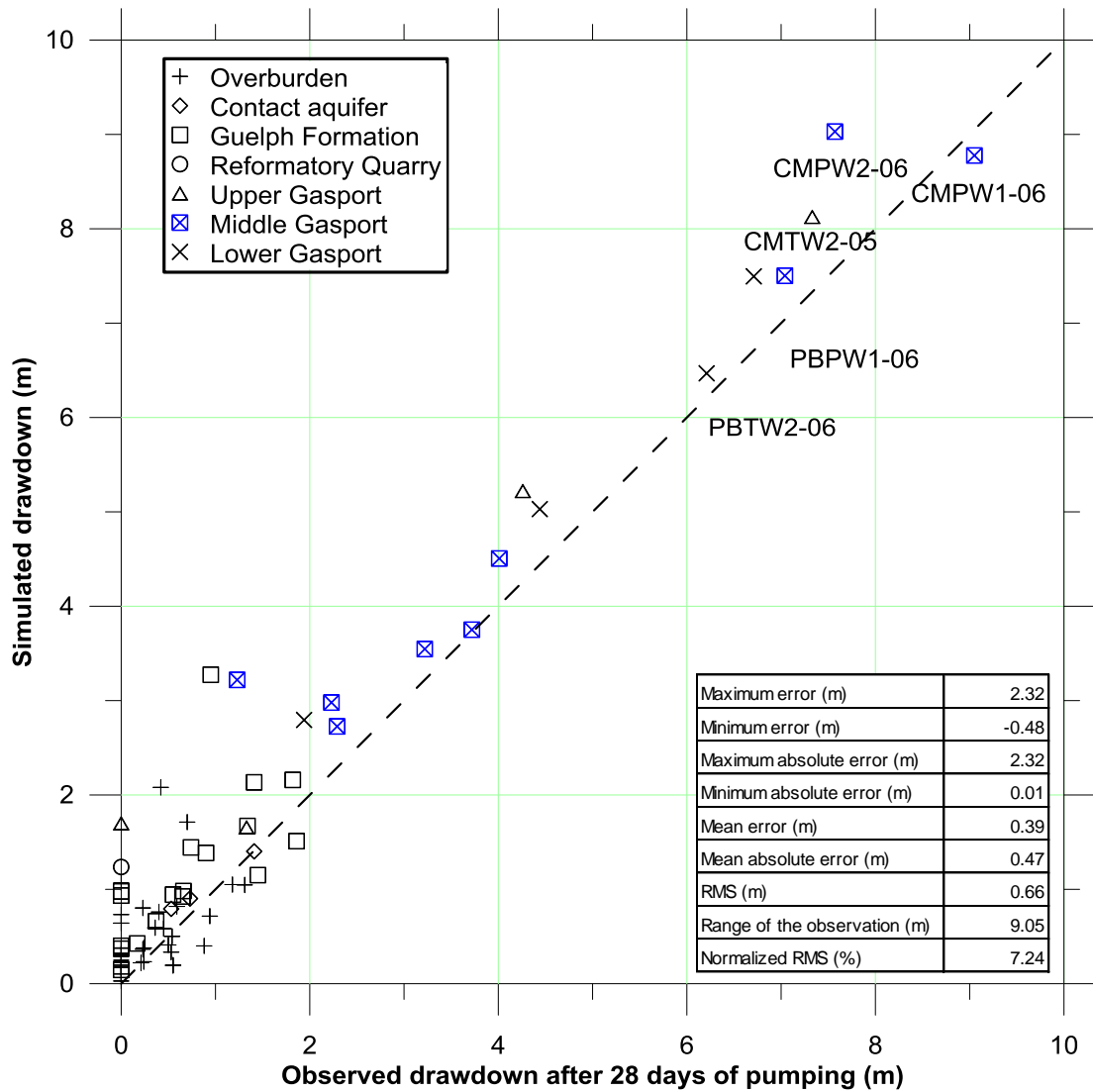
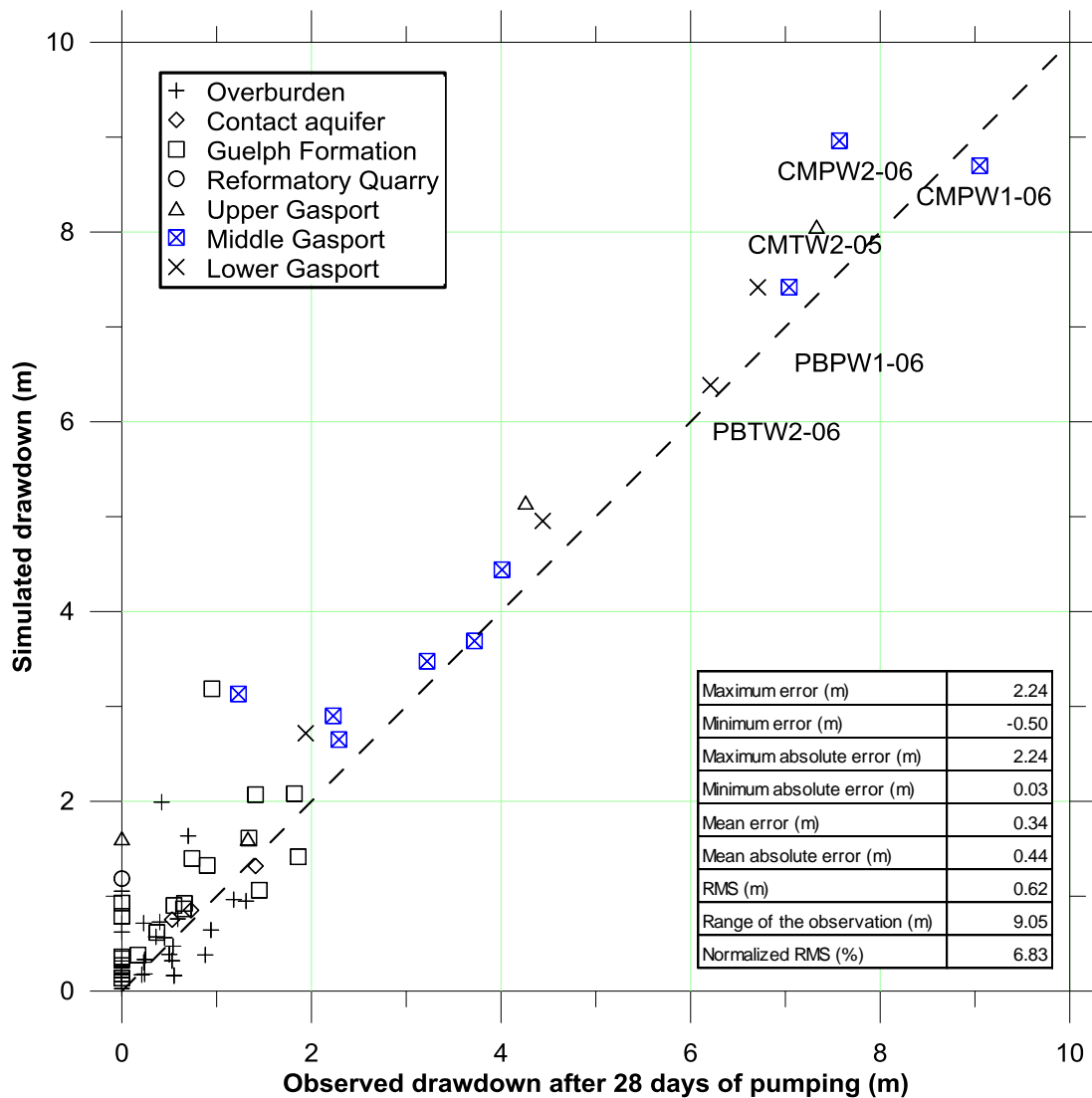


Figure 25 Final Drawdowns for
the Base Case ($\theta_{max} = 0.3$)





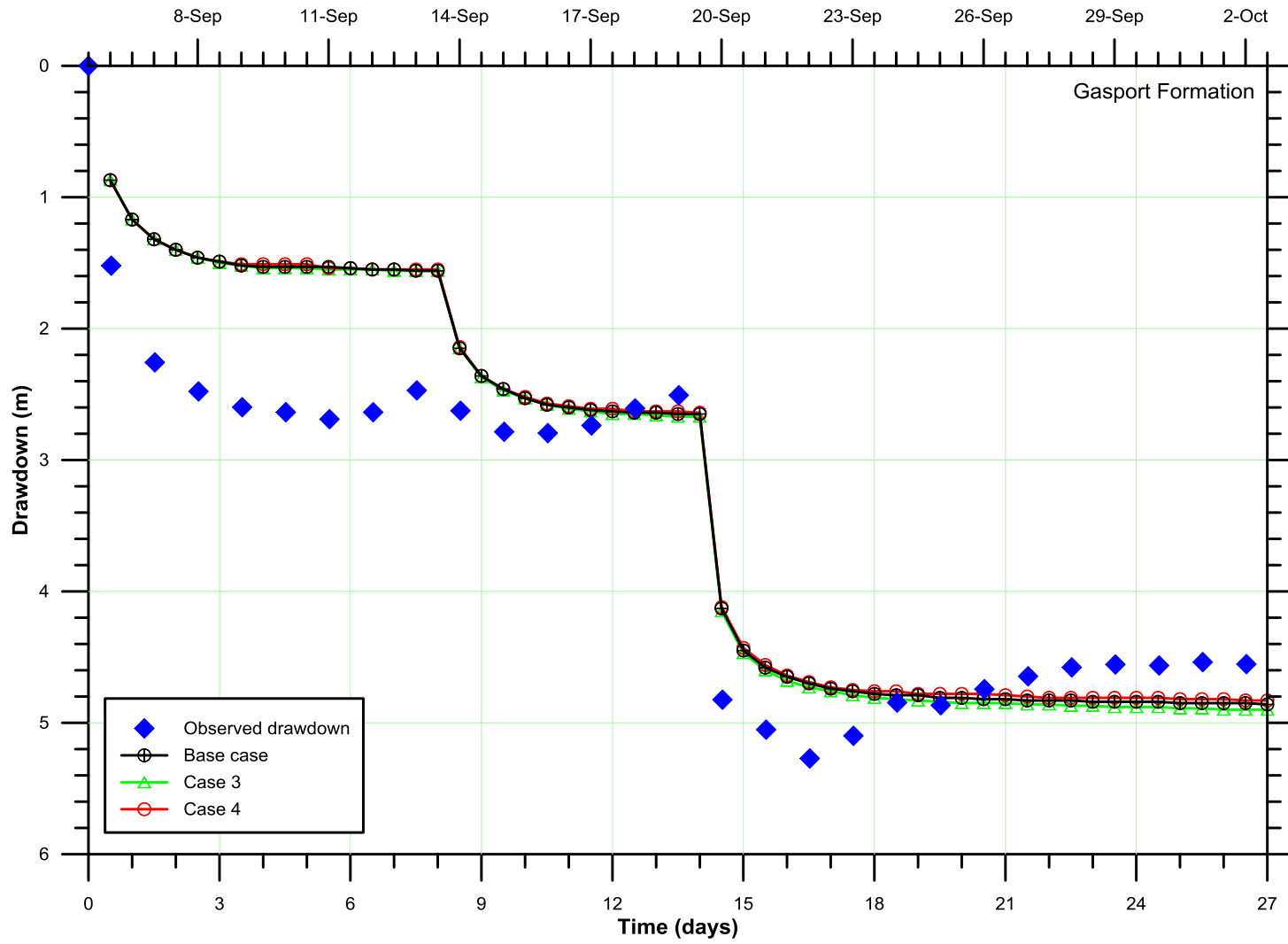


Figure 28 Simulated and Observed Hydrographs for CMOW1A-06 Sensitivity Analysis with Respect to the Specific Yield

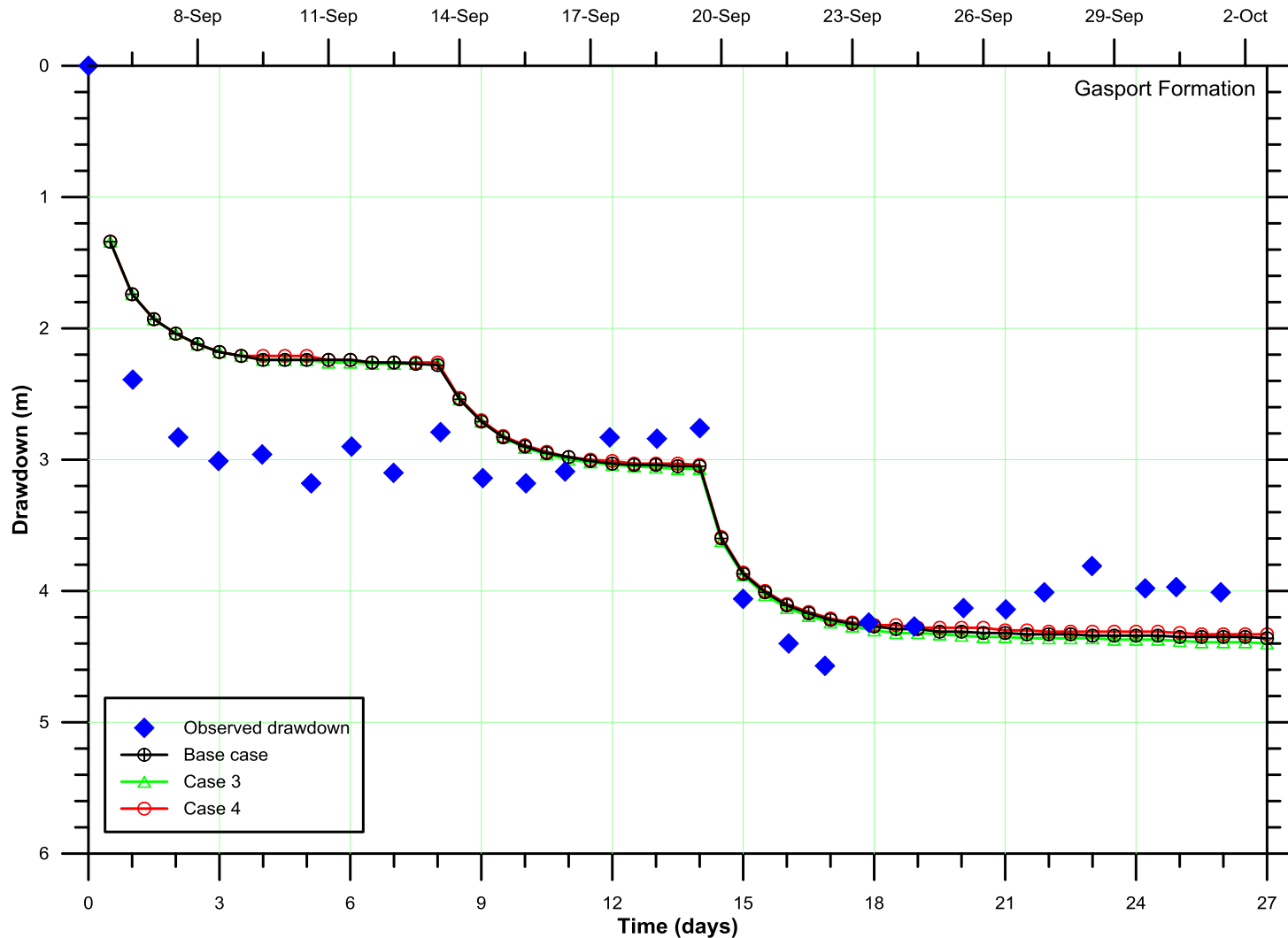


Figure 29 Simulated and Observed Hydrographs for CMO2A-06 Sensitivity Analysis with Respect to the Specific Yield

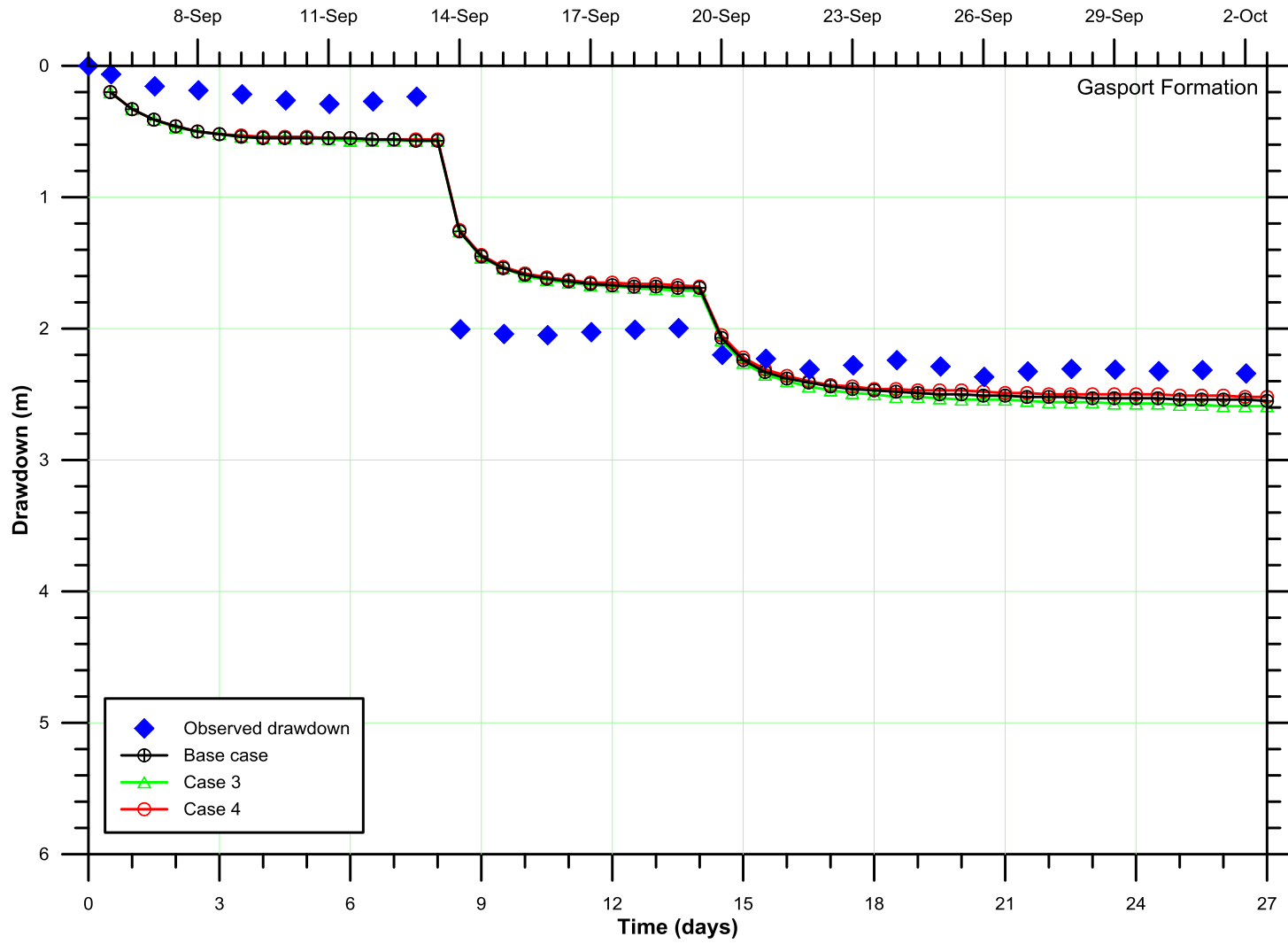


Figure 30 Simulated and Observed Hydrographs for PBOW2A-06 Sensitivity Analysis with Respect to the Specific Yield

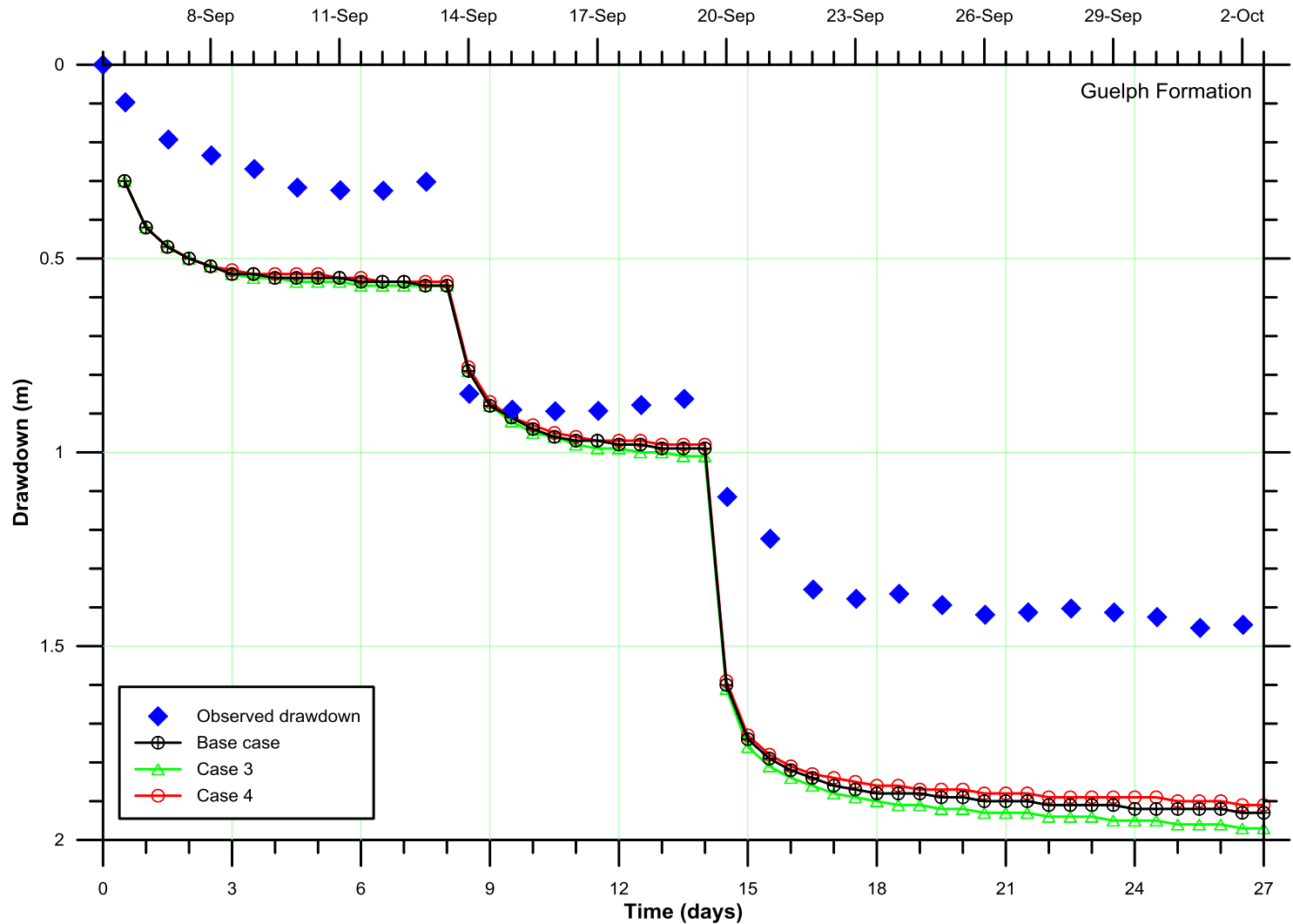


Figure 31: Simulated and Observed Hydrographs for CMOW1B-06 Sensitivity Analysis with Respect to the Specific Yield

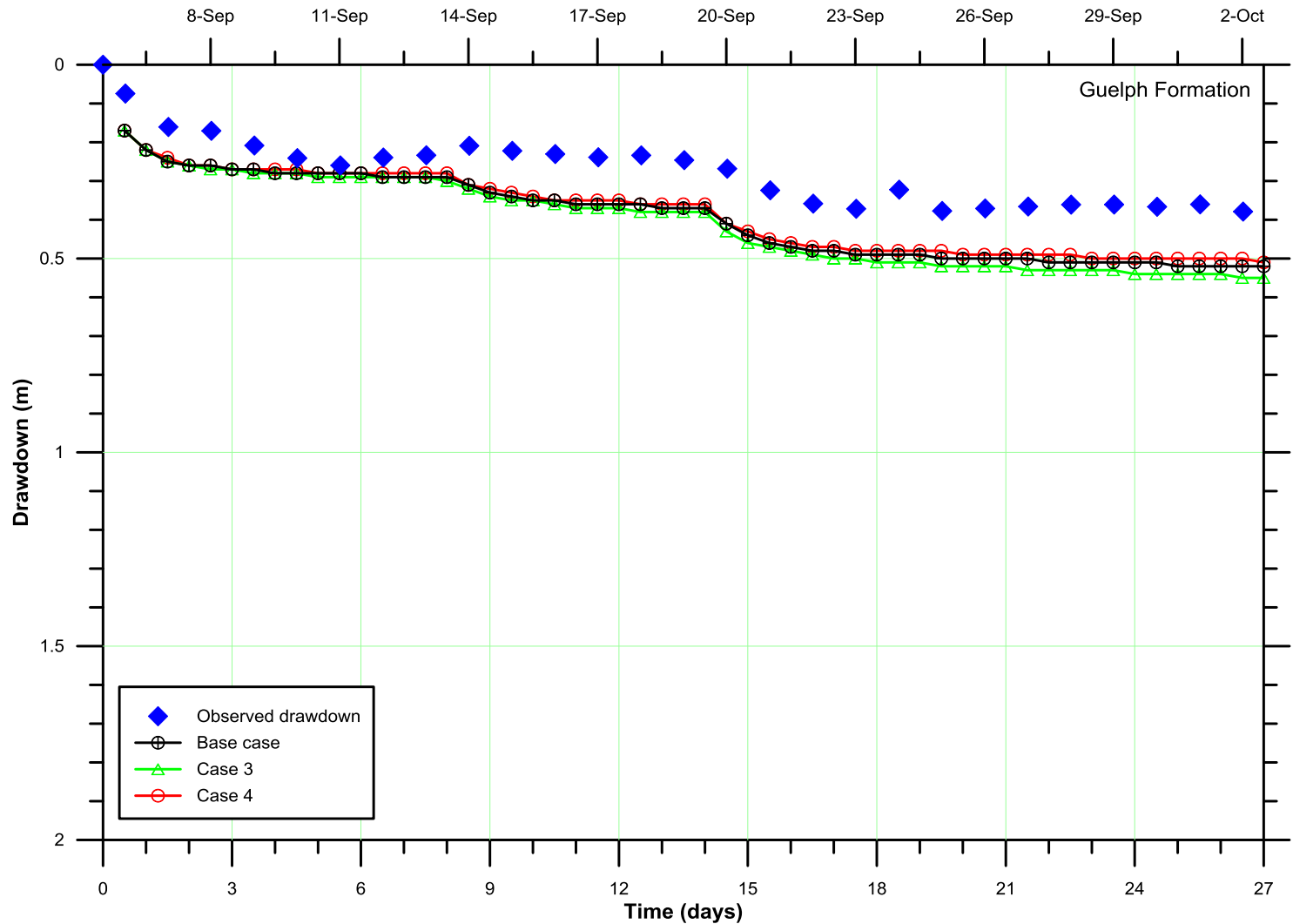


Figure 32 Simulated and Observed Hydrographs for CMOW2B-06 Sensitivity Analysis with Respect to the Specific Yield

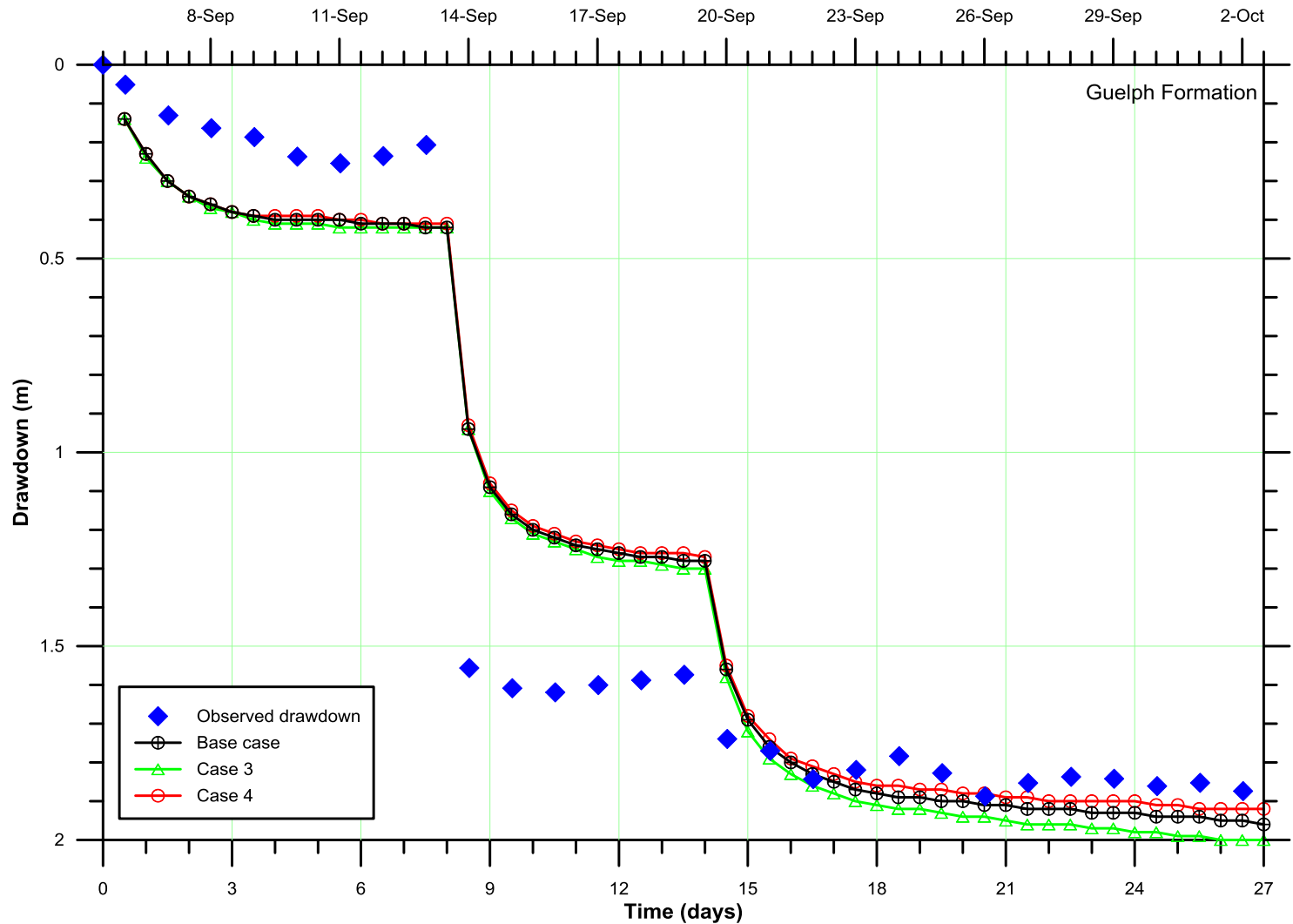


Figure 33 Simulated and Observed Hydrographs for PBO2B-06 Sensitivity Analysis with Respect to the Specific Yield

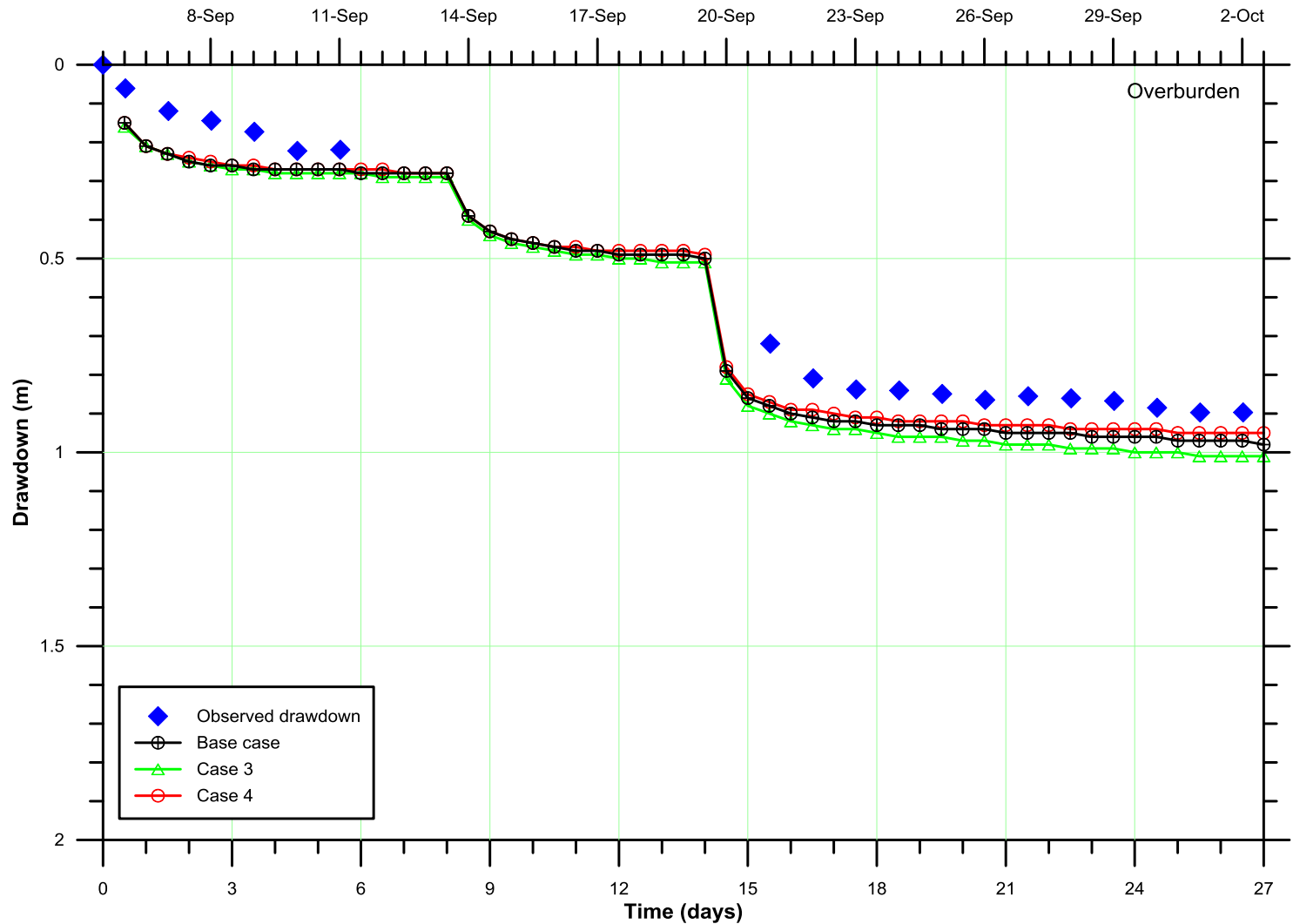


Figure 34 Simulated and Observed Hydrographs for CMOW1D-06 Sensitivity Analysis with Respect to the Specific Yield

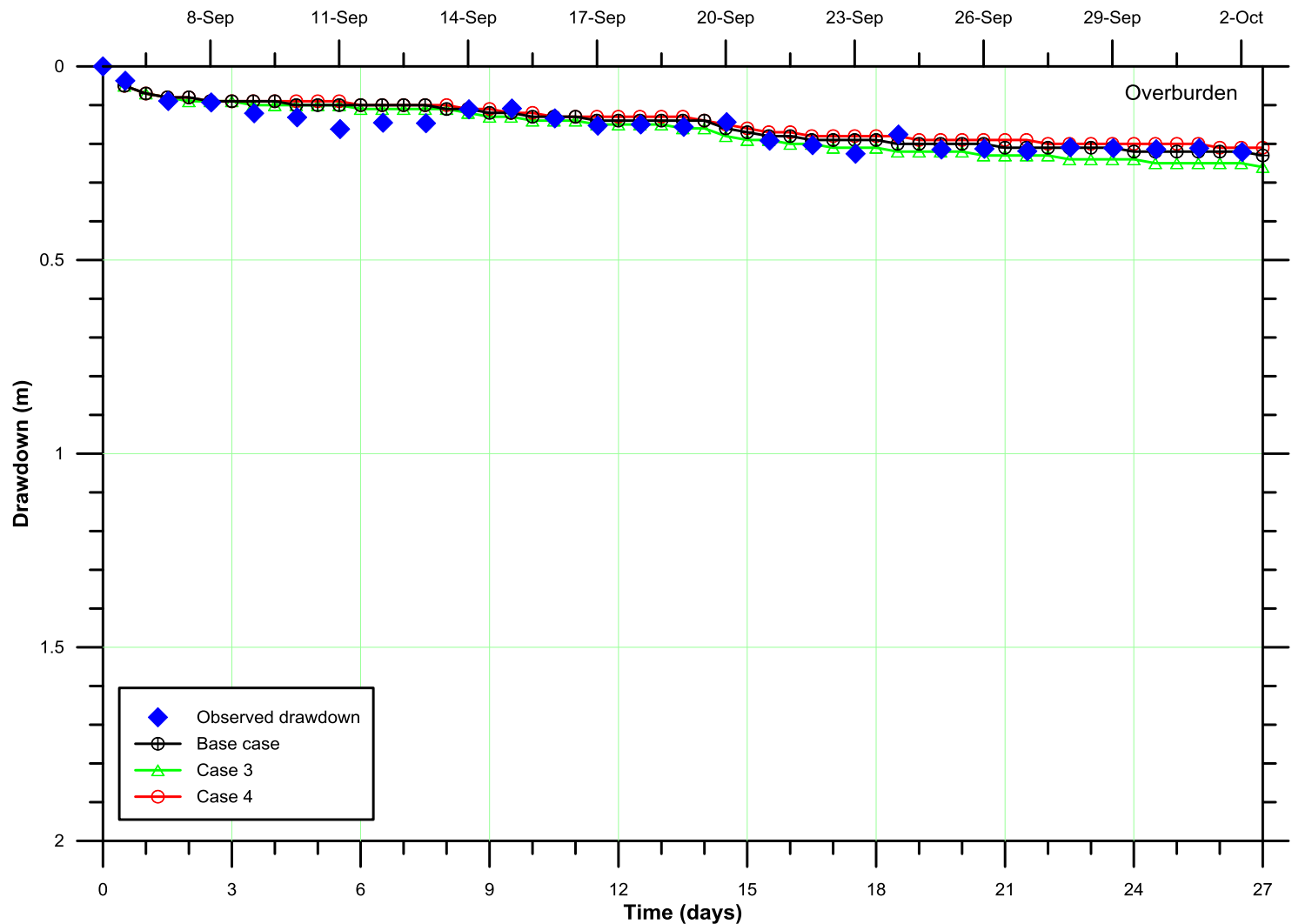


Figure 35 Simulated and Observed Hydrographs for CMOW2D-06 Sensitivity Analysis with Respect to the Specific Yield

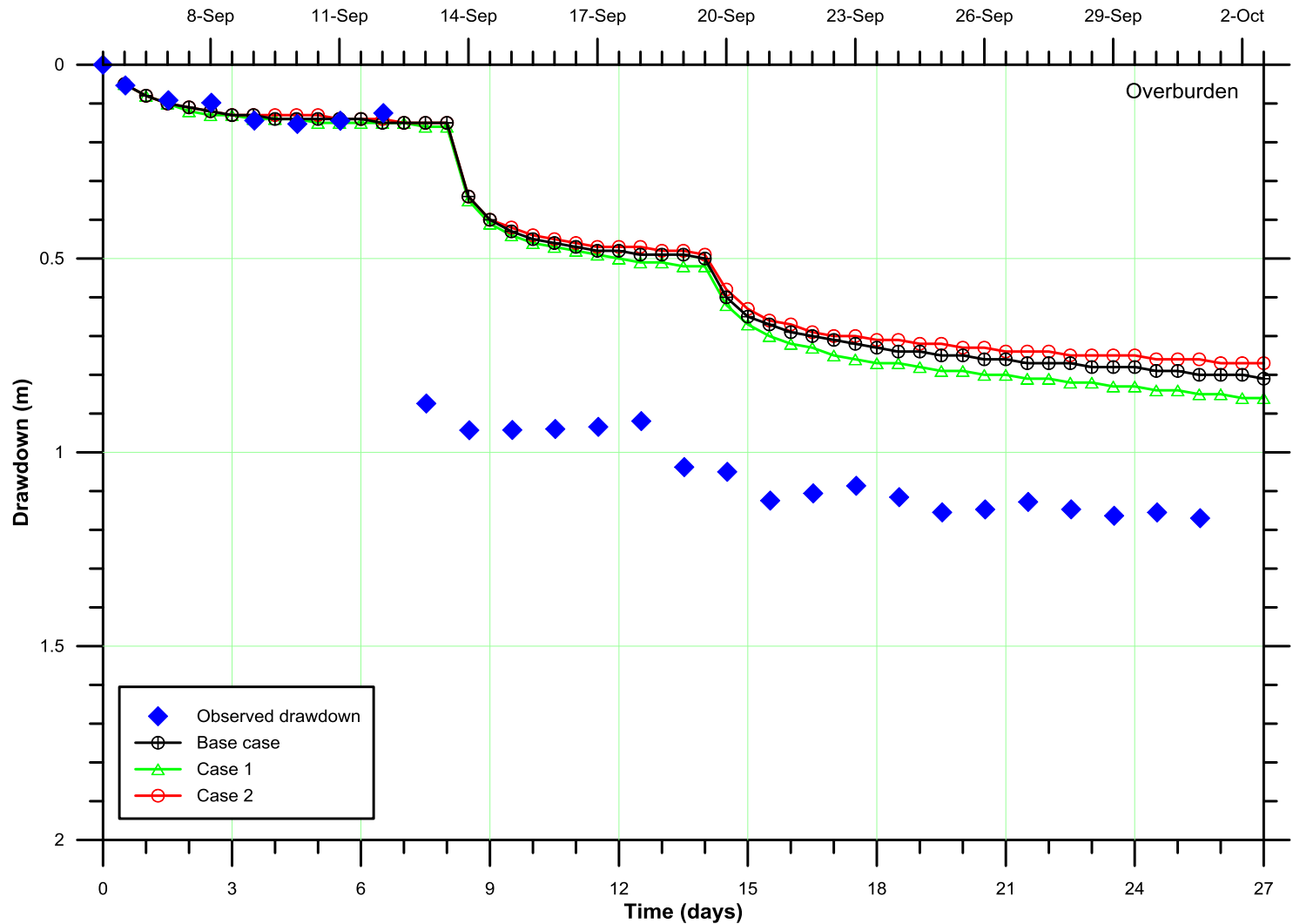
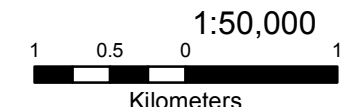
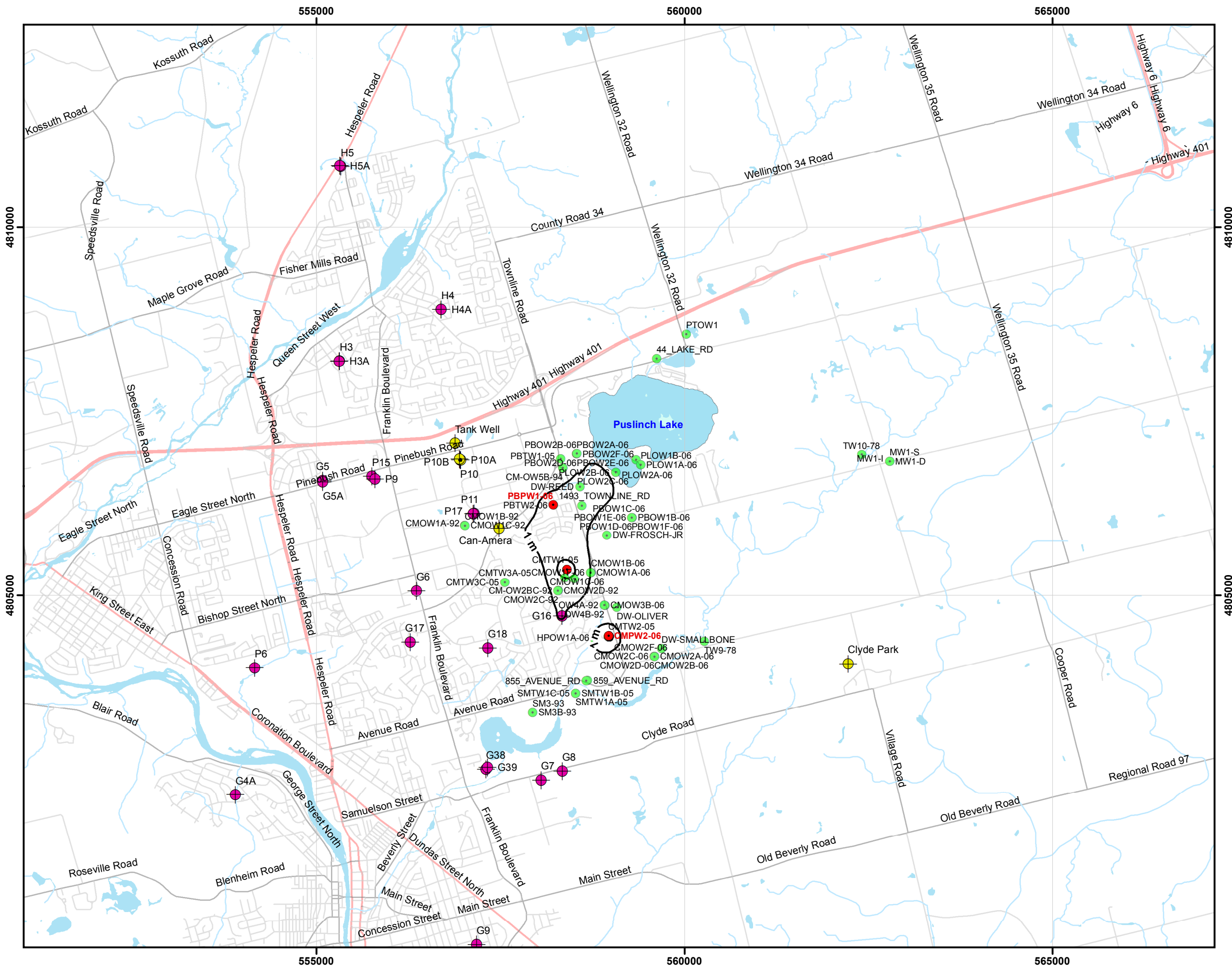


Figure 36 Simulated and Observed Hydrographs for PBOW2D-06 Sensitivity Analysis with Respect to the Specific Yield

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

- Wells Pumped during the 28-day Test
- Observation Wells
- Municipal Supply Well
- Test Production Well
- Drawdown Contours at the End of the Test
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds



Projection: UTM Zone 17N, NAD 83
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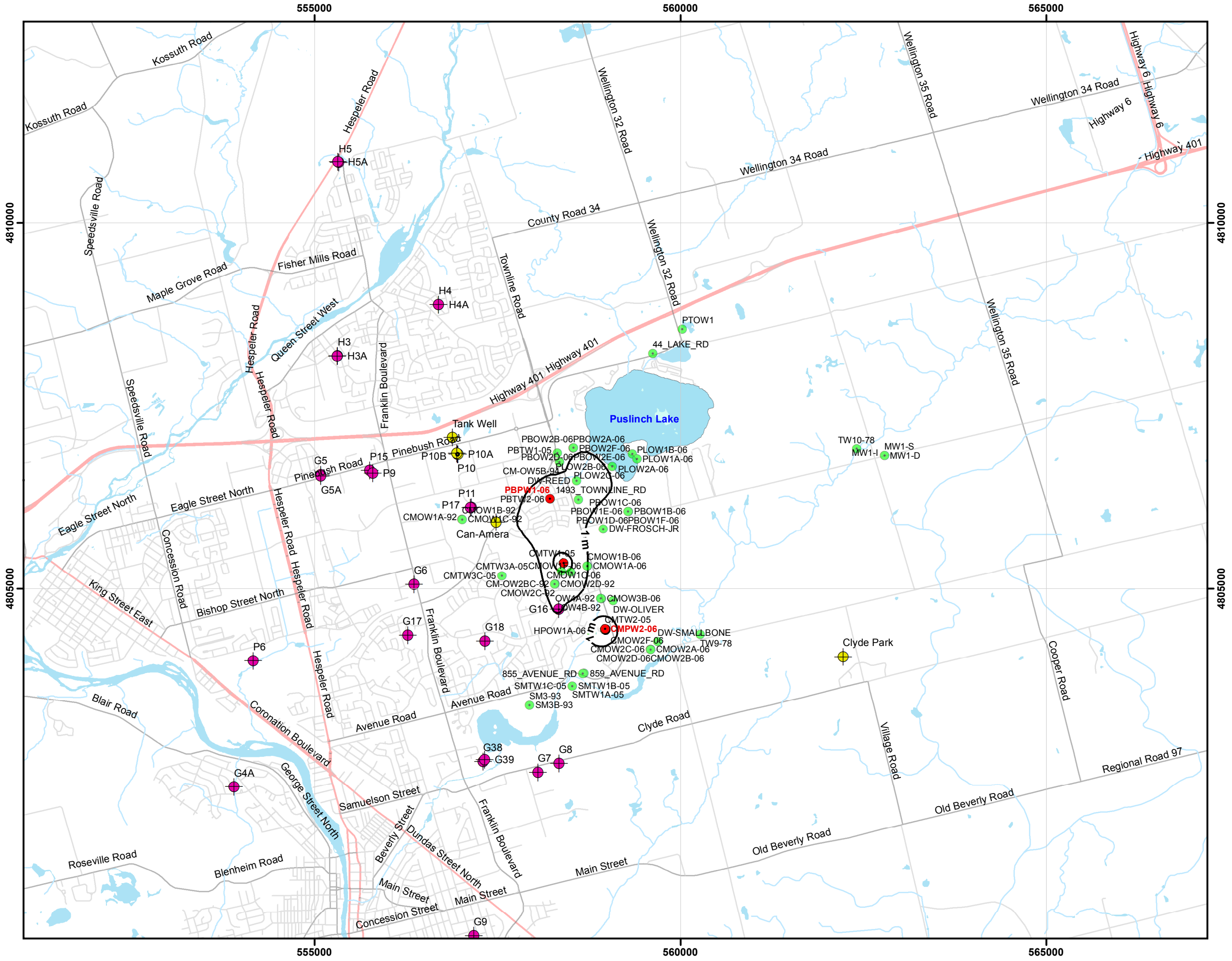


Figure 37
 Simulated Final Drawdowns in the Contact Aquifer for the Base Case (Sy=0.3)

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

- Wells Pumped during the 28-day Test
- Observation Wells
- Municipal Supply Well
- Test Production Well
- Drawdown Contours at the End of the Test
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds



Projection: UTM Zone 17N, NAD 83
 Map Version: 2, Map data 2014-05-27 J. Zhang

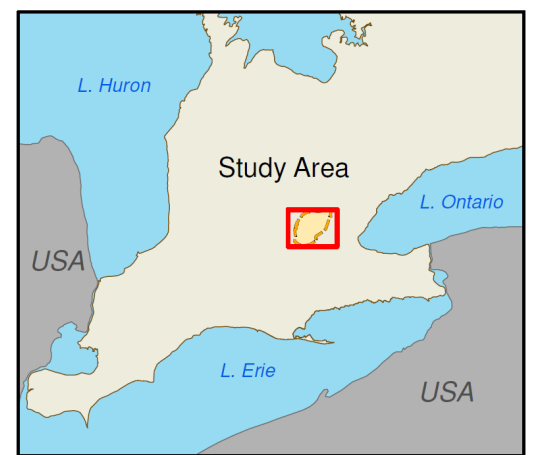
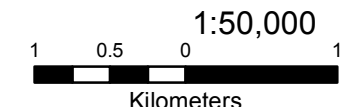
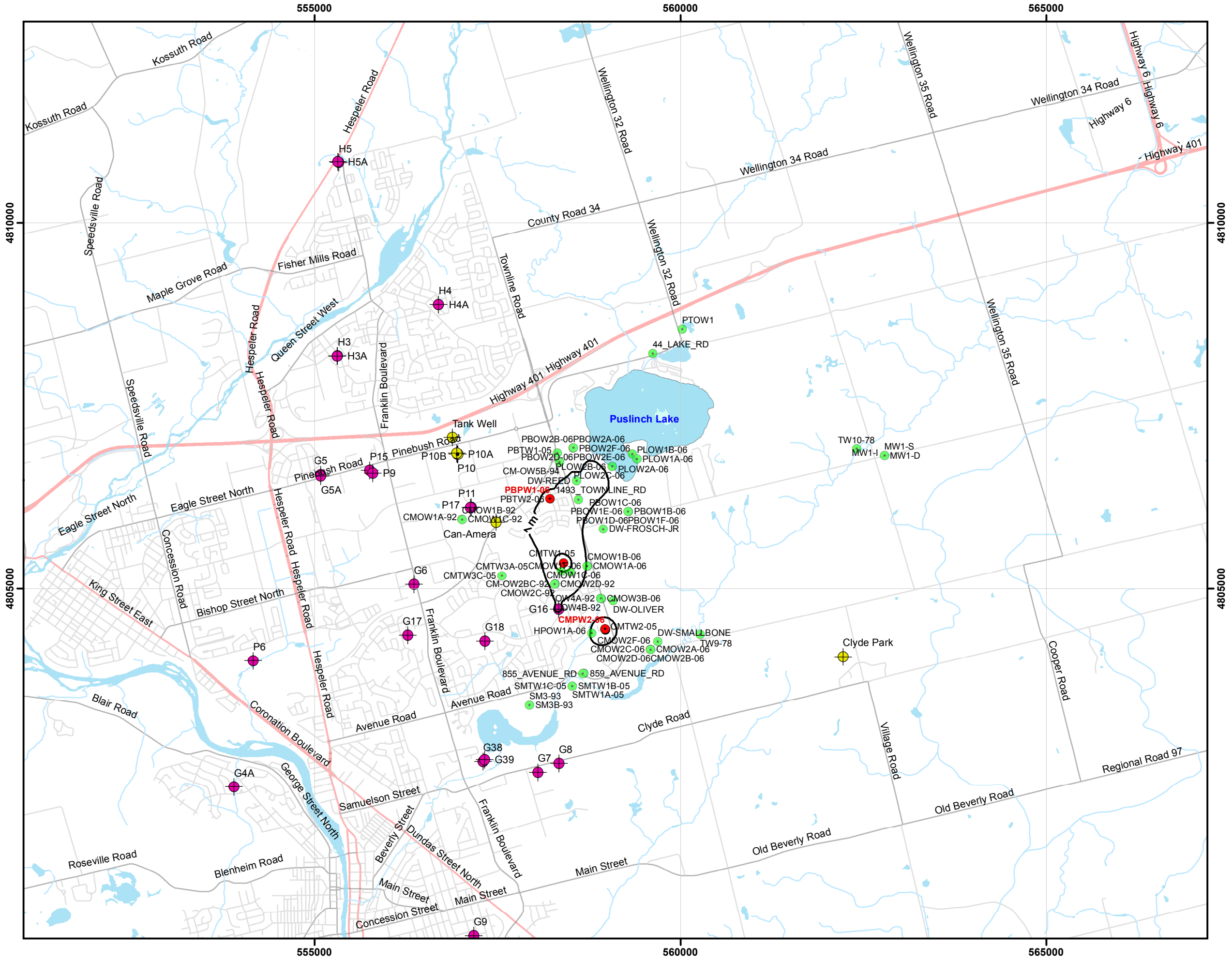


Figure 38
 Simulated Final Drawdowns in the Contact Aquifer for Case 3 (Sy=0.2)

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

- Wells Pumped during the 28-day Test
- Observation Wells
- Municipal Supply Well
- Test Production Well
- Drawdown Contours at the End of the Test
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds



Projection: UTM Zone 17N, NAD 83
 Map Version: 2, Map data 2014-05-27 J. Zhang

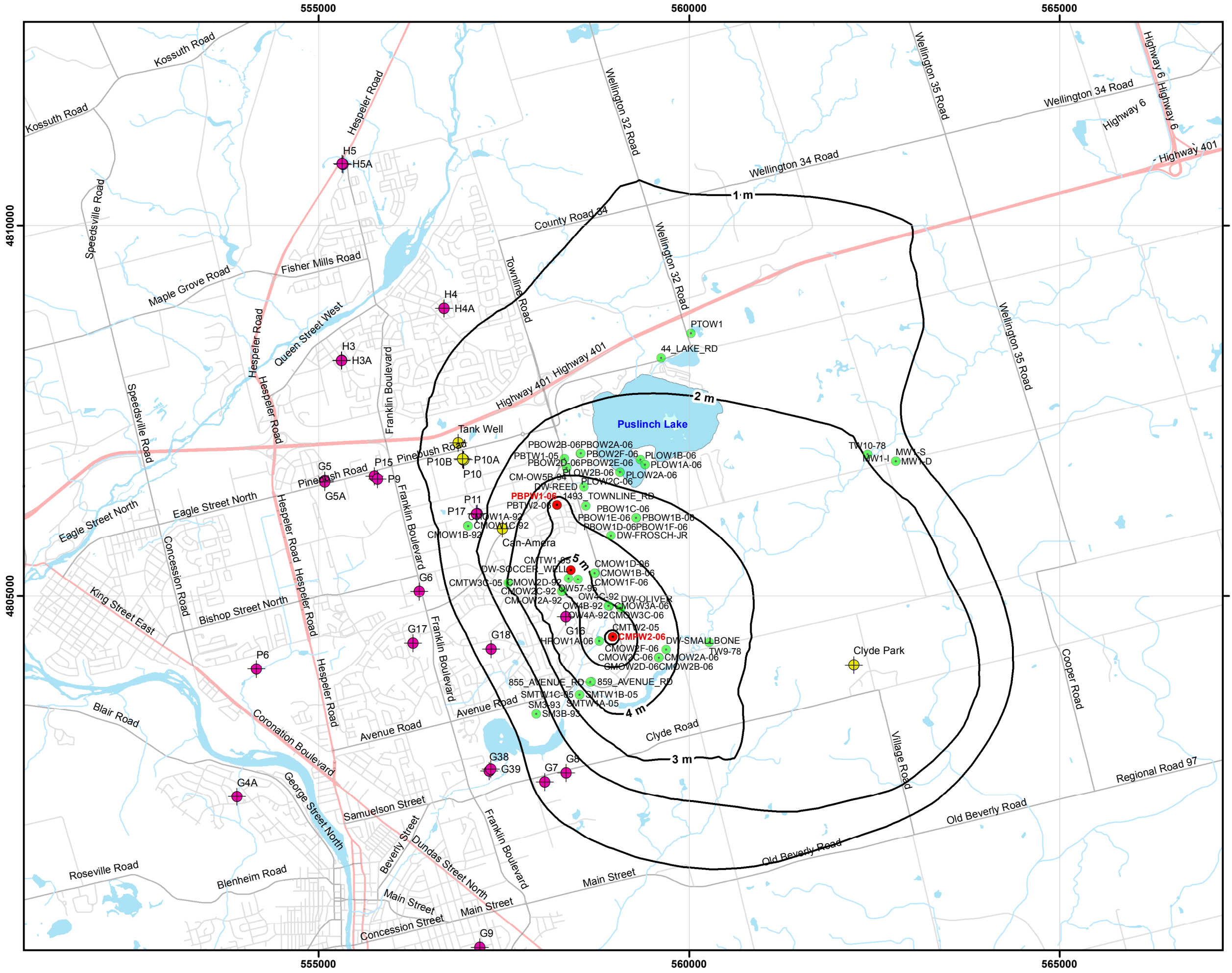


Figure 39
 Simulated Final Drawdowns in the Contact Aquifer for Case 4 (Sy=0.4)

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

- Wells Pumped during the 28-day Test
- Observation Wells
- Municipal Supply Well
- Test Production Well
- Drawdown Contours at the End of the Test
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds



Projection: UTM Zone 17N, NAD 83
 Map Version: 2, Map data 2014-05-27 J. Zhang

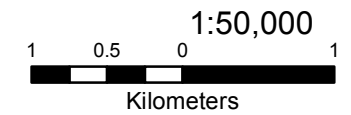


Figure 40
 Simulated Final Drawdowns in the Middle Gasport Formation Aquifer for the Base Case (Sy=0.3)

Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

- Wells Pumped during the 28-day Test
- Observation Wells
- Municipal Supply Well
- Test Production Well
- Drawdown Contours at the End of the Test
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds

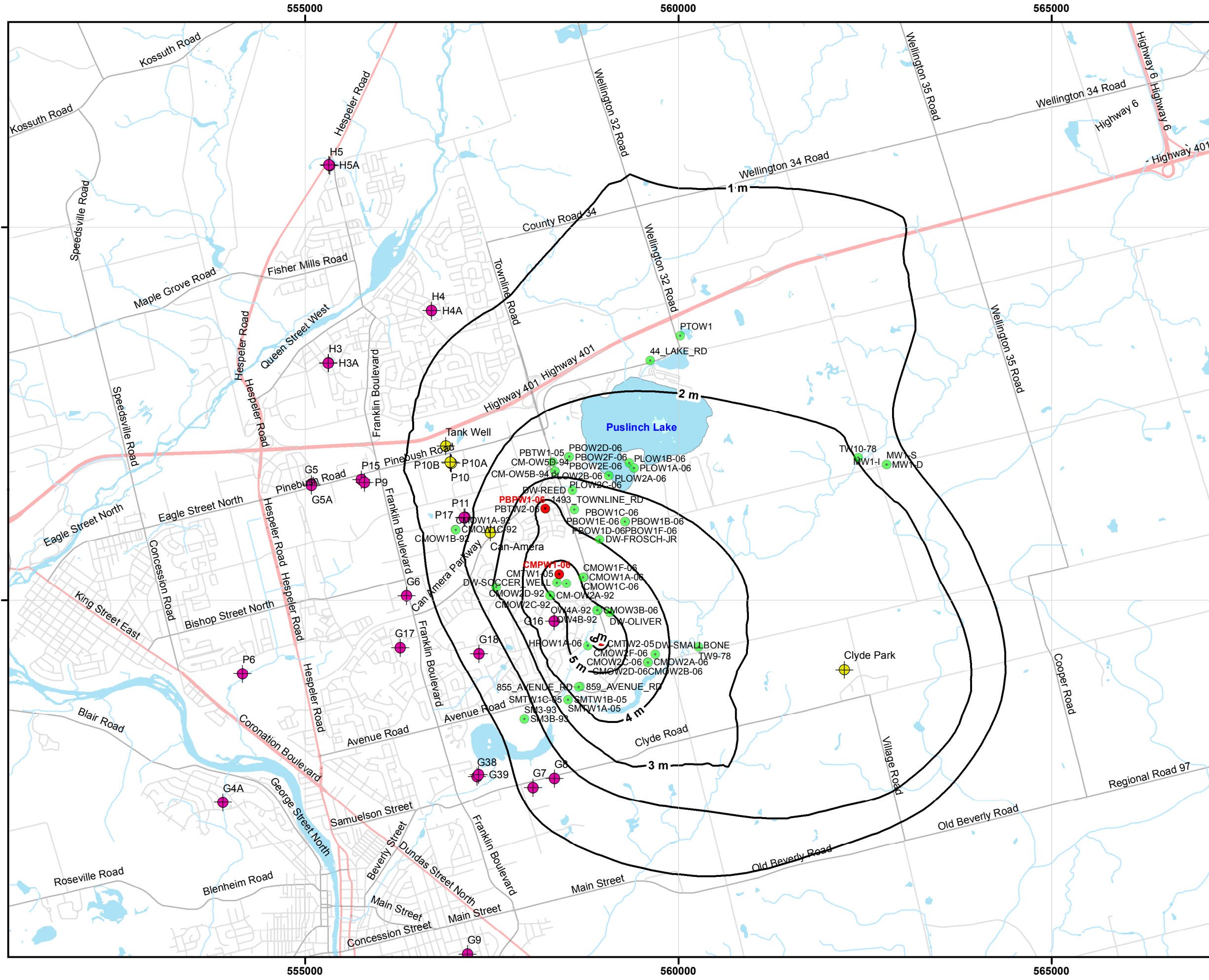


Projection: UTM Zone 17N, NAD 83
Map Version: 2, Map data 2014-05-27 J. Zhang



Figure 41

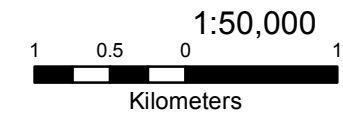
Simulated Final Drawdowns in the Middle Gasport Formation Aquifer for Case 3 (Sy=0.2)



Cambridge East IUS Water Supply Class EA: Groundwater Modelling

LEGEND

- Wells Pumped during the 28-day Test
- Observation Wells
- Municipal Supply Well
- Test Production Well
- Drawdown Contours at the End of the Test
- Expressway / Highway
- Major Roads
- Roads (collectors)
- Rivers / Streams
- Lakes and Ponds



Projection: UTM Zone 17N, NAD 83
Map Version: 2, Map data 2014-05-27 J. Zhang



Figure 42
Simulated Final Drawdowns in the Middle Gasport Formation Aquifer for Case 4 (Sy=0.4)

